

GEOTECHNICAL • ENVIRONMENTAL • HYDROGEOLOGICAL • BUILDING SCIENCE

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February 13, 2023 (Revision of Report dated November 30, 2022)

Reference No. 2206-S054

Page 1 of 4

Beachcroft Investments Inc. (Ballantry Homes) 20 Cachet Woods Court, Suite 6 Markham, Ontario L6C 3G1

Attention: Ms. Uzo Rossouw

Re: Preliminary Geotechnical Assessment

Proposed Residential Development

63 and 63A Trafalgar Road

Town of Erin

Dear Madam:

As per your request, we have completed the subsurface investigation for the captioned project and herein present our preliminary findings and recommendations.

SITE CONDITION

The subject site is located on the east side of Trafalgar Road and about 500 m north of Wellington 22 in the Town of Erin. At the time of investigation, the site is a farm field, consisting of soy bean crops. The existing site gradient is undulating with a grade difference of more than 10 m.

Based on the draft plan prepared by KLM Planning Partners Inc., the property will be developed with low rise residential dwellings serviced with municipal sewers and access roadway.

FIELD WORK

The field work, consisting of eleven (11) sampled boreholes with monitoring wells, extending to a depth of 4.7 to 6.7 m, was completed at the site between November 18 and 25, 2022. The borehole and monitoring well locations are shown on Drawing No. 1, enclosed. The depths and details of the monitoring wells are shown on the Borehole Logs.



The field work was supervised and the findings were recorded by a Geotechnical Technician. The ground elevation at each borehole location was obtained using a hand-held Global Navigation Satellite System (GNSS) equipment.

SOIL AND GROUNDWATER CONDITIONS

Detailed descriptions of the encountered subsurface conditions are presented on the enclosed Borehole Logs comprising Figures 1 to 11, inclusive. The revealed stratigraphy is plotted on the Subsurface Profile, Drawing No. 2.

The boreholes were performed on the farm field, where topsoil and ploughed soil were contacted at the ground surface in all boreholes. The thickness of the revealed topsoil is approximately 36 cm with the ploughed soil extending to a depth of 0.5 to 0.9 m below the prevailing ground surface. Beneath the ploughed soil, the site is underlain by predominantly sand and gravelly sand deposits. Sandy silt till and silt deposits were generally contacted in the lower stratigraphy in some of the boreholes. A localized sandy silt deposit was contacted near the ground surface below the ploughed soil in Borehole 6.

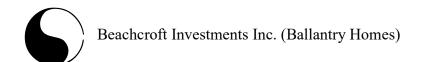
Based on sample examination, the sands and silt appeared to be in a wet state and may be water bearing at lower depths.

All boreholes were checked for the presence of groundwater upon completion of borehole drilling. Groundwater was recorded in Boreholes 3 and 5 at a depth of 5.5 m below grade or at El. 428.4 m and 428.7 m, respectively.

DISCUSSION AND RECOMMENDATIONS

Based on the borehole findings, the preliminary geotechnical considerations pertaining to the preliminary design of the development are presented below:

- 1. The topsoil and ploughed soil must be removed from the area of construction. They can be stockpiled and reused in landscaping areas only. Any surplus will have to be disposed off-site.
- 2. In areas where the site will be re-graded with additional earth fill, the earth fill should be placed in an engineered manner for underground services, building foundations and pavement construction.
- 3. The proposed structures can be supported on conventional footings, founded on engineered fill or sound native soil. The recommended soil bearing pressures of 150 kPa at Serviceability Limit State (SLS) and 240 kPa at Ultimate Limit State (ULS) can be



used for the design of spread and strip footings. The total and differential settlements of the conventional spread and strip footings, designed for the bearing pressure at SLS, are estimated to be 25 mm and 20 mm, respectively.

- 4. The footing subgrade must be inspected by a geotechnical engineer, or a geotechnical technician under the supervision of a geotechnical engineer. Footings exposed to weathering or in unheated areas, should have at least 1.4 m of earth cover for protection against frost action or must be adequately insulated.
- 5. Based on the borehole findings and the latest Ontario Building Code, the proposed development should be designed to resist an earthquake force using Site Classification 'D' (stiff soil).
- 6. Where the proposed structures have a basement, the basement should be maintained at least 1.0 m above the highest groundwater level. The perimeter walls should be dampproofed and provided with a perimeter drainage system. The subdrain must be encased in a fabric filter to protect them against blockage by silting and connected into a positive outlet.
- 7. The subgrade for slab-on-grade construction should consist of sound native soil or well compacted earth fill. The floor slab should be placed on a granular bedding of at least 15 cm thick, consisting of 19-mm Crusher-Run Limestone (CRL) or equivalent, compacted to its maximum Standard Proctor dry density (SPDD).
- 8. Where the separation between the basement and the groundwater is less than 1.0 m, underfloor subdrain system should also be implemented
- 9. Class 'B' bedding, consisting of compacted 19-mm CRL, or equivalent, is recommended for the construction of the underground utilities. Where saturated soils is evident in the subgrade soils or where dewatering is required, a Class 'A' concrete bedding should be considered. The service pipes must consist of leak-proof joints, or the joints must be wrapped with a waterproof membrane.
- 10. The on-site inorganic soils are generally suitable for trench backfill. Any wet soils must be pre-drained before reusing for backfill. The backfill in service trenches should be compacted to at least 95% SPDD in 20 cm layers or the lift thickness should be determined by test strips. In the zone within 1.0 m below the road subgrade or below the slab on grade, the material should be compacted to 98% SPDD with the water content at 2% to 3% drier than the optimum. This is to provide the required stiffness for the floor and pavement construction.

11. Excavation should be carried out in accordance with Ontario Regulation 213/91. The types of material are classified in the table below:

Material	Туре
Sandy Silt Till	2
Drained Sand, Gravelly Sand, Sandy Silt and Silt	3
Saturated Soils	4

12. For excavation extending below the groundwater level or into the saturated soils, the yield is expected to be appreciable and persistent. Dewatering by closely spaced sumps will be required.

This report is preliminary in nature and is subject to change in the comprehensive report.

We trust the above satisfies your present requirements. Should you have any further queries, please feel free to contact this office.

Yours truly,

SOIL ENGINEERS LTD.

Cedric Ramos, B.A.Sc.

Kin Fung Li, P.Eng. CR/KFL



ENCLOSURES

Logs of Boreholes	Figures 1 to 11
Borehole and Monitoring Well Location Plan	Drawing No. 1
Subsurface Profile	Drawing No. 2

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LIST OF ABBREVIATIONS AND DESCRIPTION OF TERMS

The abbreviations and terms commonly employed on the borehole logs and figures, and in the text of the report, are as follows:

SAMPLE TYPES

AS	Auger sample
CS	Chunk sample
DO	Drive open (split spoon)
DS	Denison type sample
FS	Foil sample
RC	Rock core (with size and percentage
	recovery)
ST	Slotted tube
TO	Thin-walled, open
TP	Thin-walled, piston
WS	Wash sample

SOIL DESCRIPTION

Cohesionless Soils:

vs/ft)	Relative Density
4	very loose
10	loose
30	compact
50	dense
50	very dense
	4 10 30 50 50

Cohesive Soils:

PENETRATION RESISTANCE

Dynamic Cone Penetration Resistance:

A continuous profile showing the number of blows for each foot of penetration of a 2-inch diameter, 90° point cone driven by a 140-pound hammer falling 30 inches.

Plotted as '---'

Standard Penetration Resistance or 'N' Value:

The number of blows of a 140-pound hammer falling 30 inches required to advance a 2-inch O.D. drive open sampler one foot into undisturbed soil.

Plotted as 'O'

WH	Sampler advanced by static weight
PH	Sampler advanced by hydraulic pressure
PM	Sampler advanced by manual pressure
NP	No penetration

Undrained Shear Strength (ksf)

Streng	<u>th (k</u>	<u>sf)</u>	<u>'N' (</u>	blov	vs/ft)	Consistency
less t	han	0.25	0	to	2	very soft
0.25	to	0.50	2	to	4	soft
0.50	to	1.0	4	to	8	firm
1.0	to	2.0	8	to	16	stiff
2.0	to	4.0	16	to	32	very stiff
C	ver	4.0	0	ver	32	hard

Method of Determination of Undrained Shear Strength of Cohesive Soils:

x 0.0 Field vane test in borehole; the number denotes the sensitivity to remoulding

 \triangle Laboratory vane test

☐ Compression test in laboratory

For a saturated cohesive soil, the undrained shear strength is taken as one half of the undrained compressive strength

METRIC CONVERSION FACTORS

1 ft = 0.3048 metres 1 inch = 25.4 mm 1lb = 0.454 kg 1ksf = 47.88 kPa



1

FIGURE NO.:

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 23, 2022

		(SAMP	LES		10	Dyn 3	amic 0	Cone 50		ws/30 '0	90			Atte	rberç	g Lim	nits			
EI. (m) Depth	SOIL DESCRIPTION	پ		Φ	Depth Scale (m)	×	She 50	ear Str	engt	h (kN. 150	/m²) 20				PL 			⊥ {		<u>[</u>	WAIER LEVEL
(m)		Number	Туре	N-Value	Depth 3	10 1	3	etration (blow	50	7	0	90 I				ure C		ent (%))	L +	WAI
439.6	Ground Surface				_																
0.0 439.1 0.5	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	12	0 -	0									16						
438.6	Brown, compact SAND fine to medium grained	2	DO	50/15	1 -								φ.							П	
1.0	Brown, very dense GRAVELLY SAND occ. cobbles and boulders				- -									1							
		3	DO	72	2 -						0										
		4	DO	50/10	- -								•							\mathbb{H}	
					3 -								į	5						•	
42E O		5	DO	88	<u>-</u>							0	Ľ								
435.9 3.7	Very dense to dense SILTY SAND TILL some gravel to gravelly a trace of clay occ. cobbles and boulders — brown	6	DO	50	4 -	-			0					8							lotion
	grey	7	DO	66	5 -				+	0				6							Dry Libon Completion
		8	DO	35		-		0						7						•	בו אנו בו
		9	DO	36	6 -	-		0						8						Ш	
433.0 6.6	END OF BOREHOLE																				
	Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m				7 -																
	Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing																				
					8 -				+												
					9 –																
					-				+												
					10				I												



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LOG OF BOREHOLE: JOB NO.: 2206-S054

FIGURE NO.:

2

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 22, 2022

			SAMP	PLES		10	Dyn.	amic Co		ows/30 70	90 cm)		Atte	erbei	rg Lim	its		
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	× 0	She 50 LLL Pen	ar Strer 100 L etration (blows)	ngth (ki 150 L L Resist /30 cm	N/m²) 20			PL - Mois	ture	Conte	L nt (%)		WATER LEVEL
436.3	Ground Surface										•							
0.0	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	6	0 -	0							14					
	Brown, loose SAND fine to medium grained	2	DO	5	1 -	0						6						
		3	DO	7	2 -	0						,	9					
433.9 2.4	Brown, dense to very dense GRAVELLY SAND occ. cobbles and boulders	4	DO	45	_			0				5						
	occ. cobbles and boulders	5	DO	50/15	3 -						(1						- - -
					4 -							5					-	pletion
		6	DO	50/15	5 -						(D						Dry upon Completion
		7	DO	60	6 -				0			3					- -	
<u>429.7</u> 6.6	END OF BOREHOLE Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 -													
					8 -													
					9 -													



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3

FIGURE NO.:

3

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 24, 2022

			SAMP	LES		10	-	namic 30	Cone 50		s/30 cm			Atte	berg	Limits		
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	×	She	ear St 100 L L netrati (blow	rength 0 1 on Re	n (kN/n 150 L L esistan cm)	n²) 200 1 1		• N	PL — //oistu	ıre Co	LL —	-	WATER LEVEL
434.1	Ground Surface																	
0.0 433.4	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	7	0	0								18				
0.7 432.7	Brown, compact SAND fine to medium grained	2	DO	12	1 -	0							10)				
	Brown, dense to very dense GRAVELLY SAND occ. cobbles and boulders wet below 5.5 m	3	DO	60	2 -	-			,	0			3					
		4	DO	52	- - -				0				3					_
		5	DO	39	3 -			0					3				- - - - - -	completion
					4 -	-						2						ı ≪W.L. @ El. 428.7 m upon completion
		6	DO	50/15	5 -	-]	≺W.L. @ EI.
		_			6 -								1	0			• - - -	
427.5 6.6	END OF BOREHOLE	7	DO	54			Ė		0)								
	Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 -	-												
					8 -													
					9 –													
					10		\pm											



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PROJECT DESCRIPTION: Proposed Residential Development METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin DRILLING DATE: November 18, 2022

			SAMP	PLES		10	Dynan 30		ne (blow	s/30 cm)) 90		Atte	erberç	g Limits	;		
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	× 0	Shear 0 Peneti	Streng 100 L L ration I	gth (kN/i 150 L L Resistan 30 cm)	200 ce		PL F Moist	ure C	LL	(%)		WATER LEVEL
		_	-) 7(+	<u> </u>	<u> </u>	30 	Ĭ L	-	>
427.6 0.0	Ground Surface 36 cm Topsoil				0 -						+	16					
	PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	11	-	0						•					
426.7 0.9	Brown, dense to very dense GRAVELLY SAND occ. cobbles and boulders	2	DO	50/5	1 -						0	16					
	occ. cobbles and boulders	3	DO	50/15							4 0●						
			D0	24	2 -						4						
		4	DO	34	3 -		C				2					•	
		5	DO	38	- -			0									
					4 -						3					 - - -	pletion
		-6-	DO	50/15	5 -						0					1 [[]]	Dry upon Completion
					_												Dry L
421.2 6.4	END OF BOREHOLE	7	DO	50/10	6 -						3						
0.4	Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m				7 -												
	Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				- -												
					8 -												
					9 -												
					9 -												
					10												



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Page: 1 of 1

FIGURE NO.:

4

5

FIGURE NO.:

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 24, 2022

		(SAMP	LES		10		amic C 0	Cone (50	blows/3 70	30 cm) 90		Atte	rberg	Limits		
II. n) pth n)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	>	Sh 6	ear Stre 100 L L etration (blows	ength 15 1 I I n Resi	(kN/m²) 60 2 istance m)) !00	•	PL 		LL 		
		Ž	Ту	Ż	De	10	3	0	50 1	70	90		10 	20 	30 	40	-
3.8	Ground Surface 36 cm Topsoil				0 -							<u> </u>	11.				
33.1	PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	4		0							16				
).7	Brown, compact SAND fine grained some silt	2	DO	7	1 -	0						5					
		3	DO	4	2 -	0						6					
		4	DO	21			0					5					
		5	DO	27	3 -		O						11				
29.7 4.1	Brown, dense				4 -												-
	GRAVELLY SAND occ. cobbles and boulders	6	DO	42	5 -			0									
28.2						-											
5.6	Brown, dense SANDY SILT TILL traces of gravel and clay occ. cobbles and boulders				6 -									19			
27.2	occ. copples and boulders	7	DO	40				ф									
5.6	END OF BOREHOLE Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 -												
					8 -												
					9 -												
					10												



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6

FIGURE NO.:

6

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 21, 2022

		:	SAMP	LES		10	3		50	70	90	А	tterb	erg L	imits		
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	×	She 50 L Pen	ar Stre	ngth 15	(kN/m 50 stanc istanc m)	²) 200 e) Mo	isture		LL itent (9	WATER LEVEL	
443.6	Ground Surface																
0.0	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	10	0	0						1-					
0.7	Brown, compact SANDY SILT some gravel	2	DO	12	1 -	0						11					
441.4		3	DO	10	2 -	0						12					
2.2	Brown, compact to very dense SANDY GRAVEL occ. cobbles and boulders	4	DO	25	-		0					12					
		5	DO	38	3 -			0				11					
		6	DO	24	4 -		0					12					pletion
438.6		7	DO	50/28	5 -							8					Dry upon Completion
5.0	END OF BOREHOLE Installed 50 mm Ø monitoring well to 4.6 m completed with 3.1 m screen Sand backfill from 0.9 to 4.6 m Bentonite seal from 0.0 m to 0.9 m Provided with a monument casing				6 -												Dry upo
					7 -												
					8 -												
					9 –												
					9 -												



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LOG OF BOREHOLE: JOB NO.: 2206-S054

METHOD OF BORING: Flight Auger

PROJECT DESCRIPTION: Proposed Residential Development

7

FIGURE NO.:

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 23, 2022

			SAMP	LES		10		30	5	0	70				At	terb	erg	Limit	s			
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)		50 P	hear	Strer 100 ration plows	ngth (15 L L Resi /30 cr	kN/m) stanc n)	²) 200 e			Moi			LL onter	ut (%))		WATER LEVEL
442.3	Ground Surface																					
0.0	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	6	0 -	0									13						I	
0.7	Brown, very loose to very dense SAND fine to well graded a trace to some gravel	2	DO	3	1 -	b									12							
		3	DO	18	2 -		0							3								
		4	DO	48	_				С)				3								
		5	DO	34	3 -			C)					3							•+ - - -	
					4 -																	pletion
		6	DO	50/23	5 -								¢	3							ш	Dry upon Completion
					6 -									3							• - • - • -	۵
435.7 6.6	END OF BOREHOLE	7	DO	45	- -				0					•								
	Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 -																	
					8 -																	
					9 -																	
					10																	



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8

FIGURE NO.:

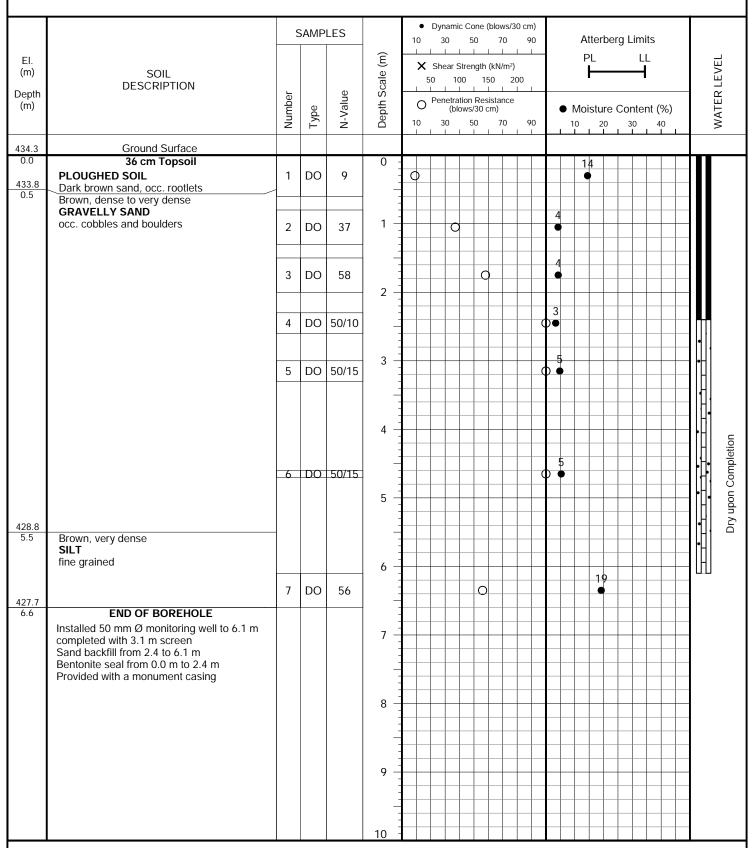
8

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 22, 2022





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FIGURE NO.:

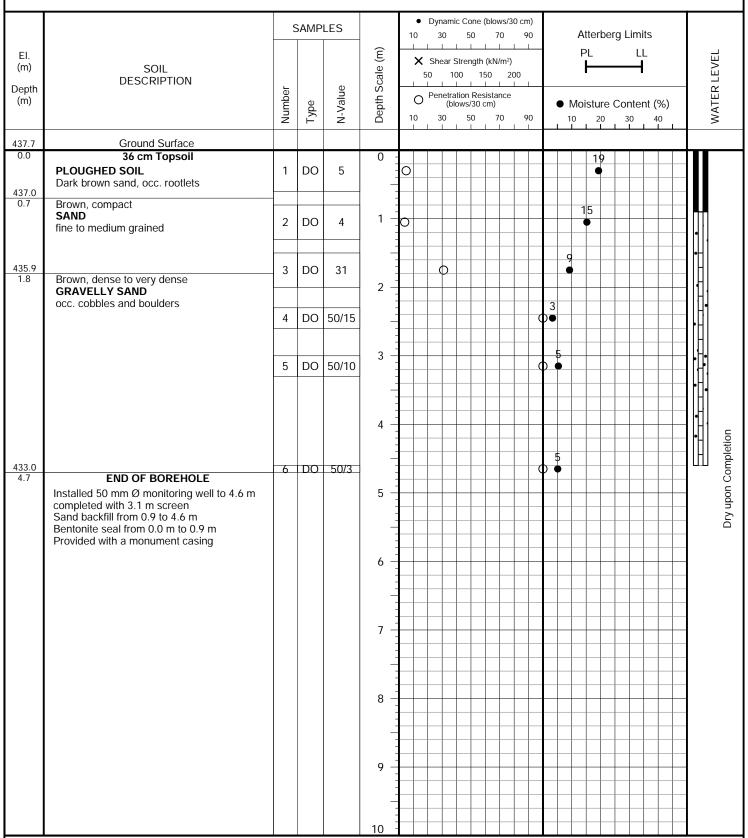
9

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 24, 2022



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10

FIGURE NO.:

10

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 25, 2022

		5	SAMP	LES		10		ynan 30	nic Co 5		lows/3 70	30 cm) 90			Α	tterk	berg	Lim	its		
EI. (m) Depth (m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)		50 D P	hear	100	ngth (I 150 L Resis /30 cn	kN/m²)	00)		F	PL 	re Co	L	ent (%		WATER LEVEL
37.9	Ground Surface																				
0.0 137.2	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	4	0 :	0										17 •					
0.7	Brown, compact SAND fine to medium grained	2	DO	17	1 -		0							7							
136.4 1.5	Brown, dense to very dense GRAVELLY SAND occ. cobbles and boulders wet below 5.1 m	3	DO	50/28	2 -								0	7							
		4	DO	26	-		(>						4						•	
		5	DO	46	3 -				0					6						• - • •	
		6	DO	22	4 -		0)						3						•	notion
		7	DO	50/15	5 -								þ	4						 - -	Dry IIoon Completion
		8	DO	42	- - - - - - -				0						12					•	Č
31.2		9	DO	32	- - - -			0							12 •						
5.7	END OF BOREHOLE Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 -																
					8 -																
					9 -																



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11

FIGURE NO.:

11

PROJECT DESCRIPTION: Proposed Residential Development

METHOD OF BORING: Flight Auger

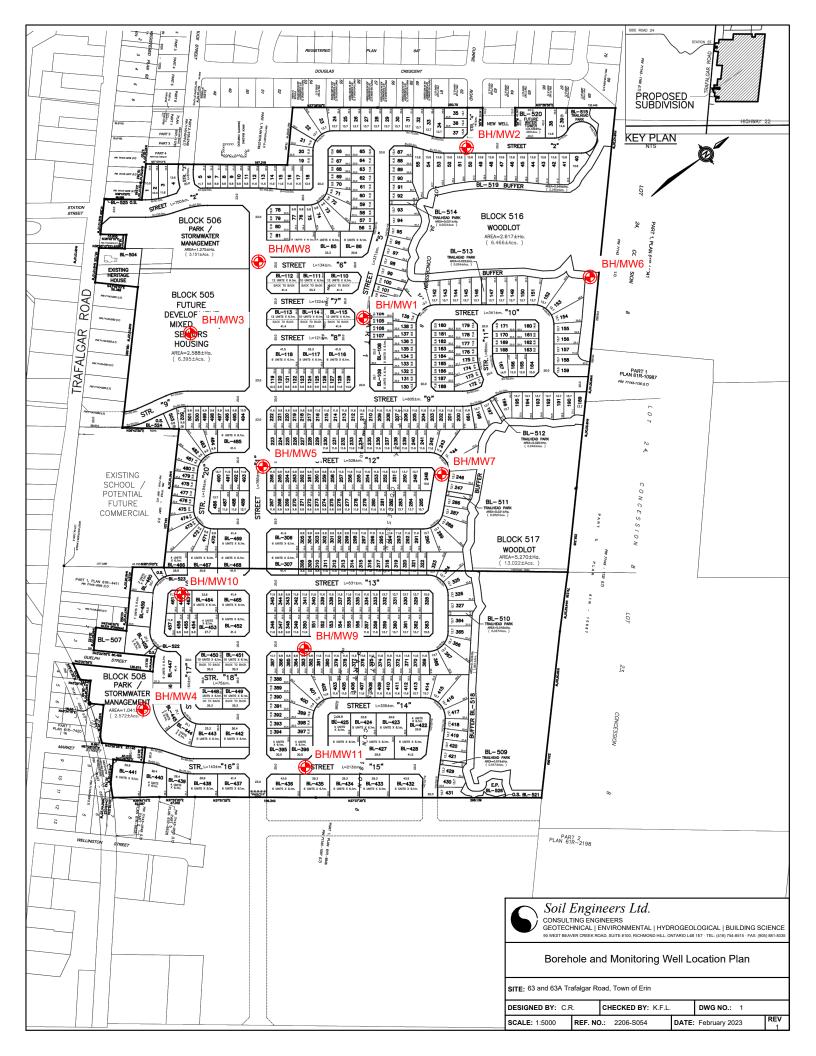
PROJECT LOCATION: 63 and 63A Trafalgar Road, Town of Erin

DRILLING DATE: November 25, 2022

		5	SAMP	LES		• 10	Dyn.	amic Co	one (b 0	lows/: 70	30 cm) 90			Atte	rberg	Limit	S		
El. m) epth m)	SOIL DESCRIPTION	Number	Туре	N-Value	Depth Scale (m)	×	She	ar Strer 100 I etration (blows)	ngth (I	(N/m²)	200		• N	PL 	ıre Co	_LL 			WATER LEVEL
5.5	Ground Surface																	丄	
.0	36 cm Topsoil PLOUGHED SOIL Dark brown sand, occ. rootlets	1	DO	12	0	0								12 •				_	
.7	Brown, dense to very dense SAND well graded trace to some gravel	2	DO	40	1 -			0					5					_	
		3	DO	74	2 -					0			6					_	
		4	DO	62					c)		2						_	
		5	DO	46	3 -			0				4	1					 - -	
1.4 .1	Dense SILT fine grained occ. clay seams				4 -										25			- - - - - - - -	
		6	DO	38	5 -			0											•
	<u>brown</u>	7	DO	38	6 -			0							22				
<u>3.9</u> 6	END OF BOREHOLE Installed 50 mm Ø monitoring well to 6.1 m completed with 3.1 m screen Sand backfill from 2.4 to 6.1 m Bentonite seal from 0.0 m to 2.4 m Provided with a monument casing				7 - 8 - 9 -														



Soil Engineers Ltd.





Soil Engineers Ltd.CONSULTING ENGINEERS

GEOTECHNICAL | ENVIRONMENTAL | HYDROGEOLOGICAL | BUILDING SCIENCE

SUBSURFACE PROFILE **DRAWING NO. 2 SCALE: AS SHOWN**

JOB NO.: 2206-S054

REPORT DATE: February 2023

Proposed Residential Development PROJECT DESCRIPTION:

GRAVELLY SAND

SANDY SILT SILT

PLOUGHED SOIL

SAND

SANDY SILT TILL

LEGEND

63 and 63A Trafalgar Road, Town of Erin **PROJECT LOCATION:**

