TRAFFIC IMPACT STUDY

PROPOSED RESIDENTIAL SUBDIVISION HILLSBURGH HEIGHTS INC. 5916 TRAFALGAR ROAD NORTH HILLSBURGH URBAN AREA TOWN OF ERIN TOWN FILE NOS. OP21-01 & Z21-09

UPDATED JULY 28TH 2022



PROJECT NO. W21081

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1. INTRODUCTION

CANDEVCON LIMITED was retained by Hillsburgh Heights Inc. to undertake a Traffic Impact Study in support of the Draft Plan of Subdivision and Zoning By-law Amendment Application for the proposed Residential Subdivision that is located immediately west of Trafalgar Road North and approximately two (2) kilometres north of Wellington Road 22, in the Town of Erin. **Figure 1** illustrates the location of the proposed Residential Subdivision.

As a requirement of the approval process, the Town of Erin and the County of Wellington require the preparation of a Traffic Impact Study to support the proposed Residential Subdivision and to examine the implications of the proposed Residential Subdivision on the adjacent transportation infrastructure.

It is anticipated that the proposed Residential Subdivision will be fully built-out and occupied by 2026. As a result, a full build-out 2026 horizon along with a five (5) year post build-out 2031 horizon were analyzed.

The Terms of Reference for the Study (copy included in **Appendix A**) were circulated to the County of Wellington and to the Town of Erin and the comments were received as included in **Appendix A**. The comments¹ provided on behalf of the Town requested that the TIS should also account for four other proposed developments in the Community of Hillsburgh. Since no information is available with respect to the development timing, it was assumed that they would also be fully built-out by 2026. The comments provided on behalf of the Town also stipulated that 5 year and 10 year post full build-out be analyzed. Since there are too many variables associated with a 10 year build-out in the context of other potential developments, a 10 year build-out was not analyzed.

The purpose of this Study is to determine the traffic impacts of the proposed Residential Subdivision as well as the other proposed developments on the surrounding road network.

¹ E-mail dated October 21st 2021.

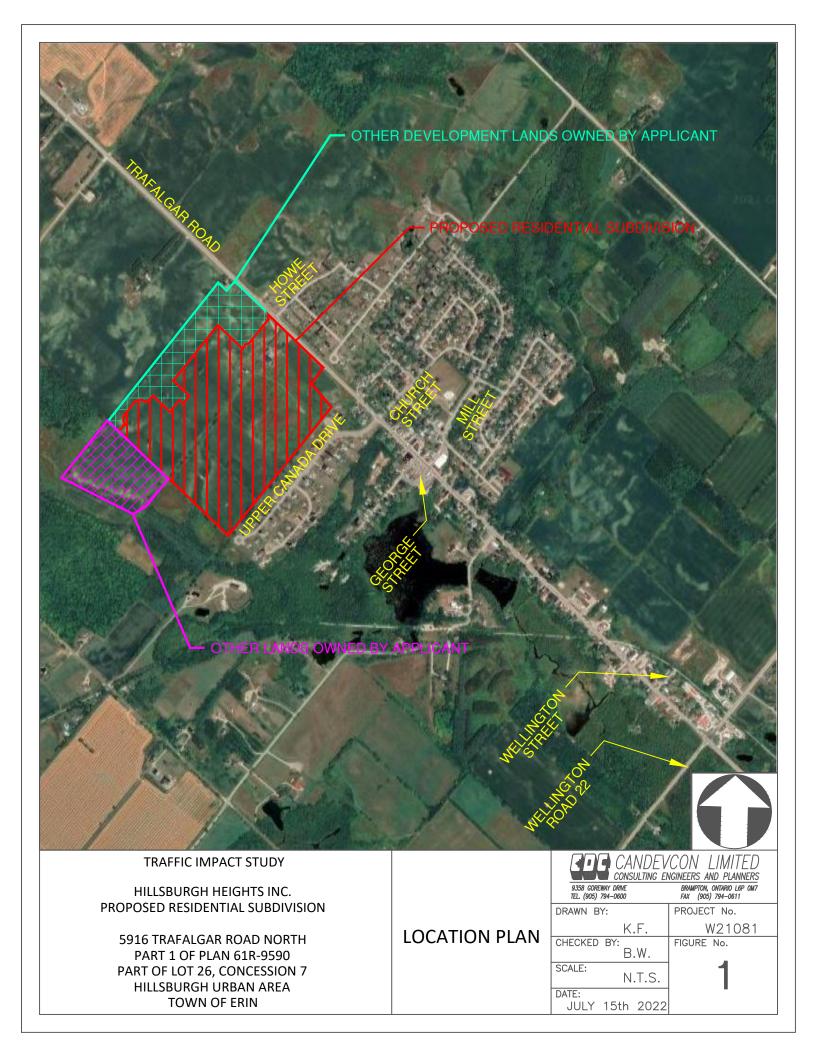
1. INTRODUCTION (CONT'D)

The Traffic Impact Study addresses the future operations at the following intersections as stipulated by the Town:

- Trafalgar Road North at Wellington Road 22,
- George Street/Mill Street at Trafalgar Road North,
- Upper Canada Drive/Church Street at Trafalgar Road North,
- Proposed Street 'A'/Howe Street at Trafalgar Road North,
- Proposed Street 'E' at Trafalgar Road North,
- Proposed Street 'A' at proposed Street 'G'/future Local Road.

The Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North and Howe Street at Trafalgar Road North intersections were studied under the Existing (2021), the Future (2026 & 2031) Total Background and the Future (2026 & 2031) Total Traffic scenarios. The proposed Street 'E' at Trafalgar Road North and proposed Street 'A' at proposed Street 'G'/future Local Road intersections were studied under the Future (2026 & 2031) Total Traffic scenarios.

The Traffic Impact Study addresses the traffic operations during the typical Weekday A.M. and Weekday P.M. Peak Hours.



2. SUBJECT DEVELOPMENT – STUDY AREA

The Subject Property is located immediately west of Trafalgar Road North and approximately two (2) kilometres north of Wellington Road 22. The total area of the property is 79.81 acres.

The Subject Development is surrounded by the following land uses:

- To the north, development lands owned by the applicant with future commercial and residential land uses,
- To the east, Trafalgar Road North with existing residential beyond,
- To the south, existing residential with Upper Canada Drive beyond,
- To the west, vacant lands owned by the applicant and other vacant lands with woodlands beyond.

The proposed Residential Subdivision comprises 196 single detached homes (includes the heritage house in Block 8), 174 townhouse units, a school block, a park and twelve (12) public roads. Vehicle access to the proposed Residential Subdivision is provided via Streets 'A' and 'E', which connects with Trafalgar Road North. The proposed Street 'A' access will be a full-moves access that aligns with Howe Street to form a four-legged intersection. The proposed Street 'E' access will be a full-moves access that is located at the southeast corner of the Subject Property. In addition, west of the Howe Street/proposed Street 'A' at Trafalgar Road North intersection, proposed Street 'B' and a future Local Road will align at proposed Street 'A'; forming a four-legged roundabout intersection.

To minimize the proposed Residential Subdivision's impact to Trafalgar Road North, an internal connection to McMurchy Lane was considered so that site-generated trips can use Upper Canada Drive to access Trafalgar Road North rather than providing the Street 'E' at Trafalgar Road North intersection. However, it was determined that an internal connection to McMurchy Lane was not feasible due to grading constraints.

The proposed Draft Plan of Subdivision is provided in Figure 2.

EXISTING AGRICULTURAL ANTICIPATED BACKGROUND DEVELOPMENT (LANDS OWNED BY APPLICANT) BLOCK 5 FUTURE DEVELOPMENT (RESIDENTIAL) 184 BLOCK 5A FUTURE DEVELOPMENT (COMMERCIAL) BLOCK 2 SCHOOL STREET 'B' BLOCK 1 PARK TREET 'A' HOWE STREET BLOCK 6 VISTA STREET 'A' BLOCK 1BA No.4 TH Autous BLOCK 13 OPEN SPACE/ AGRICULTURAL STREET 'L' STRE BLOCK 3 SWM POND 10 HILL STREET 'E' 11 STREET 'H' BARBOUR DRIVE 46 47 95 50 BLOCK 4 SWM POND 30 STREET 'H' BLOCK 7 WALKWAY existind residentilal QUEEN STREET \ \ 1 UPPER CANADA DRIVE CHURCH STREET ==== TRAFFIC IMPACT STUDY HILLSBURGH HEIGHTS INC. **PROPOSED DRAFT PLAN OF SUBDIVISION** PROPOSED RESIDENTIAL SUBDIVISION 5916 TRAFALGAR ROAD NORTH TOWN OF ERIN

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	TEL. (905) 79	4-0600		FAX (90	5) 794-0611	
DATE:	JULY,	15th	2022	JOB	No. W2 ⁻	1081
DESIG	N:	K.F.		FIG.	No.	
SCALE	:	N.T.S			2	

3. EXISTING AND FUTURE ROAD NETWORK

3.1 Existing Road Network

The road network within the Study Area is described below:

Trafalgar Road North

Trafalgar Road North is an existing Arterial Road that is under the jurisdiction of the County of Wellington. Within the Study Area, Trafalgar Road North is a two (2) lane roadway with a posted speed limit of 40 km/h. From Wellington Road 22 to approximately 200 metres south of Howe Street the roadway consists of an urban cross-section. From approximately 200 metres south of Howe Street to the north end of the Study Area (Howe Street), the roadway consists of a rural cross-section. Within the Study Area, where an urban cross-section is provided, a pedestrian sidewalk or a multi-use path is provided on at least one side of the roadway.

Wellington Road 22

Wellington Road 22 is an existing Arterial Road that is under the jurisdiction of the County of Wellington. Within the vicinity of the Study Area, Wellington Road 22 is a two (2) lane roadway with a rural cross-section. The roadway has a posted speed limit of 70 km/h east of Trafalgar Road North and a posted speed limit of 80 km/h west of Trafalgar Road North.

George Street

George Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of 50 km/h and a rural cross section. George Street consists of two (2) components: an east-west roadway that connects with Trafalgar Road to the east and that terminates at a driveway of a dwelling unit to the west; and a roadway that travels in the west direction before travelling in the north direction with connections to Mill Street and Trafalgar Road North easterly and to its other component northerly.

3. EXISTING AND FUTURE ROAD NETWORK (CONT'D)

3.1 Existing Road Network (Cont'd)

Mill Street

Mill Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of 50 km/h and a rural cross section. From its connection with George Street and Trafalgar Road North, the roadway travels in the east direction before travelling in the north direction to connect with Orangeville Street.

Upper Canada Drive

Upper Canada Drive is an existing east-west local road that is under the jurisdiction of the Town of Erin. The roadway connects with Trafalgar Road North and Church Street easterly and terminates in a cul-de-sac at the western end. The local road comprises two (2) lanes with an assumed speed limit of 50 km/h, an urban cross section and a pedestrian sidewalk on the south side.

Church Street

Church Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of 50 km/h and a rural cross section. From its connection with Upper Canada Drive and Trafalgar Road North, the roadway travels in the east direction before travelling in the south direction to connect with Mill Street. From Trafalgar Road North to Barker Street, a pedestrian sidewalk is provided on at least one side of the roadway.

Howe Street

Howe Street is an existing local road that is under the jurisdiction of the Town of Erin. The roadway connects with Trafalgar Road North at the west end to form a T-intersection and connects with Wallace Street at the east end to form a road elbow. The local road comprises two (2) lanes with an assumed speed limit of 50 km/h and a rural cross section.

3. EXISTING AND FUTURE ROAD NETWORK (CONT'D)

3.2 Future Road Network

By the 2031 horizon year, it is not anticipated that Trafalgar Road North, Wellington Road 22, George Street, Mill Street, Upper Canada Drive, Church Street, and Howe Street will be widened. However, by the 2026 horizon year, it is anticipated that two (2) collector roads will be constructed within the vicinity of the Study Area². A collector road (proposed West Collector Road) will connect with Wellington Road 22 at approximately 500 metres west of Trafalgar Road North. From Wellington Road 22, the collector road will travel in the north direction before travelling in the north-east direction to connect with Station Street. The east end of Station Street that connects with Trafalgar Road North will be upgraded to a collector road to form part of the proposed roadway. The second collector road (proposed East Collector Road) that is anticipated will connect with Wellington Road 22 at approximately 350 metres east of Trafalgar Road North. From Wellington Road 22, the collector road will travel in the north direction before travelling in the west direction to connect with Trafalgar Road North. The connection with Trafalgar Road North will be immediately south of the existing Station Street at Trafalgar Road North intersection. The proposed collector roads are illustrated in **Figure** 5.

The proposed Hillsburgh Heights Residential Subdivision comprises twelve (12) public roads with a full-moves access (proposed Street 'A') that connects with Trafalgar Road North and Howe Street and a full-moves access (proposed Street 'E') that connects with Trafalgar Road North. In addition, west of the Howe Street/proposed Street 'A' at Trafalgar Road North intersection, proposed Street 'G' and a future Local Road will align at proposed Street 'A'; forming a four-legged roundabout intersection.

Given the existing conditions on Trafalgar Road in relation to pedestrian connections, there are currently no sidewalk on Trafalgar Road or Howe Street. There are no future warrants for any sidewalk connections required on the eastside of Trafalgar Road.

² Town of Erin's Official Plan – Office Consolidation, Town of Erin, October 2021.

4. EXISTING TRAFFIC CONDITIONS

4.1 Existing Traffic

The Existing (2021) traffic volumes for the concerned intersections are based on the turning movement counts taken by Ontario Traffic Inc. (OTI) on Thursday October 28, 2021. (See **Appendix B**) To capture the Weekday A.M. and P.M. Peak Hours, counts were taken from 7:00 A.M. to 10:00 A.M. and from 3:00 P.M. to 6:00 P.M.

For the intersection of Trafalgar Road North at Wellington Road 22, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:30 A.M. and 8:30 A.M. and between 4:15 P.M. and 5:15 P.M., respectively.

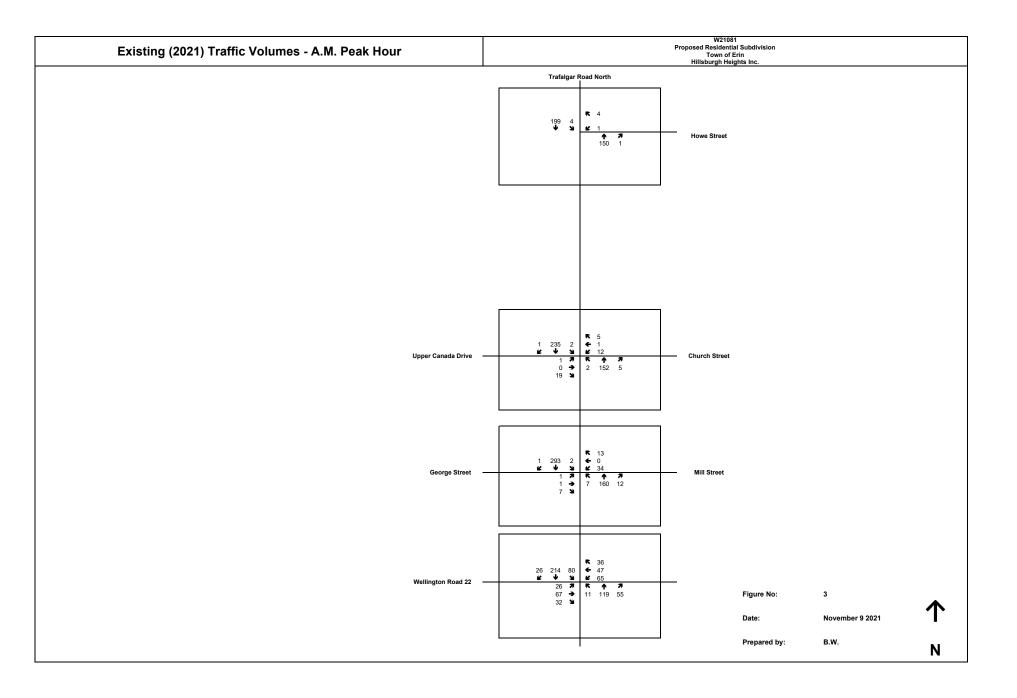
For the intersection of George Street/Mill Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:15 A.M. and 8:15 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

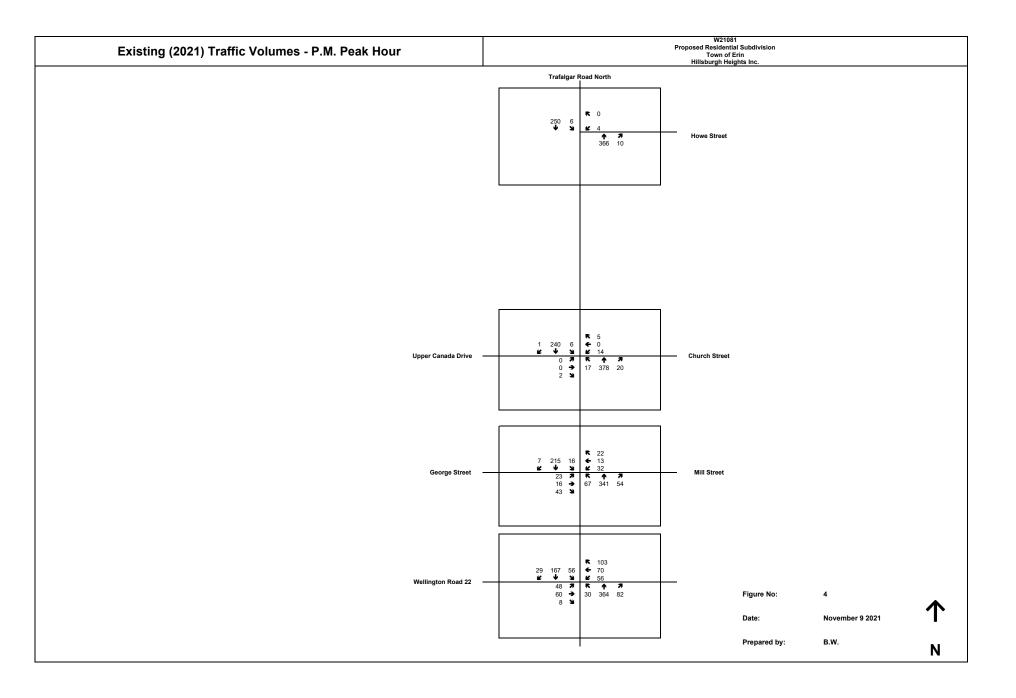
For the intersection of Upper Canada Drive/Church Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:15 A.M. and 8:15 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

For the intersection of Howe Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 8:00 A.M. and 9:00 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

The turning movement counts were conducted during the Covid-19 pandemic. Although all of the services were open and most of the capacity restrictions were lifted when these counts were taken, some offices were still not operating at full capacity and the traffic volumes are anticipated to be less than typical. Based on this assumption, a pandemic projection factor of 20% was used conservatively to project the traffic volumes to typical conditions. To determine the Existing (2021) Traffic Volumes, the projection factor was applied to all of the turning movements.

The Existing (2021) Traffic Volumes are illustrated in Figures 3 and 4





4. EXISTING TRAFFIC CONDITIONS (CONT'D)

4.2 Existing Traffic Analysis

The Existing (2021) peak hour traffic volumes are provided in **Figures 3 and 4** and the Level of Service (LOS) was analyzed using SYNCHRO 9.0 software³.

Trafalgar Road North at Wellington Road 22 was analyzed as a semi-actuated signalized intersection with Trafalgar Road North as the main street. The lane configuration used in the analysis comprises a left and a shared through-right turning lane at the northbound and southbound approaches; and a shared left-through-right turning lane at the eastbound and westbound approaches. The signal timing plans were received from the County of Wellington and are included in **Appendix C**.

George Street/Mill Street at Trafalgar Road North was analyzed as an un-signalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Upper Canada Drive/Church Street at Trafalgar Road North was analyzed as an unsignalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Howe Street at Trafalgar Road North was analyzed as an un-signalized intersection with a stop-control at the westbound approach. The lane configuration used in the analysis comprises a through and a right turning lane at the northbound approach; a shared leftright turning lane at the westbound approach; and a shared through-left turning lane at the southbound approach.

³ Synchro 9 Traffic Signal Optimization and Simulation Modeling Software, Version 9, Trafficware Corporation, 2014.

4. EXISTING TRAFFIC CONDITIONS (CONT'D)

4.2 Existing Traffic Analysis (Cont'd)

The results of the analysis are summarized in **Table 1**. The related calculations are provided in **Appendix E**. The LOS definitions for signalized and un-signalized intersections are included in **Appendix D** for reference.

EXISTING TRAFFIC CONDITIONS (CONT'D) 4.

4.2 Existing Traffic Analysis (Cont'd)

Table 1: Existing (2021)	Turning Lane	10100		Peak Hou	r	P.M. Peak Hour				
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)	
	Overall	0.52	B	13.9	n/a	0.60	B	15.2	n/a	
Trafalgar Road North	EB Approach	0.37	В	20.0	25.5	0.37	С	23.2	26.9	
at	WB Approach	0.52	С	25.2	31.9	0.60	С	23.2	42.9	
	NBL	0.02	А	8.5	3.3	0.06	А	9.1	6.8	
Wellington Road 22	NB TR	0.23	А	7.9	22.2	0.51	В	12.6	70.6	
(Signalized)	SBL	0.17	А	9.7	14.1	0.15	В	10.2	11.6	
	SB TR	0.30	А	9.9	34.5	0.22	А	9.1	27.8	
Trafalgar Road North	Overall	0.09	Α	0.2	n/a	0.24	Α	0.2	n/a	
at	WB Approach	0.01	А	9.8	0.2	0.01	В	13.8	0.2	
Howe Street	NB Approach	0.09	А	0.0	0.0	0.24	А	0.0	0.0	
(Un-signalized)	SB Approach	0.00	А	0.2	0.1	0.01	А	0.3	0.1	
Trafalgar Road North	Overall	0.10	Α	1.5	n/a	0.23	Α	4.3	n/a	
at	EB Approach	0.02	В	10.9	0.4	0.22	С	16.5	6.5	
George Street/	WB Approach	0.10	В	12.8	2.6	0.23	С	20.3	7.1	
Mill Street	NB Approach	0.01	А	0.4	0.2	0.05	А	1.6	1.4	
(Un-signalized)	SB Approach	0.00	А	0.1	0.0	0.01	А	0.7	0.4	
Trafalgar Road North	Overall	0.04	Α	1.0	n/a	0.05	Α	0.8	n/a	
at	EB Approach	0.03	В	10.1	0.8	0.00	А	9.6	0.1	
Upper Canada Drive	WB Approach	0.04	В	12.3	1.1	0.05	В	14.8	1.2	
/Church Street	NB Approach	0.00	Α	0.1	0.0	0.01	Α	0.5	0.3	
(Un-signalized)	SB Approach	0.00	А	0.1	0.0	0.01	А	0.2	0.1	

Table 1: Existing (2021) Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle. Note 2: Signalized intersections are based on existing signal timing plans.

4. EXISTING TRAFFIC CONDITIONS (CONT'D)

4.2 Existing Traffic Analysis (Cont'd)

Trafalgar Road North at Wellington Road 22

The analysis of the Existing (2021) Traffic Conditions indicates that the signalized intersection operates at a Level of Service "B" during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements operate at a Level of Service "C" or better.

Trafalgar Road North at Howe Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service "A" during the A.M. Peak Hour and at a Level of Service "B" or better during the P.M. Peak Hour.

Trafalgar Road North at George Street/Mill Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

4. EXISTING TRAFFIC CONDITIONS (CONT'D)

4.2 Existing Traffic Analysis (Cont'd)

Trafalgar Road North at George Street/Mill Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service "B" or better during the A.M. and P.M. Peak Hours.

5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS

5.1 Other Background Traffic

The Study will consider the site-generated trips from five (5) anticipated developments within the vicinity of Study Area.

An anticipated development owned by Carson Reid Homes Ltd is located immediately south of Station Street and approximately 300 metres west of Trafalgar Road North. The anticipated development will be serviced by the proposed West Collector Road and will comprise of 182 single detached homes.

An anticipated development owned by Thomasfield Homes Ltd is located immediately north of Wellington Road 22 and approximately 450 metres west of Trafalgar Road North. The anticipated development will be serviced by the proposed West Collector Road and will comprise of 210 single detached homes.

An anticipated development owned by Dominion Packers & Realties (Tavares) that comprises 700 single detached homes is located immediately south of Douglas Crescent and east of Trafalgar Road North. It is assumed that the anticipated development will be serviced by the proposed East Collector Road and the Spruce Street roadway, which is connected to Mill Street.

An anticipated development owned by Chantler that comprises 213 single detached homes is located immediately north of Wellington Road 22 and approximately 350 metres east of Trafalgar Road North. It is assumed that the anticipated development will be serviced by the proposed East Collector Road and Wellington Street, which comprises a westerly connection with Trafalgar Road North.

5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS

5.1 Other Background Traffic

A proposed Residential Subdivision owned by the applicant is immediately north of the Subject Subdivision. At the time this report was prepared, details to the proposed Residential Subdivision are preliminary. However, the proposed Residential Subdivision comprises residential land uses at the north end and commercial land uses at the southeast corner of the property. For this Study, it is assumed that the proposed Residential Subdivision will comprise of 44 single detached homes, 23 townhouse units and 60,000 ft² of commercial land use. It is also assumed that access to the residential land uses within the proposed Residential Subdivision will be provided via connection with the elbow for Street 'B', single detached lots fronting the north end of Street 'C' and a future Local Road that aligns with Street 'G' at Street 'A' to form a four-legged roundabout intersection. For the commercial land uses at the southeast corner of the property, it is assumed that access is provided via a full-moves access at Trafalgar Road North, a rightin/right-out access at Street 'A' and a full-moves access at the future Local Road that aligns with Street 'G' at Street 'A' to form a four-legged roundabout intersection. Unlike the four (4) anticipated background developments mentioned above, it is anticipated that the proposed Residential Subdivision will be fully built and occupied between the 2026 and 2031 horizon years. Therefore, site-generated trips from the proposed Residential Subdivision will only be included in the 2031 horizon year. In addition, since access to the anticipated background development is dependent on the construction of the Subject Subdivision, the site-generated trips from the proposed Residential Subdivision will only be included in the Future (2031) Total Traffic scenario.

The locations of the anticipated developments and the future road network are illustrated in **Figure 5**.

NOTE:

THIS STUDY ASSUMES A SPRUCE STREET CONNECTION FOR THE DOMINION PACKERS & REALTIES (TAVARES) DEVELOPMENT AND A WELLINGTON STREET CONNECTION FOR THE CHANTLER DEVELOPMENT.

PROPOSED RESIDENTIAL SUBDIVISION (LANDS OWNED BY APPLICANT)

& REALTIES (TAVARES)

CHANTL

DOMINION PACKERS

DS OWNED BY APPLICANT

THE EXISTING SECTION OF STATION STREET WILL BE REPLACED BY THE PROPOSED WEST COLLECTOR ROAD

> WELLINGTON -STREET (SEE NOTE)

RSON REID HOMES LTD. -

HOMASFIELD HOMES LTD.

PROPOSED WEST COLLECTOR ROAD

TRAFFIC IMPACT STUDY

HILLSBURGH HEIGHTS INC. PROPOSED RESIDENTIAL SUBDIVISION

5916 TRAFALGAR ROAD NORTH PART 1 OF PLAN 61R-9590 PART OF LOT 26, CONCESSION 7 HILLSBURGH URBAN AREA TOWN OF ERIN THE FUTURE ROAD NETWORK AND THE LOCATION OF ANTICIPATED DEVELOPMENTS

9358 GOREWAY DRIVE	CON LIMITED NGINEERS AND PLANNERS BRAMPTON, ONTARIO L6P 0M7
TEL. (905) 794–0600	FAX (905) 794–0611
DRAWN BY:	PROJECT No.
K.F.	W21081
CHECKED BY: B.W.	FIGURE No.
SCALE: N.T.S.	5
DATE: JULY 20th 2022	

5.1.1. Other Background Traffic - Carson Reid Homes Ltd

For the single detached homes (Land Use 210) within the anticipated development, the trip generation formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours⁴.

Table 2 summarizes the trip generation formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

 Table 2: Trip Generation Formulae with Inbound and Outbound Percentages

 – Anticipated Developments

	A.M. Peak	Hour		P.M. Peak Hour			
ITE Land Use	Fitted Curve Equation	% In	% Out	Out Fitted Curve Equation		% Out	
Single-Family Detached Housing (LU 210)	T = 0.71X + 4.80 (Note 1)	25%	75%	Ln(T) = 0.96 Ln(X) + 0.20 (Note 1)	63%	37%	

Note 1: T represents the total number of trips and X represents the number of dwelling units.

The resulting number of trips generated was determined by the trip generation formulae in **Table 2** and the number of dwelling units. The anticipated development comprises 182 single detached homes.

The resulting number of trips generated is provided in **Table 3** for the A.M. and P.M. Peak Hours of adjacent street traffic.

⁴ Trip Generation Manual, 10th Edition, Institute of Transportation Engineers, 2017.

5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

	No. of dwelling units	A.M	I. Peak H	lour	P.M. Peak Hour		
ITE Land Use		Trips In	Trips Out	Total	Trips In	Trips Out	Total
Single-Family Detached Housing	182	34	100	134	114	67	181
(LU 210)							

5.1.1. Other Background Traffic - Carson Reid Homes Ltd (Cont'd)

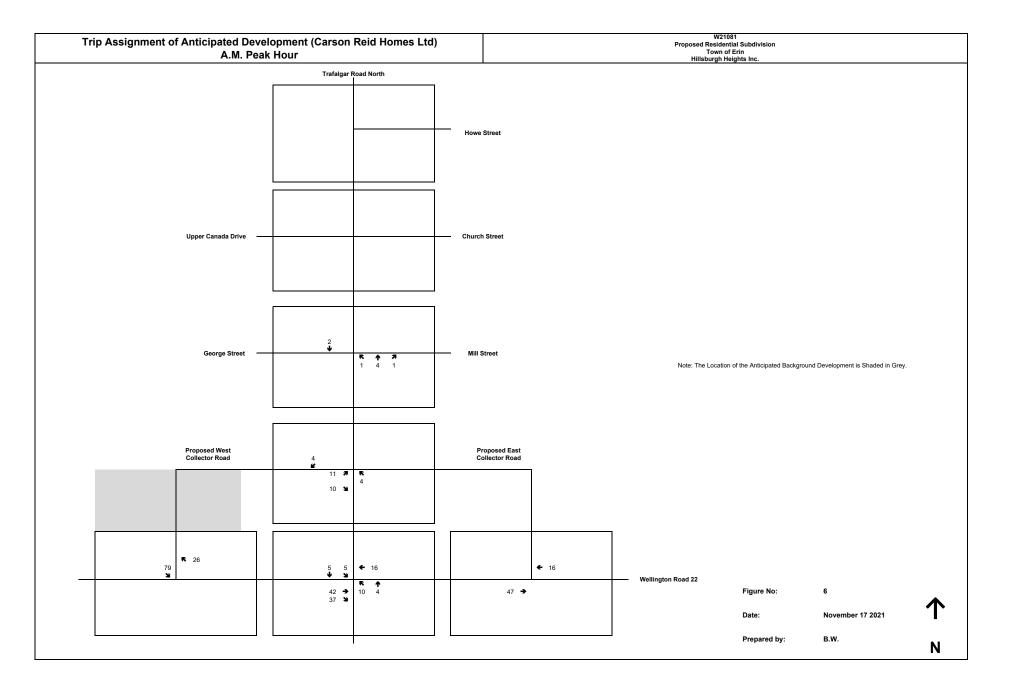
The anticipated development is expected to generate a total of 134 trips during the A.M. Peak Hour (34 inbound trips and 100 outbound trips) and 181 trips during the P.M. Peak Hour (114 inbound trips and 67 outbound trips).

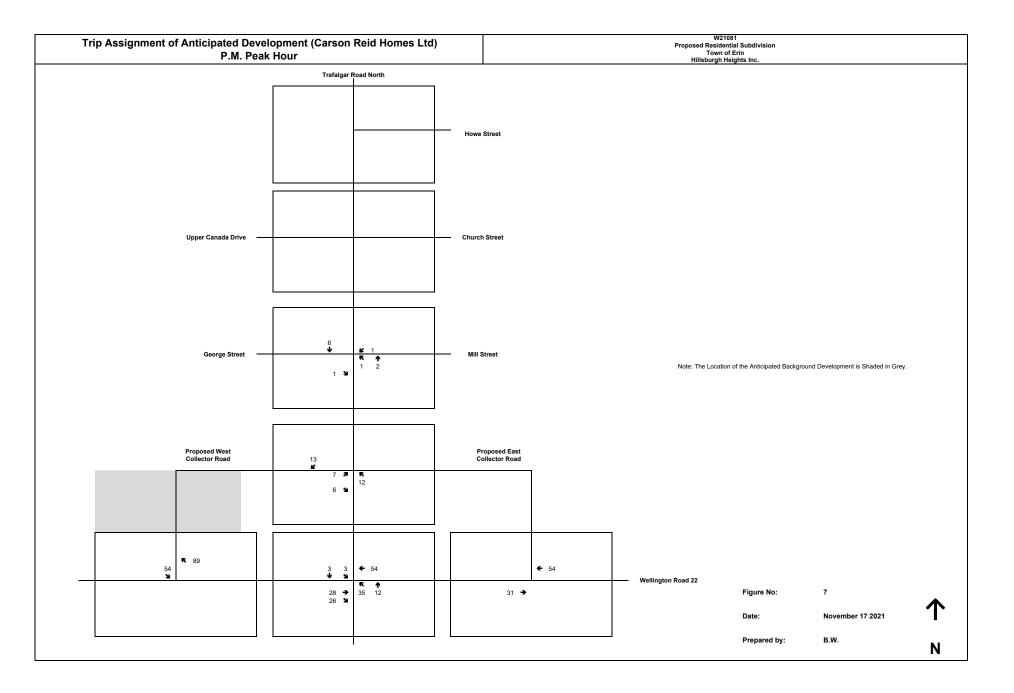
For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in **Appendix F**.

The assumed trip distribution and assignment will be as follows:

- 11% (11%) to/from the north and within the Study Area via Trafalgar Road North,
- 47% (47%) to/from the east via Wellington Road 22,
- 42% (42%) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in **Figures 6 and 7**.





5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

5.1.2. Other Background Traffic - Thomasfield Homes Ltd

The resulting number of trips generated was determined by the trip generation formulae in **Table 2** and the number of dwelling units. The anticipated development comprises 210 single detached homes.

The resulting number of trips generated is provided in **Table 4** for the A.M. and P.M. Peak Hours of adjacent street traffic.

ITE Land Use	No. of dwelling units	A.M. Peak Hour			P.M. Peak Hour		
		Trips In	Trips Out	Total	Trips In	Trips Out	Total
Single-Family Detached Housing	210	39	115	154	130	77	207
(LU 210)							

Table 4: Site-Generated Trips - Thomasfield Homes Ltd

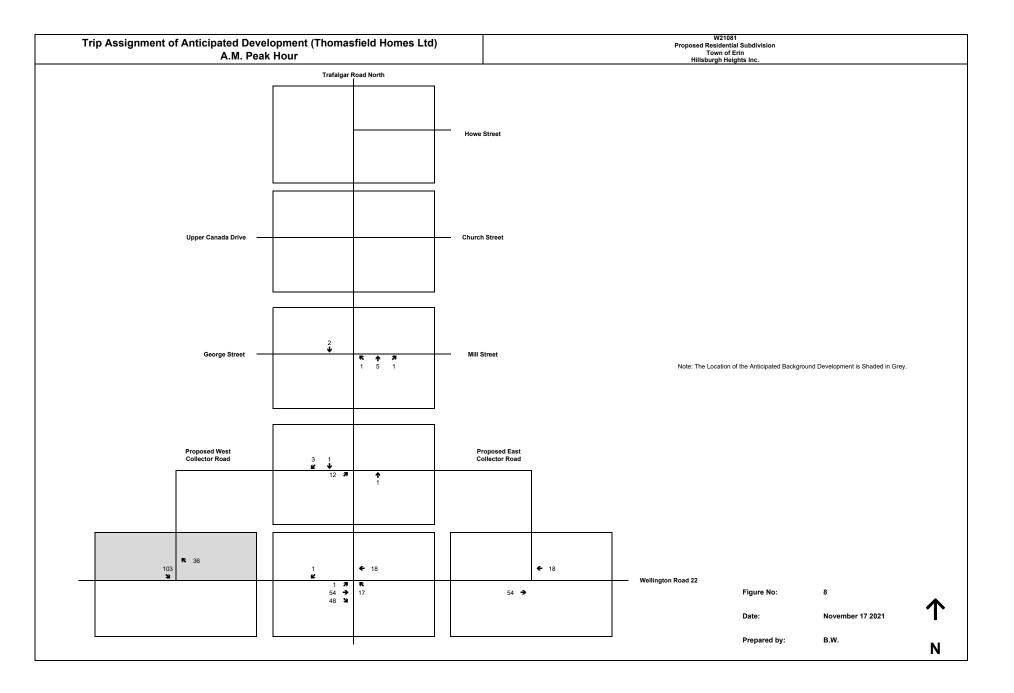
The anticipated development is expected to generate a total of 154 trips during the A.M. Peak Hour (39 inbound trips and 115 outbound trips) and 207 trips during the P.M. Peak Hour (130 inbound trips and 77 outbound trips).

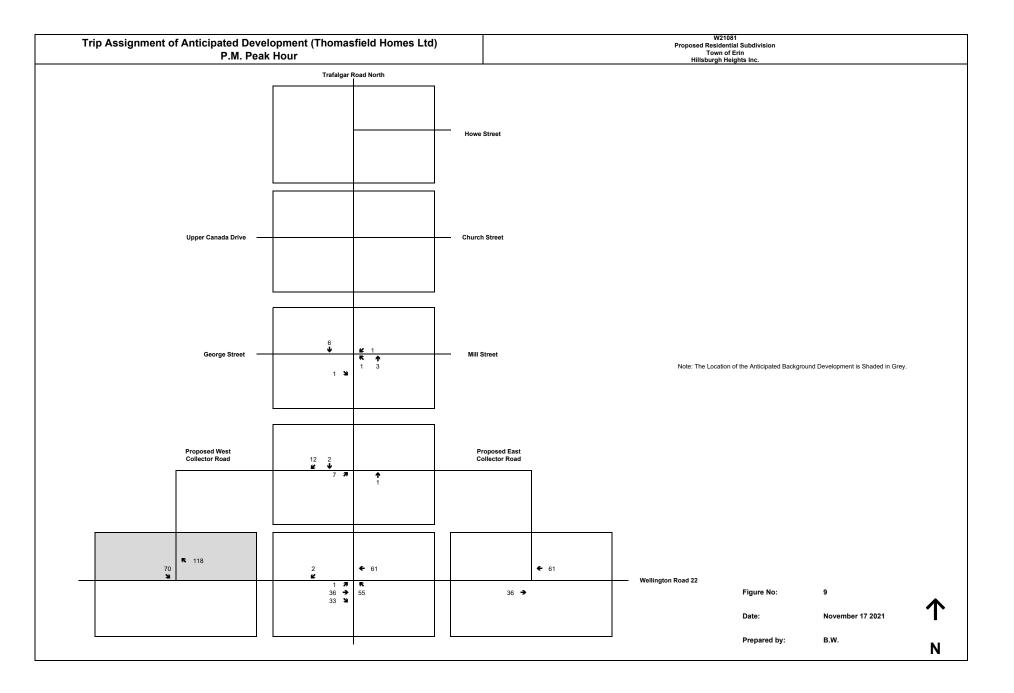
For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- 11% (11%) to/from the north and within the Study Area via Trafalgar Road North,
- 47% (47%) to/from the east via Wellington Road 22,
- 42% (42%) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in **Figures 8 and 9**.





5.1.3. Other Background Traffic - Dominion Packers & Realties (Tavares)

The resulting number of trips generated was determined by the trip generation formulae in **Table 2** and the number of dwelling units. The anticipated development comprises 700 single detached homes.

The resulting number of trips generated is provided in **Table 5** for the A.M. and P.M. Peak Hours of adjacent street traffic.

ITE Land Use	No. of dwelling units	A.M. Peak Hour			P.M. Peak Hour		
		Trips In	Trips Out	Total	Trips In	Trips Out	Total
Single-Family Detached Housing	700	126	376	502	415	243	658
(LU 210)							

Table 5: Site-Generated Trips - Dominion Packers & Realties (Tavares)

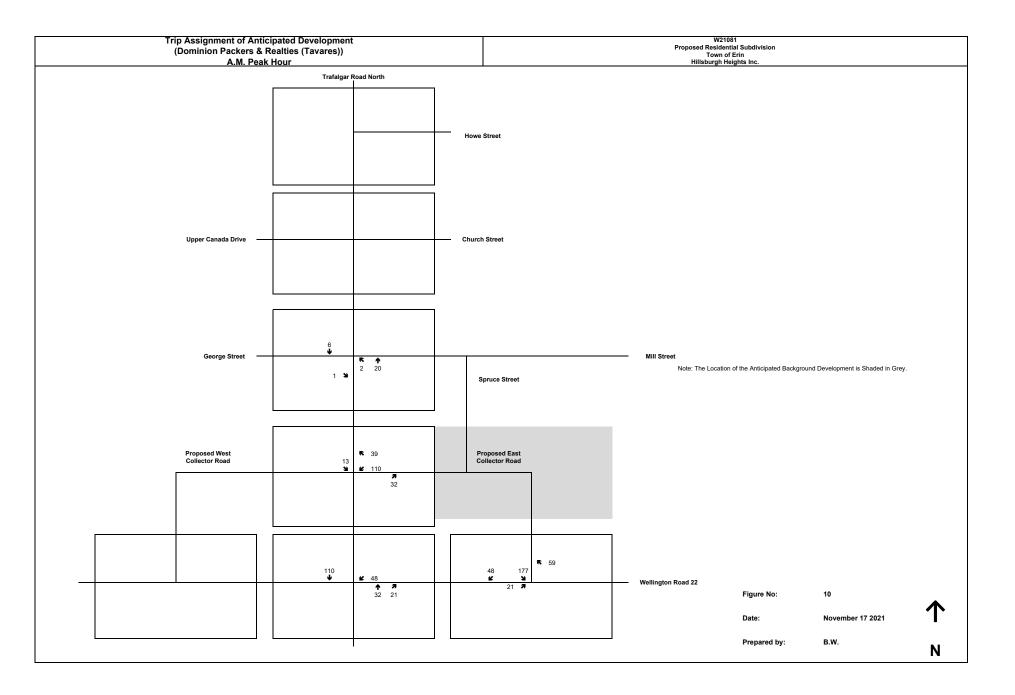
The anticipated development is expected to generate a total of 502 trips during the A.M. Peak Hour (126 inbound trips and 376 outbound trips) and 658 trips during the P.M. Peak Hour (415 inbound trips and 243 outbound trips).

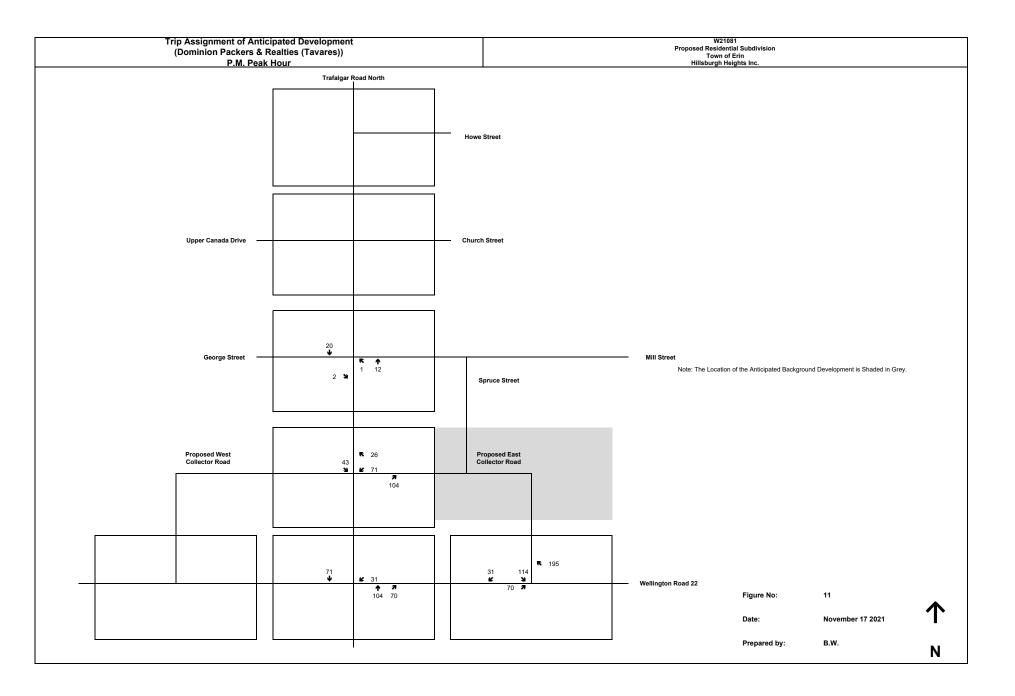
For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- 11% (11%) to/from the north and within the Study Area via Trafalgar Road North,
- 47% (47%) to/from the east via Wellington Road 22,
- 42% (42%) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in **Figures 10 and 11**.





5.1.4. Other Background Traffic - Chantler

The resulting number of trips generated was determined by the trip generation formulae in **Table 2** and the number of dwelling units. The anticipated development comprises 213 single detached homes.

The resulting number of trips generated is provided in **Table 6** for the A.M. and P.M. Peak Hours of adjacent street traffic.

ITE Land Use	No. of dwelling units	A.M	I. Peak H	Iour	P.M. Peak Hour			
		Trips In	Trips Out	Total	Trips In	Trips Out	Total	
Single-Family Detached Housing	213	39	117	156	132	78	210	
(LU 210)								

Table 6: Site-Generated Trips - Chantler

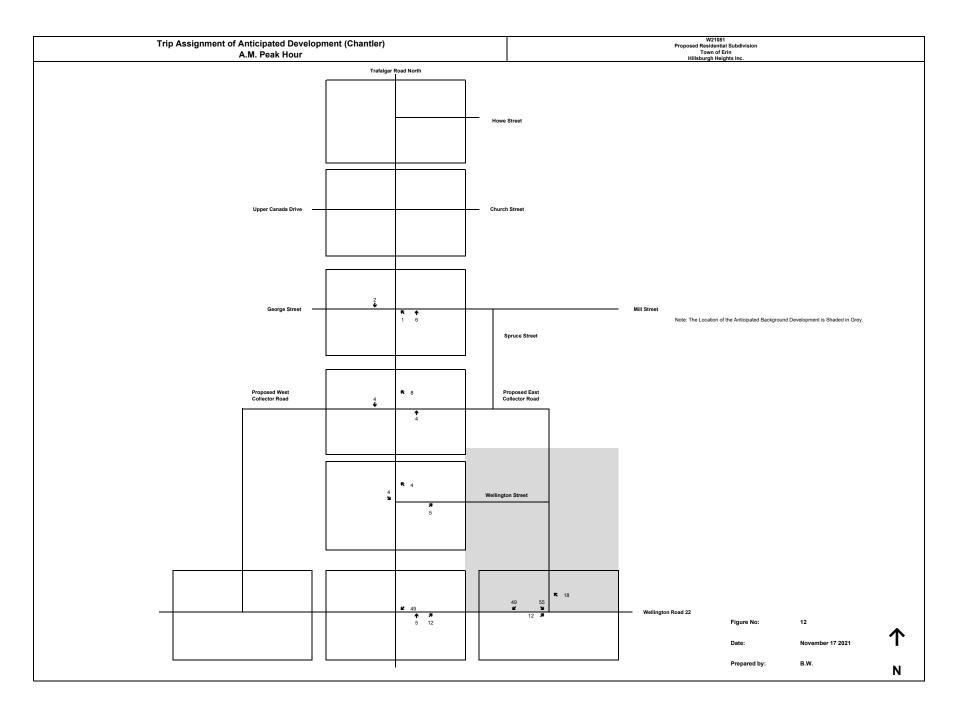
The anticipated development is expected to generate a total of 156 trips during the A.M. Peak Hour (39 inbound trips and 117 outbound trips) and 210 trips during the P.M. Peak Hour (132 inbound trips and 78 outbound trips).

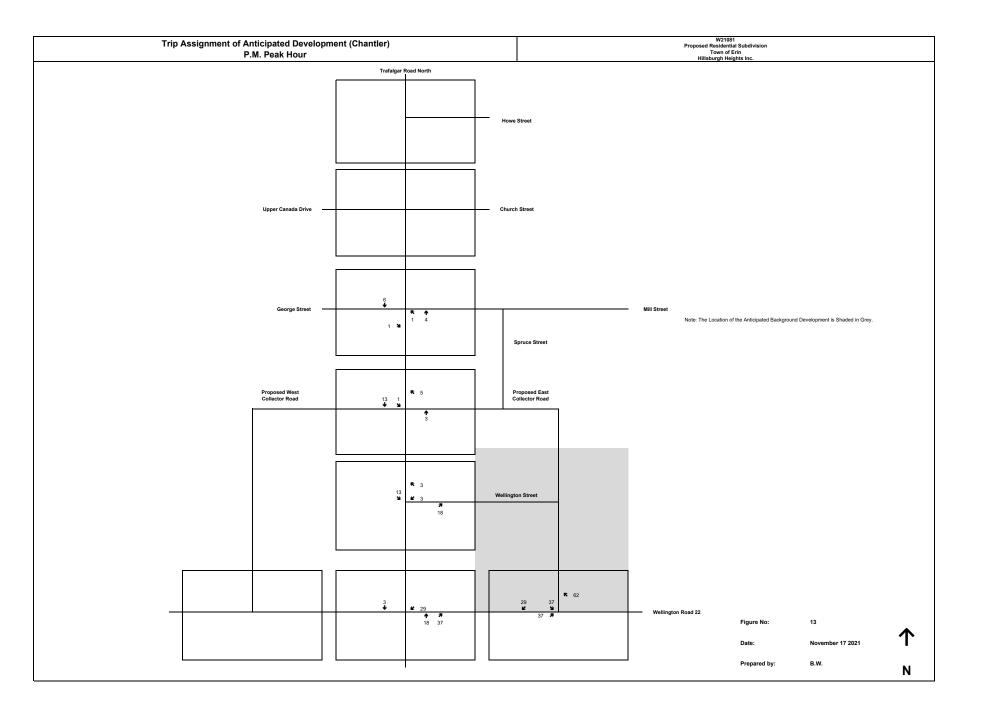
For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- 11% (11%) to/from the north and within the Study Area via Trafalgar Road North,
- 47% (47%) to/from the east via Wellington Road 22,
- 42% (42%) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in **Figures 12 and 13**.





5.1.5. Other Background Traffic – Anticipated Background Development Owned by the Applicant

For the single detached homes (Land Use 210), the townhouse units (Land Use 220) and the commercial land uses (Land Use 820) within the anticipated development, the trip generation formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours.

Table 7 summarizes the trip generation formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

	A.M. Peak Hour			P.M. Peak Hour				
ITE Land Use	Fitted Curve Equation	% In	% Out	Fitted Curve Equation	% In	% Out		
Single-Family Detached Housing (LU 210)	T = 0.71X + 4.80 (Note 1)	25%	75%	Ln(T) = 0.96 Ln(X) + 0.20 (Note 1)	63%	37%		
Multifamily Housing (Low-Rise) (LU 220)	Ln(T) = 0.95 Ln(X) - 0.51 (Note 1)	23%	77%	Ln(T) = 0.89 Ln(X) - 0.02 (Note 1)	63%	37%		
Shopping Centre (LU 820)	T= 0.50X + 151.78 (Note 2)	62%	38%	Ln(T) = 0.74Ln(X) + 2.89 (Note 2)	48%	52%		

 Table 7: Trip Generation Formulae with Inbound and Outbound Percentages

 - Anticipated Background Development Owned by the Applicant

Note 1: T represents the total number of trips and X represents the number of dwelling units. Note 2: T represents the total number of trips and X represents every 1,000 square feet of G.L.A.

> The resulting number of trips generated was determined by the trip generation formulae in **Table 7** and the land uses that were assumed. It is assumed that the anticipated development comprises 44 single detached homes, 23 townhouse units and 60,000 ft² of commercial land use. For the commercial land uses, this Study applied a pass-by percentage of 34% for the P.M. Peak Hour. The pass-by trip percentage was based on the data provided in the ITE Trip GenerationHandbook 3^{rd} Edition⁵.

⁵ Trip Generation Handbook (3rd Edition), Institute of Transportation Engineers, September 2017.

5.1.5. Other Background Traffic – Anticipated Background Development Owned by the Applicant (Cont'd)

The resulting number of trips generated is provided in **Table 8** for the A.M. and P.M. Peak Hours of adjacent street traffic.

	Tips - Anticipate	Trips		I. Peak H		P.M. Peak Hour		
TTE Land Use	ITE Land Use Quantity		Trips In	Trips Out	Total	Trips In	Trips Out	Total
Single-Family Detached Housing (LU 210)	44 Dwelling units	Gross Trips	9	27	36	29	17	46
Multifamily Housing (Low-Rise) (LU 220)	23 Dwelling Units	Gross Trips	3	9	12	10	6	16
Shopping Centre	60,000 ft ²	Gross Trips	113	69	182	179	193	372
(LU 820)	G.L.A.	Passby Net Trips	0 113	0 69	0 182	61 118	61 132	122 250
TOTAL	-	Gross Trips Passby Net Trips	125 0 125	105 0 105	230 0 230	218 61 157	216 61 155	434 122 312

Table 8: Site-Generated Trips - Anticipated Background Development Owned by the Applicant

The anticipated development is expected to generate a total of 230 trips during the A.M. Peak Hour (125 inbound trips and 105 outbound trips) and 434 trips during the P.M. Peak Hour (218 inbound trips and 216 outbound trips).

5.1.5. Other Background Traffic – Anticipated Background Development Owned by the Applicant (Cont'd)

For the single detached homes and the townhouse units, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

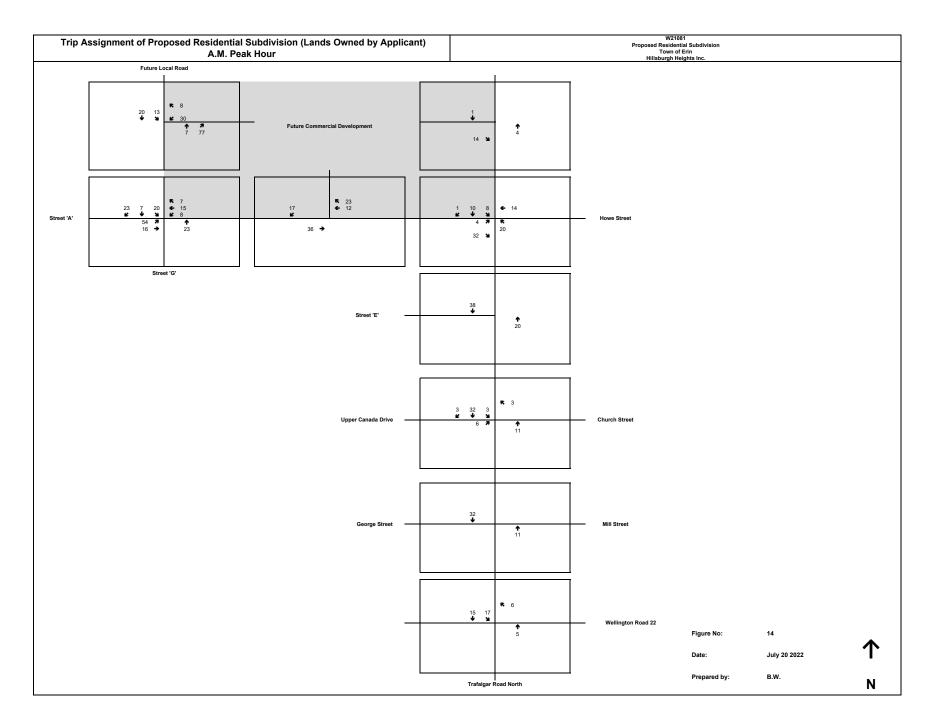
- 47% (47%) to/from the east via Wellington Road 22,
- 11% (11%) to/from the north via Trafalgar Road North and within the Study Area,
- 42% (42%) to/from the south via Trafalgar Road North.

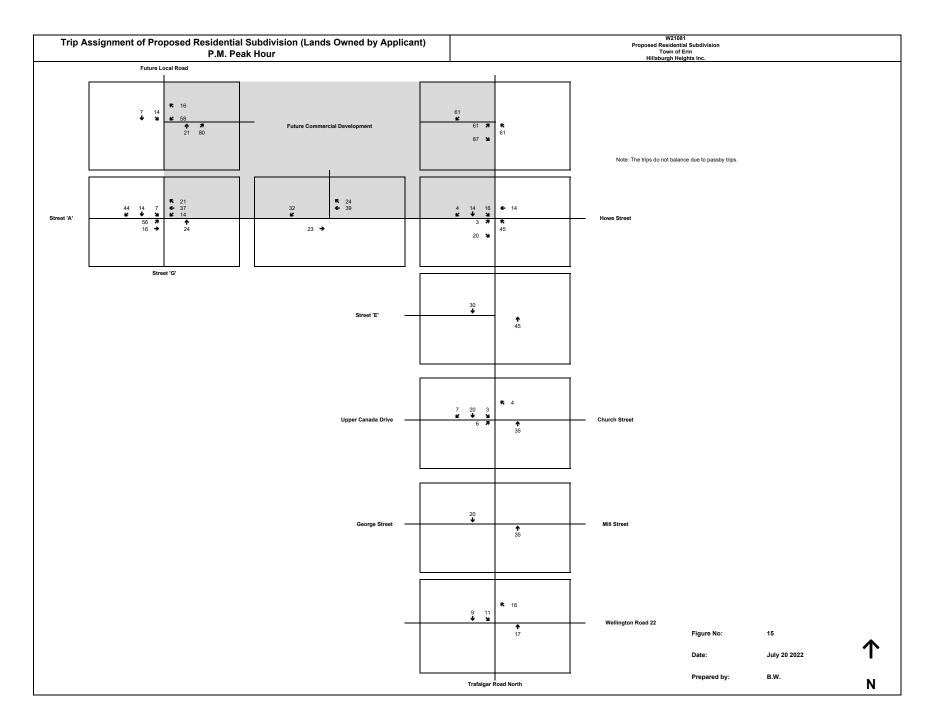
For the commercial land uses, based on the nature of the land uses, the trip distribution and assignment is based on the residential land use within the vicinity of the proposed Residential Subdivision.

The assumed trip distribution and assignment will be as follows:

- 80% (80%) to/from the residential land uses within the Subject Subdivision and the anticipated background development owned by the applicant,
- 12% (12%) to/from the east via Howe Street,
- 5% (5%) to/from the south via Upper Canada Drive and Trafalgar Road North,
- 3% (3%) to/from the south via Church Street.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in **Figures 14 and 15**.





5.2 Traffic Growth Rate

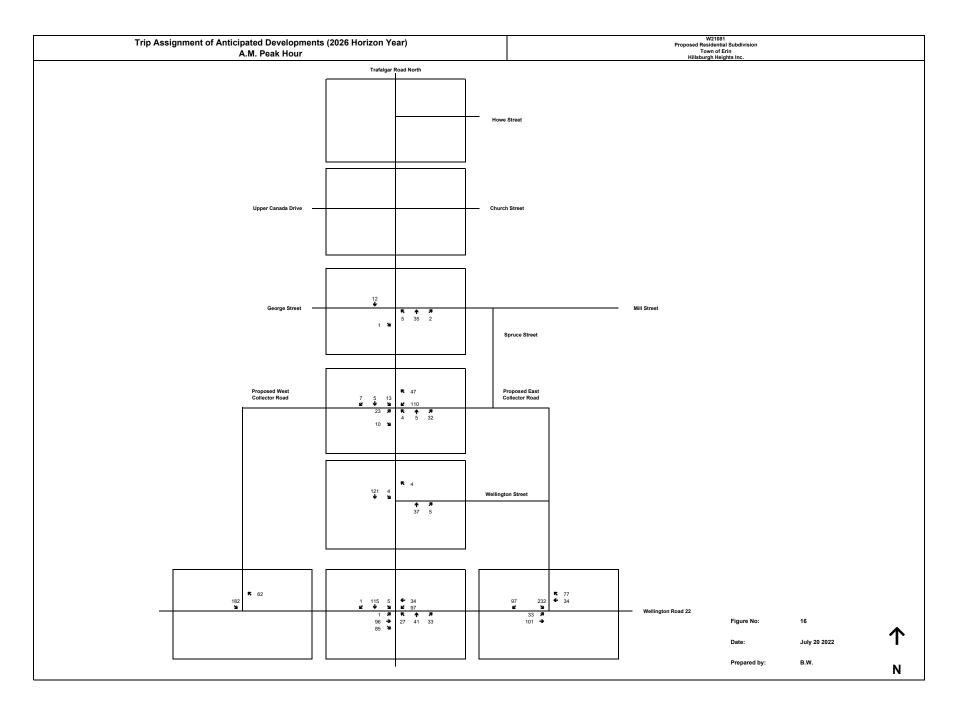
The traffic growth rates for Trafalgar Road North and Wellington Road 22 were obtained from the County of Wellington. An annual growth rate of 2% was considered for these roadways from 2021 to 2031.

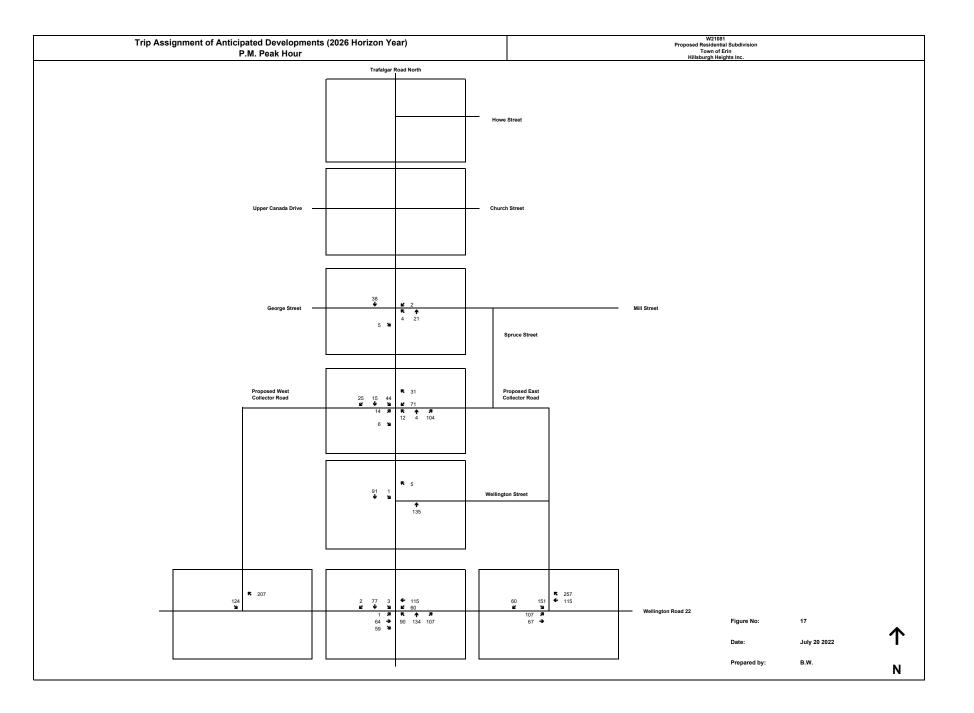
For the George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North and Howe Street at Trafalgar Road North intersections, traffic growth was applied to the through movements on Trafalgar Road North. For the intersection of Trafalgar Road North at Wellington Road 22, an annual growth rate of 2% was applied to all of the turning movements.

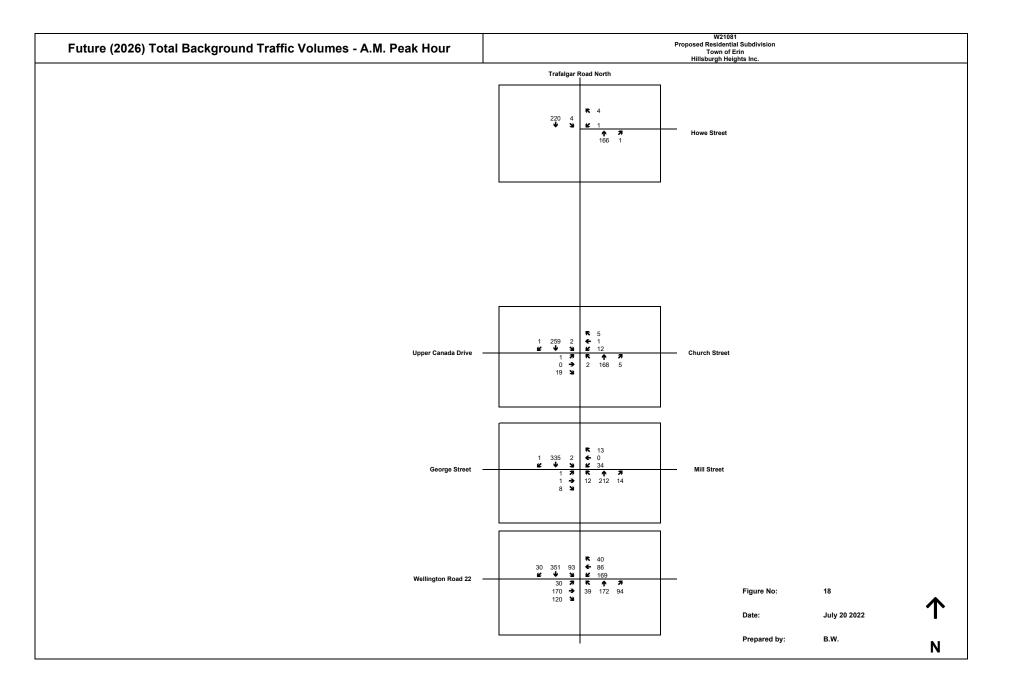
5.3 Future (2026) Total Background Traffic

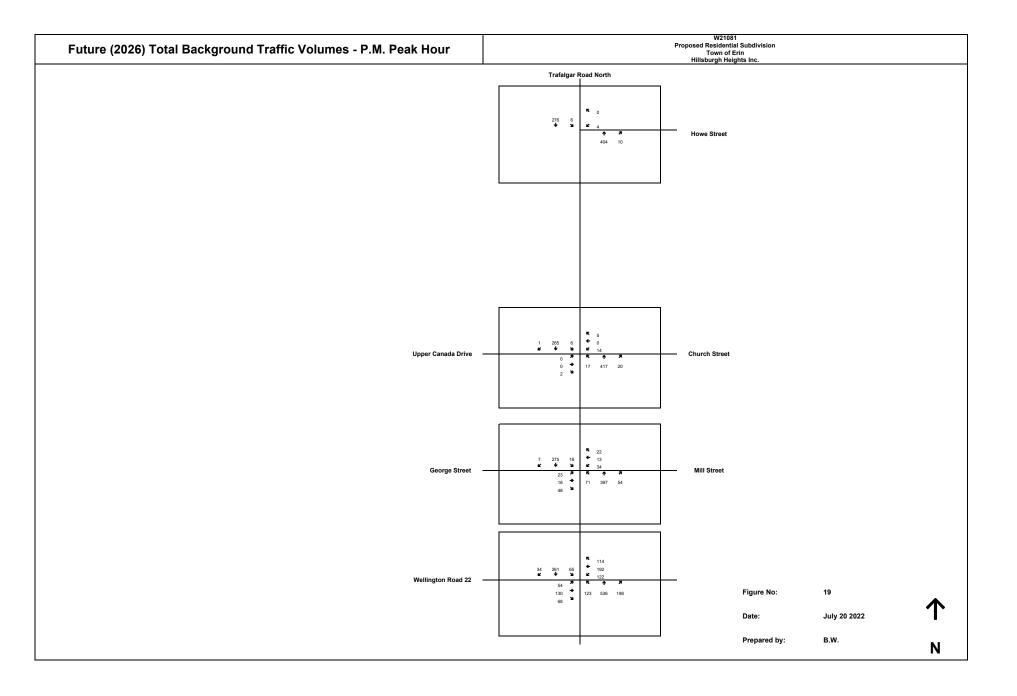
The Future (2026) Total Background Traffic is based on the Existing (2021) Traffic volumes projected with traffic growth for five (5) years for Trafalgar Road North and Wellington Road 22 plus the site-generated trips from the anticipated developments owned by Carson Reid Homes Ltd, Thomasfield Homes Ltd, Dominion Packers & Realties (Tavares) and Chantler. The site-generated trip volumes used in the analysis for the anticipated developments are illustrated in **Figures 16 and 17**.

The Future (2026) Total Background Traffic Volumes are illustrated in **Figures 18 and 19** for the A.M. and P.M. Peak Hours.









5.4 Future (2026) Total Background Traffic Analysis

For the Future (2026) Total Background Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

The signal timing plans and the lane configurations used in the Existing (2021) Traffic Analysis are used in the Future (2026) Total Background Traffic Analysis.

The results of the analysis are summarized in **Table 9**. The related calculations are provided in **Appendix E**.

5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

Table 9: Future (2026)	Turning Lane			Peak Hou		P.M. Peak Hour				
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)	
	Overall	1.02	С	31.9	n/a	0.96	D	37.7	n/a	
Trafalgar Road North	EB Approach	0.61	С	23.3	64.7	0.57	С	23.5	54.3	
at	WB Approach	1.02	F	81.8	100.1	0.93	D	50.6	122.7	
	NBL	0.12	В	13.3	9.6	0.34	В	16.7	26.5	
Wellington Road 22	NB TR	0.42	В	13.9	42.7	0.96	D	45.0	193.5	
(Signalized)	SBL	0.27	В	15.4	20.0	0.76	Е	69.8	33.3	
	SB TR	0.57	В	18.9	71.0	0.38	В	14.9	49.8	
Trafalgar Road North	Overall	0.10	Α	0.2	n/a	0.26	Α	0.2	n/a	
at	WB Approach	0.01	А	9.9	0.2	0.01	В	14.7	0.3	
Howe Street	NB Approach	0.10	Α	0.0	0.0	0.26	А	0.0	0.0	
(Un-signalized)	SB Approach	0.00	А	0.2	0.1	0.01	А	0.2	0.2	
Trafalgar Road North	Overall	0.12	Α	1.5	n/a	0.30	Α	4.6	n/a	
at	EB Approach	0.02	В	11.4	0.5	0.26	С	19.1	8.4	
George Street/	WB Approach	0.12	В	14.4	3.2	0.30	D	25.8	9.6	
Mill Street	NB Approach	0.01	А	0.5	0.3	0.06	А	1.7	1.5	
(Un-signalized)	SB Approach	0.00	А	0.1	0.0	0.02	А	0.6	0.4	
Trafalgar Road North	Overall	0.05	Α	1.0	n/a	0.05	Α	0.8	n/a	
at	EB Approach	0.03	В	10.3	0.8	0.00	А	9.7	0.1	
Upper Canada Drive/	WB Approach	0.05	В	12.8	1.1	0.05	С	15.9	1.4	
Church Street	NB Approach	0.00	А	0.1	0.0	0.01	А	0.4	0.3	
(Un-signalized)	SB Approach	0.00	А	0.1	0.0	0.01	А	0.2	0.1	

Table 9: Future (2026) Total Background Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

Note 2: Signalized intersections are based on existing signal timing plans.

5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

Trafalgar Road North at Wellington Road 22

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "C" during the A.M. Peak Hour and a Level of Service "D" during the P.M. Peak Hour. With the growth in background traffic, impacts to the intersection moderate during the A.M. and P.M. Peak Hours.

During the A.M. Peak Hour, the westbound approach will begin to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 due to the growth in background traffic. All of the other turning movements will continue to operate at a Level of Service "C" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "E" or better during the P.M. Peak Hour.

Trafalgar Road North at Howe Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service "A" during the A.M. Peak Hour and at a Level of Service "B" or better during the P.M. Peak Hour.

5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. Peak Hour and low during the P.M. Peak Hour.

All of the turning movements will continue to operate at a Level of Service "B" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "D" or better during the P.M. Peak Hour.

Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service "B" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "C" or better during the P.M. Peak Hour.

5.5 Future (2031) Total Background Traffic

The Future (2031) Total Background Traffic is based on the Existing (2021) Traffic volumes projected with traffic growth for ten (10) years for Trafalgar Road North and Wellington Road 22 plus the site-generated trips from the anticipated developments owned by Carson Reid Homes Ltd, Thomasfield Homes Ltd, Dominion Packers & Realties (Tavares) and Chantler.

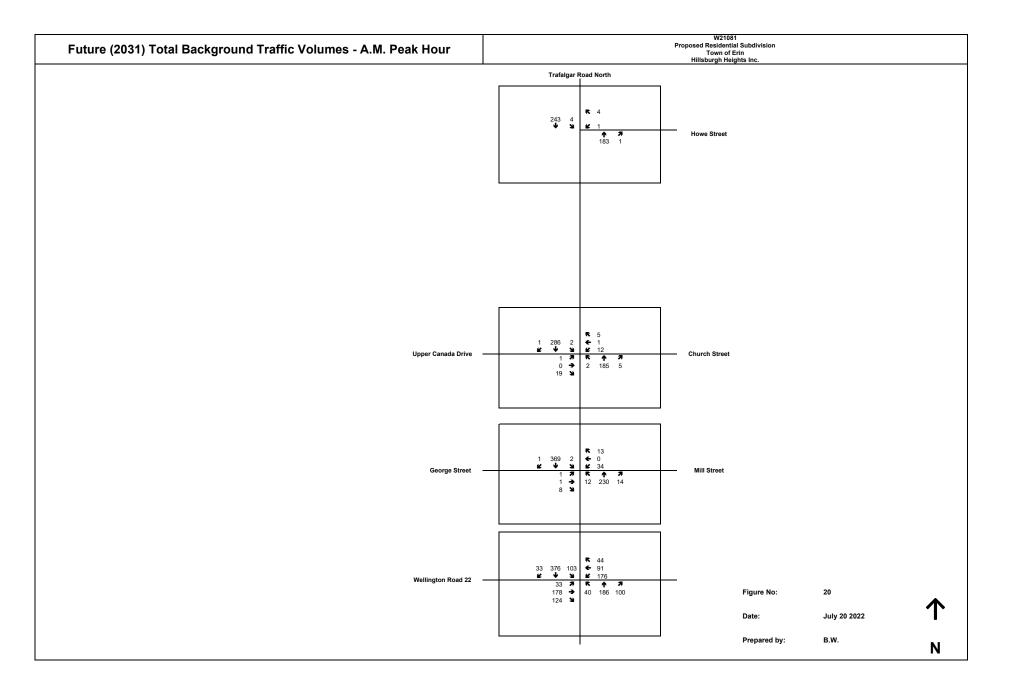
The Future (2031) Total Background Traffic Volumes are illustrated in **Figures 20 and 21** for the A.M. and P.M. Peak Hours.

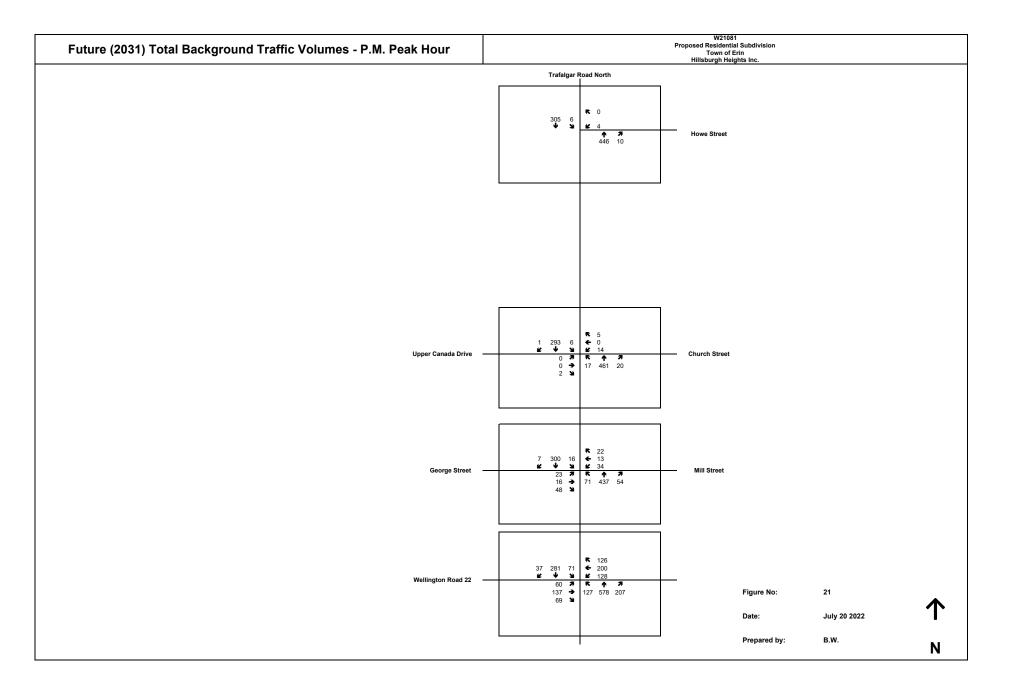
5.6 Future (2031) Total Background Traffic Analysis

For the Future (2031) Total Background Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

The signal timing plans and the lane configurations used in the Future (2026) Total Background Traffic Analysis are used in the Future (2031) Total Background Traffic Analysis.

The results of the analysis are summarized in **Table 10.** The related calculations are provided in **Appendix E**.





5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

	Turning Lane			Peak Hou			P.M. 1	Peak Hou	r
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
	Overall	1.10	D	37.6	n/a	1.04	D	48.5	n/a
Trafalgar Road North	EB Approach	0.64	С	24.5	69.5	0.60	C	25.0	59.3
at	WB Approach	1.10	F	106.0	108.5	0.97	E	59.1	133.9
Wellington Road 22	NBL	0.14	В	13.6	9.9	0.38	В	17.8	28.3
Ũ	NB TR	0.45	В	14.6	46.8	1.04	Е	65.2	213.2
(Signalized)	SBL	0.31	В	16.2	22.3	0.85	F	86.5	36.6
	SB TR	0.62	В	20.0	78.0	0.42	В	15.6	54.4
Trafalgar Road North	Overall	0.11	Α	0.2	n/a	0.29	Α	0.2	n/a
at	WB Approach	0.01	В	10.1	0.2	0.01	С	15.8	0.3
Howe Street	NB Approach	0.11	A	0.0	0.0	0.29	A	0.0	0.0
(Un-signalized)	SB Approach	0.00	А	0.1	0.1	0.01	А	0.2	0.2
Trafalgar Road North	Overall	0.13	Α	1.4	n/a	0.33	Α	4.8	n/a
at	EB Approach	0.02	В	11.8	0.5	0.29	C	20.9	9.4
George Street/	WB Approach	0.13	С	15.3	3.5	0.33	D	29.2	11.1
Mill Street	NB Approach	0.01	A	0.5	0.3	0.06	А	1.7	1.6
(Un-signalized)	SB Approach	0.00	А	0.1	0.0	0.02	А	0.6	0.4
Trafalgar Road North	Overall	0.05	Α	0.9	n/a	0.06	Α	0.8	n/a
at	EB Approach	0.04	В	10.6	0.9	0.00	А	9.9	0.1
Upper Canada Drive/	WB Approach	0.05	В	13.5	1.2	0.06	C	17.2	1.5
Church Street	NB Approach	0.00	А	0.1	0.0	0.01	Α	0.4	0.3
(Un-signalized)	SB Approach	0.00	A	0.1	0.0	0.01	А	0.2	0.1

Table 10: Future (2031) Total Background Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

Note 2: Signalized intersections are based on existing signal timing plans.

5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

Trafalgar Road North at Wellington Road 22

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "D" during the A.M. Peak Hour and will continue to operate at a Level of Service "D" during the P.M. Peak Hour. With the growth in background traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

During the A.M. Peak Hour, the westbound approach will continue to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 due to the growth in background traffic.

In addition, during the P.M. Peak Hour, the shared through-right turning lane at the northbound approach will begin to operate with a volume over capacity ratio that is greater than 1.0 and the southbound left turning movement will begin to operate at a Level of Service "F" due to the growth in background traffic.

Trafalgar Road North at Howe Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. Peak Hour and low during the P.M. Peak Hour.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and will continue to operate at a Level of Service "D" or better during the P.M. Peak Hour.

Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

6. TRIP GENERATION AND DISTRIBUTION

6.1. Trip Generation

For the single detached homes (Land Use 210), townhouse units (Land Use 220) and School Block (Land Use 520), the trip generation rates and formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours.

Based on the pre-consultation comments provided by the Upper Grand District School Board, it is assumed that the proposed School Block within the Subject Subdivision will be an elementary school with a capacity for 450 students.⁶

Table 11 summarizes the trip generation rates and formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

	A.M. Peak	A.M. Peak Hour			P.M. Peak Hour					
ITE Land Use	Fitted Curve Equation	% In	% Out	Fitted Curve Equation	% In	% Out				
	Equation			Equation						
Single-Family										
Detached Housing	T = 0.71X + 4.80	25%	75%	Ln(T) = 0.96 Ln(X) + 0.20	63%	37%				
	(Note 1)	2370	1570	(Note 1)	0370	5170				
(LU 210)										
Multifamily Housing										
(Low-Rise)	Ln(T) = 0.95 Ln(X) - 0.51	23%	77%	Ln(T) = 0.89 Ln(X) - 0.02	63%	37%				
	(Note 1)	2370	1170	(Note 1)	0370	5170				
(LU 220)										
Elementary School	0.67			0.17						
	(Note 2)	54%	46%	(Note 2)	48%	52%				
(LU 520)	(11010-2)			(1000 2)						

 Table 11: Trip Generation Rates and Formulae with Inbound and Outbound Percentages

Note 1: T represents the total number of trips and X represents the number of dwelling units. Note 2: Trip rate is per student.

⁶ Re: Development Pre-Consultation Meeting D'Angelo Property, 5916 Trafalgar Road North, Erin (Hillsburgh), Adam Laranjeiro, July 13, 2021.

6.2 Total Site-Generated Trips

The resulting number of trips generated was determined by the trip generation rates and formulae in **Table 11** and the proposed land uses. It is anticipated that the Subject Property will comprise of 196 single detached homes, 174 townhouse units and an Elementary School with a capacity for 450 students.

The resulting number of trips generated is provided in **Table 12** for the A.M. and P.M. Peak Hours of adjacent street traffic.

		A.M	I. Peak H	Iour	P.M. Peak Hour			
ITE Land Use	Quantity	Trips In	Trips Out	Total	Trips In	Trips Out	Total	
Single-Family Detached Housing (LU 210)	196 dwelling units	36	108	144	122	72	194	
Multifamily Housing (Low-Rise) (LU 220)	174 dwelling units	19	62	81	61	36	97	
Elementary School (LU 520)	450 students	163	139	302	37	40	77	
TOTAL		218	309	527	220	148	368	

 Table 12: Site-Generated Trips

The proposed Residential Subdivision is expected to generate a total of 527 trips during the A.M. Peak Hour (218 inbound trips and 309 outbound trips) and 368 trips during the P.M. Peak Hour (220 inbound trips and 148 outbound trips).

6.3 Trip Distribution and Assignment

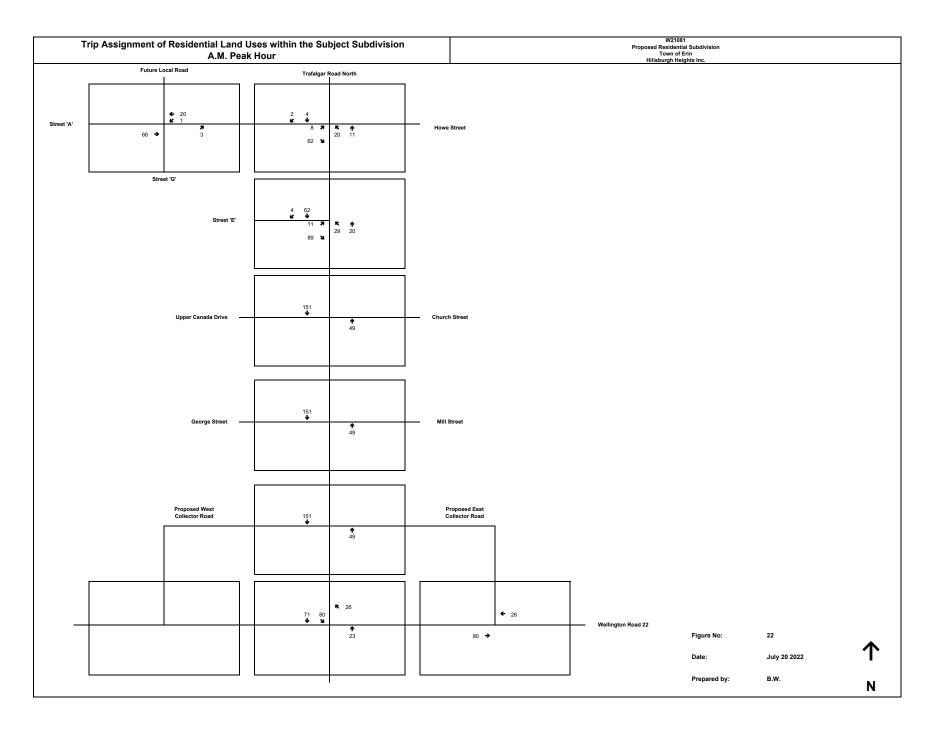
For the single detached homes and the townhouse units, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in **Appendix F**.

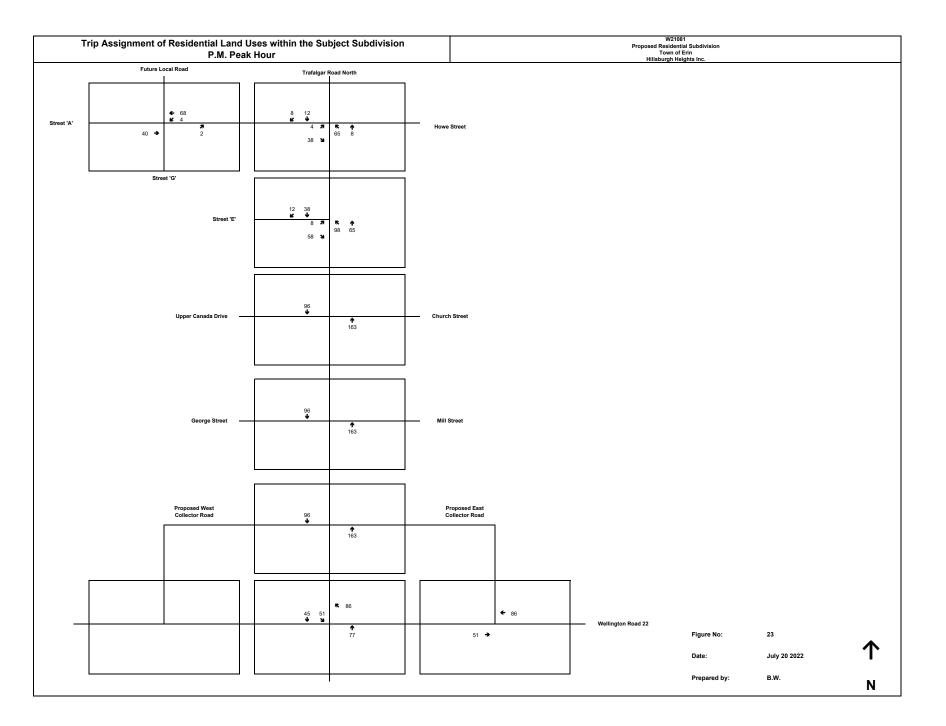
The assumed trip distribution and assignment will be as follows:

- 47% (47%) to/from the east via Wellington Road 22,
- 11% (11%) to/from the north via Trafalgar Road North and within the Study Area,
- 42% (42%) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the single detached homes and the townhouse units are illustrated in **Figures 22 and 23**.

For site-generated trips entering the elementary school during the A.M. Peak Hour and site-generate trips leaving the elementary school during the P.M. Peak Hour, the trip distribution and assignment is based on the future residential land use within an assumed attendance area for the elementary school. The attendance area was determined by using the attendance area for the existing Ross R. MacKay Public Elementary School, which is bounded by Erin-Garafraxa Townline to the north, Winston Churchill Boulevard to the east, Sideroad 17 to the south and Fourth Line to the west. Based on the location of the proposed Elementary School and the existing Ross R. MacKay Public Elementary School, it was assumed that the Attendance Area will be separated evenly and at the midpoint of the two locations.





6.3 Trip Distribution and Assignment (Cont'd)

For site-generated trips leaving the elementary school during the A.M. Peak Hour and site-generate trips entering the elementary school during the P.M. Peak Hour, the assumed trip distribution and assignment is based on the results of the 2016 Transportation Tomorrow Survey as per residential land use within the attendance area and the future road network. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in **Appendix F**. In addition, at the time this Study was prepared, the access locations for the elementary school were not determined. Therefore, this Study assumes that the elementary school will be serviced by a full-moves access along the proposed Street 'A' frontage.

For the 2026 horizon year, the assumed trip distribution and assignment will be as follows:

A.M. Peak Hour

- 47% from within the Subject Subdivision,
- 3% from the east and outside the vicinity of the Study Area via Orangeville Street and Howe Street,
- 41% from the east via Howe Street, Church Street or Mill Street,
- 8% from the south via Upper Canada Drive or George Street,
- 1% from the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

Total 100% inbound

- 4% to the north via Trafalgar Road North,
- 46% to the east via Wellington Road 22,
- 43% to the south via Trafalgar Road North,
- 7% to the south and within the Study Area via Trafalgar Road North.

Total 100% outbound

6.3 Trip Distribution and Assignment (Cont'd)

P.M. Peak Hour

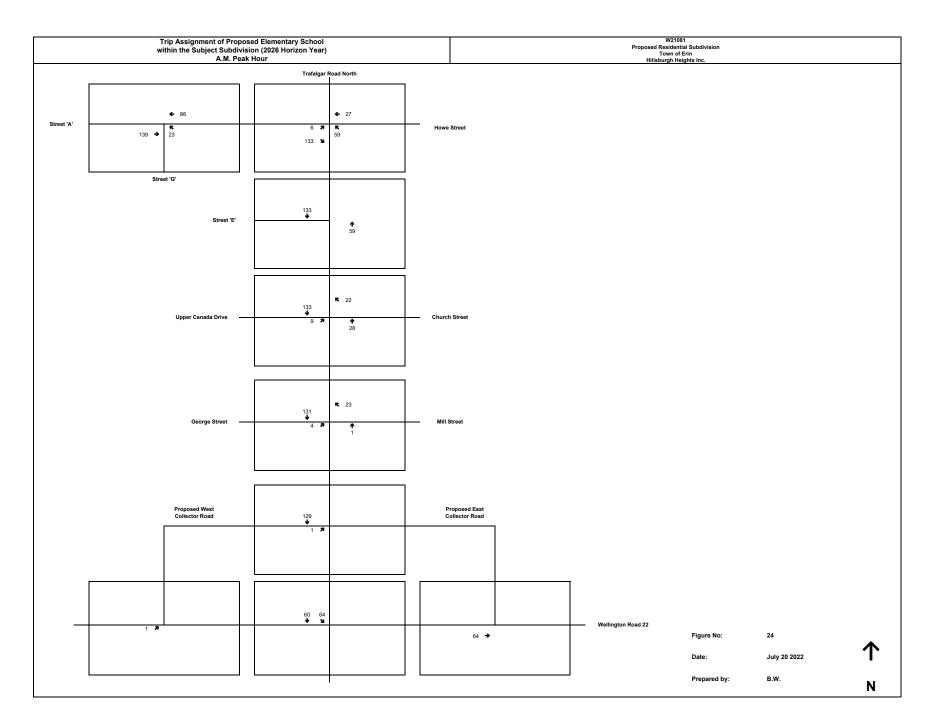
- 4% from the north via Trafalgar Road North,
- 46% from the east via Wellington Road 22,
- 43% from the south via Trafalgar Road North,
- 7% from the south and within the Study Area via Trafalgar Road North.

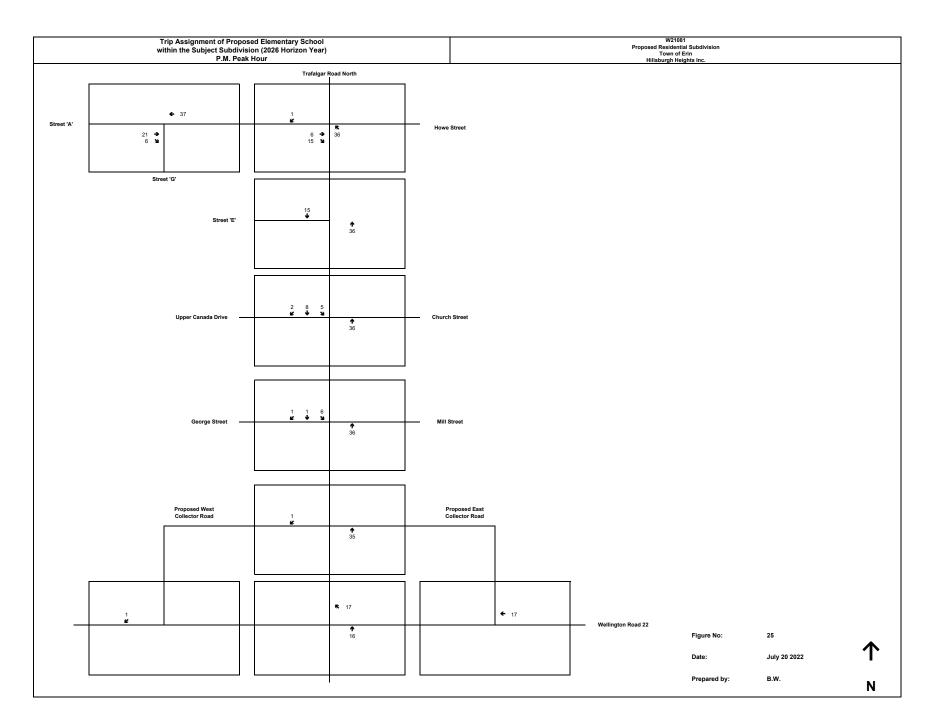
Total 100% inbound

- 47% to within the Subject Subdivision,
- 3% to the east and outside the vicinity of the Study Area via Church Street and Howe Street,
- 41% to the east via Howe Street, Church Street or Mill Street,
- 8% to the south via Upper Canada Drive or George Street,
- 1% to the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

Total 100% outbound

The site-generated trip volumes and trip assignment used in the analysis for the Elementary School during the 2026 horizon year are illustrated in **Figures 24 and 25**.





6.3 Trip Distribution and Assignment (Cont'd)

For the 2031 horizon year, with the occupancy of the anticipated development immediately north of the Subject Subdivision that is owned by the applicant, the assumed trip distribution and assignment will be as follows:

A.M. Peak Hour

- 51% from within the Subject Subdivision and the anticipated development owned by the applicant,
- 3% from the east and outside the vicinity of the Study Area via Orangeville Street and Howe Street,
- 38% from the east via Howe Street, Church Street or Mill Street,
- 7% from the south via Upper Canada Drive or George Street,
- 1% from the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

Total 100% inbound

- 4% to the north via Trafalgar Road North,
- 46% to the east via Wellington Road 22,
- 43% to the south via Trafalgar Road North,
- 7% to the south and within the Study Area via Trafalgar Road North.

Total 100% outbound

6.3 Trip Distribution and Assignment (Cont'd)

P.M. Peak Hour

- 4% from the north via Trafalgar Road North,
- 46% from the east via Wellington Road 22,
- 43% from the south via Trafalgar Road North,
- 7% from the south and within the Study Area via Trafalgar Road North.

Total 100% inbound

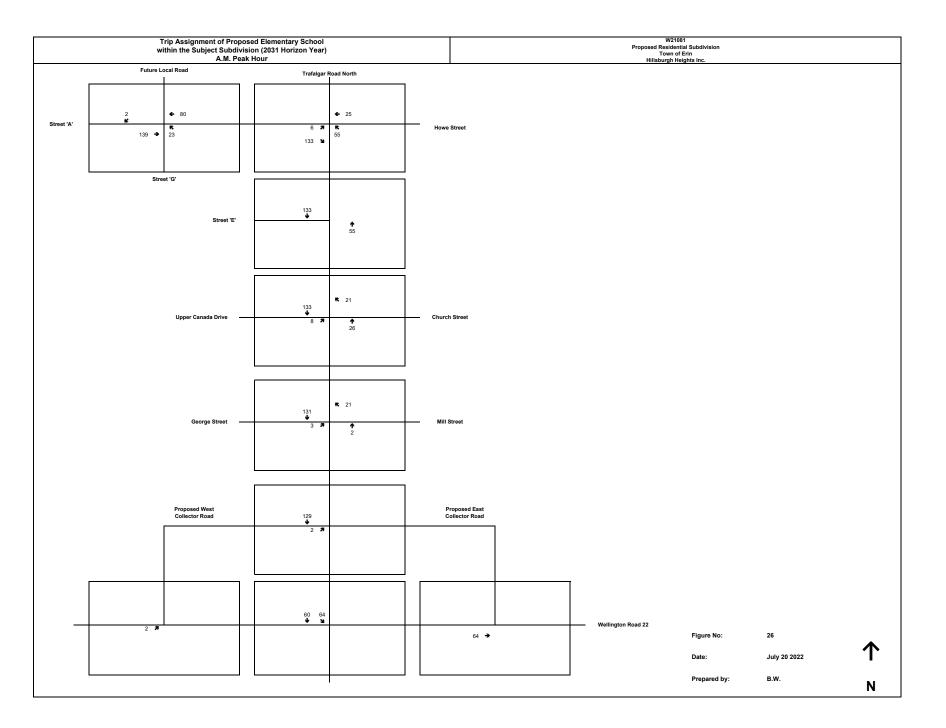
- 51% to within the Subject Subdivision and the anticipated development owned by the applicant,
- 3% to the east and outside the vicinity of the Study Area via Church Street and Howe Street,
- 38% to the east via Howe Street, Church Street or Mill Street,
- 7% to the south via Upper Canada Drive or George Street,
- 1% to the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

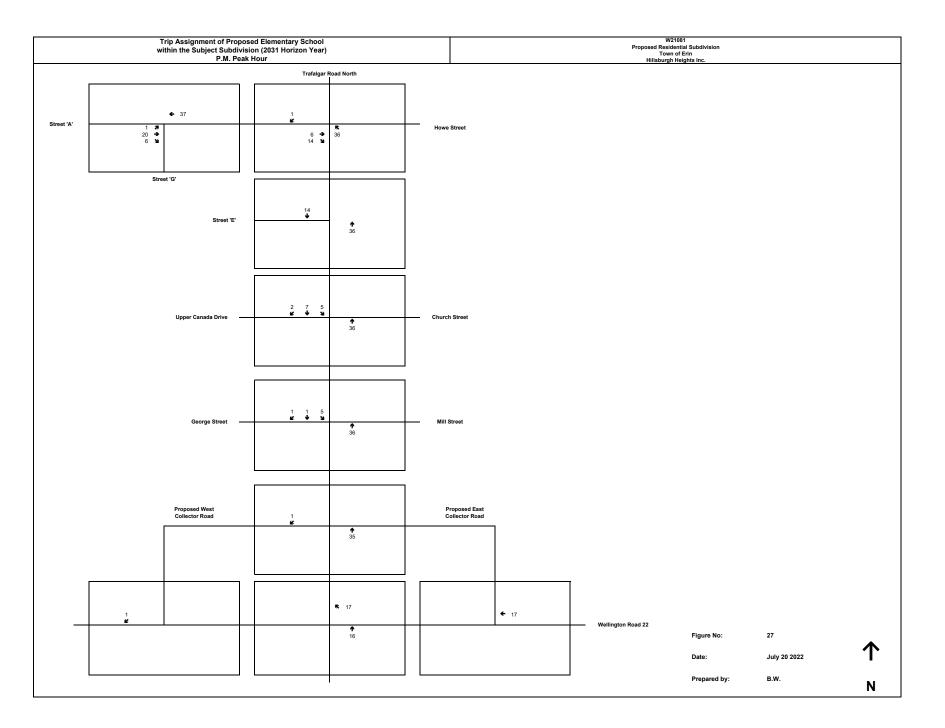
Total 100% outbound

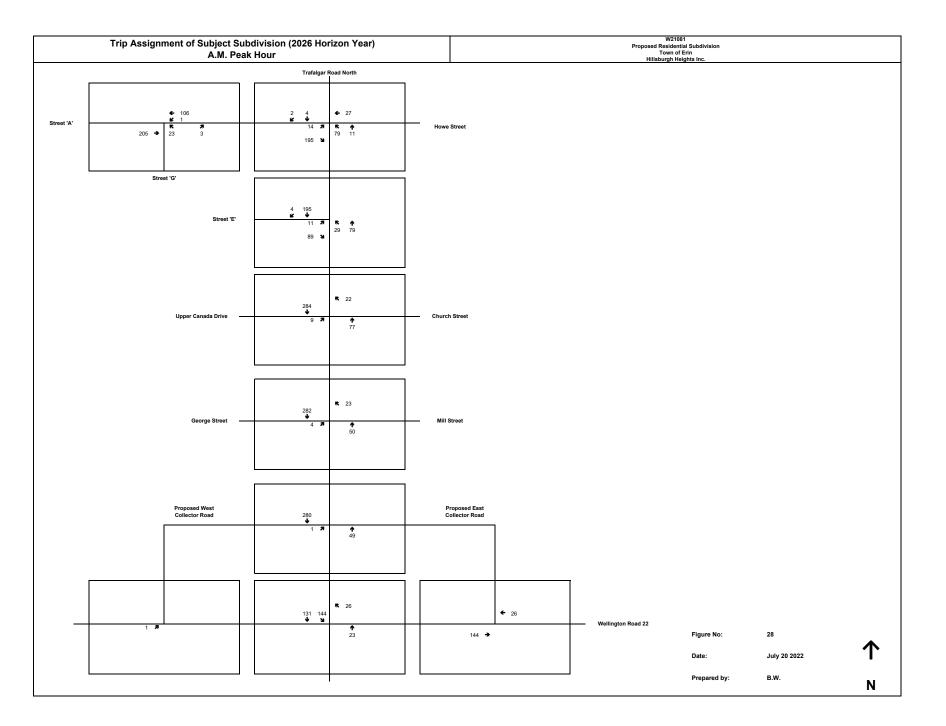
The site-generated trip volumes and trip assignment used in the analysis for the Elementary School during the 2031 horizon year are illustrated in **Figures 26 and 27**.

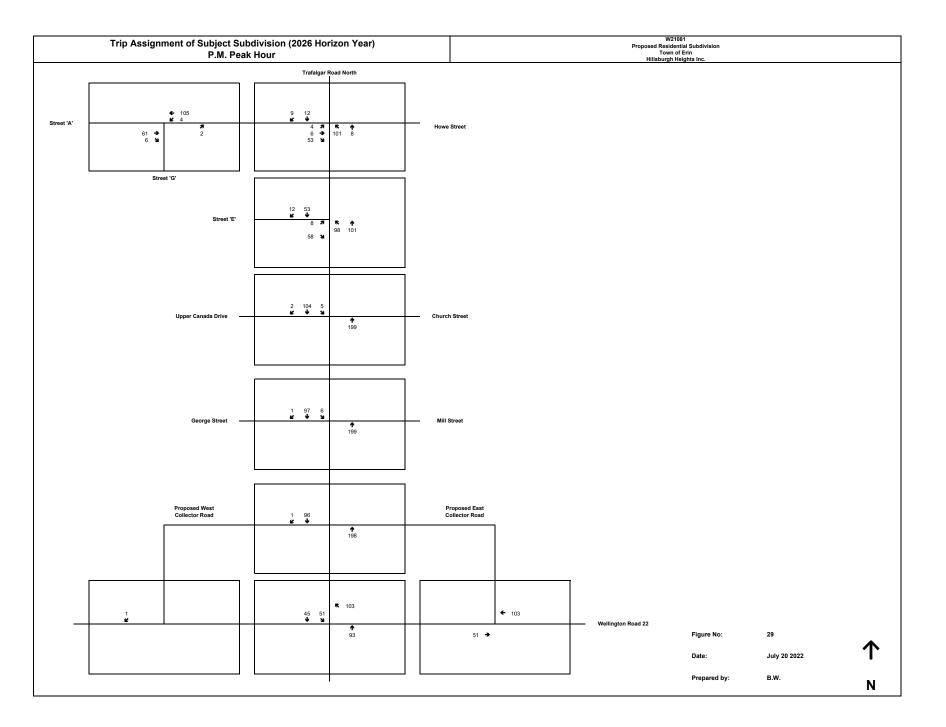
For the 2026 horizon year, the site-generated trip volumes and trip assignment used in the analysis for the proposed Residential Subdivision are illustrated in **Figures 28 and 29**.

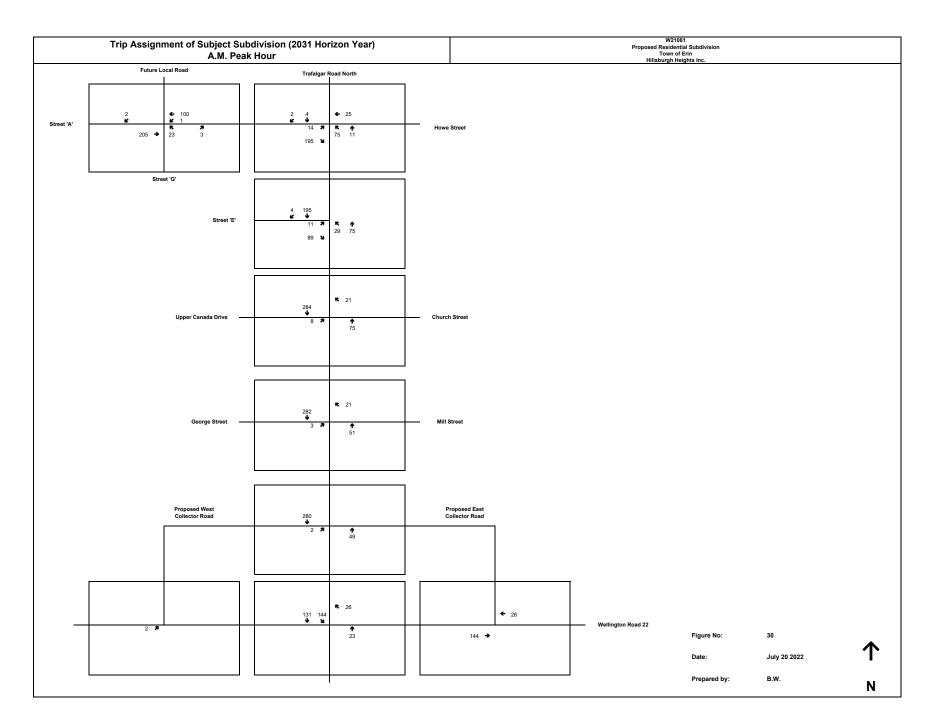
For the 2031 horizon year, the site-generated trip volumes and trip assignment used in the analysis for the proposed Residential Subdivision are illustrated in **Figures 30 and 31**.

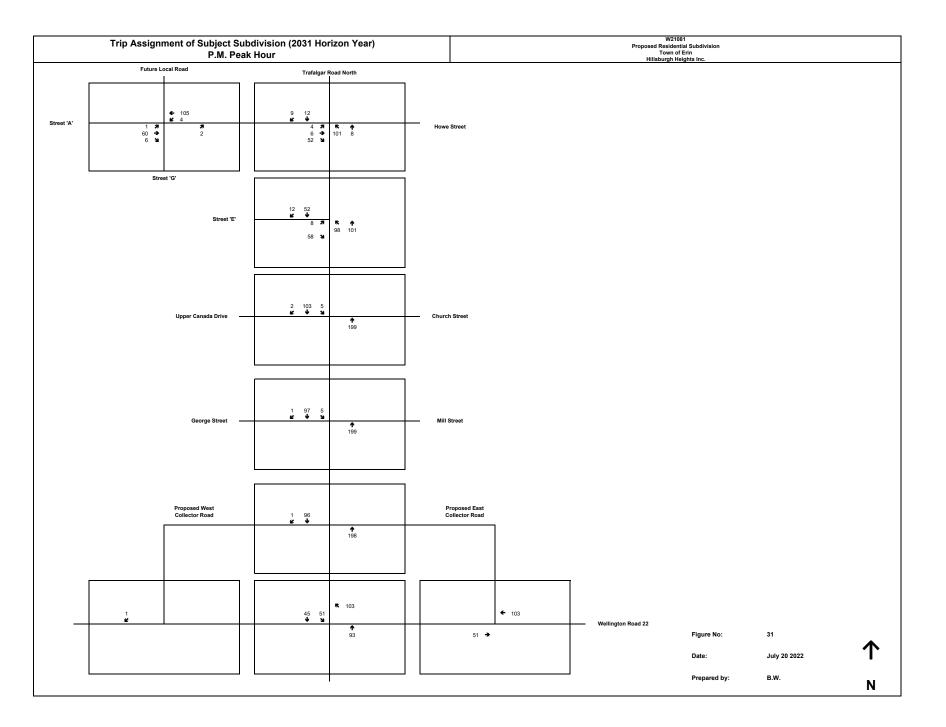












7.1 Future (2026) Total Traffic

The Future (2026) Total Traffic is based on the Future (2026) Total Background Traffic Volumes plus the Site-Generated Traffic Volumes for the Subject Property. The Future (2026) Total Traffic Volumes are provided in **Figures 32 and 33**.

7.2 Future (2026) Total Traffic Analysis

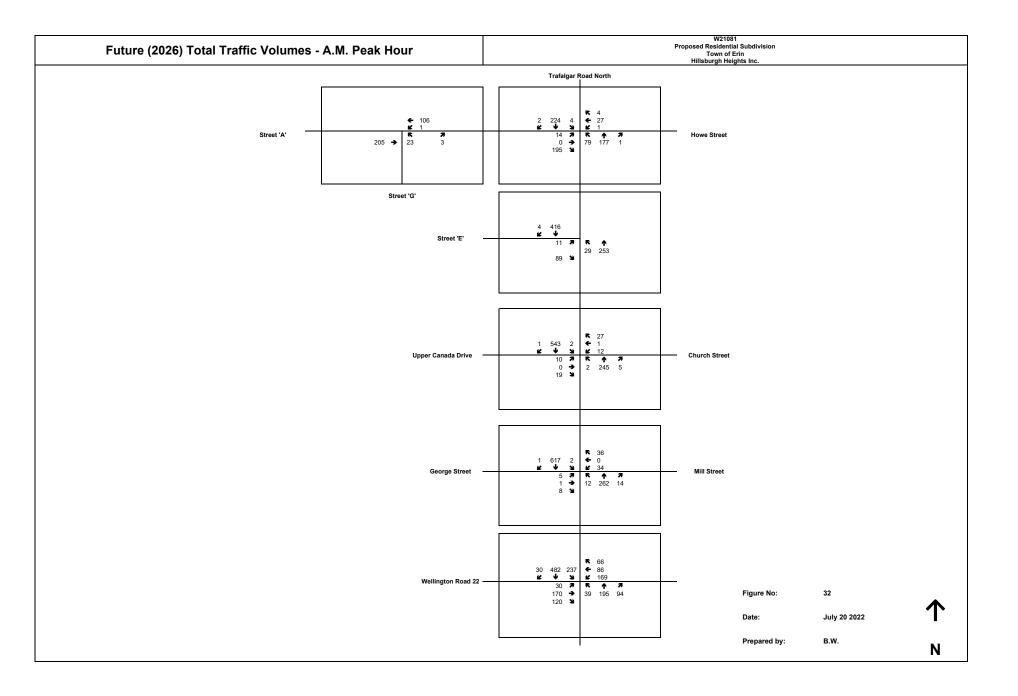
For the Future (2026) Total Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

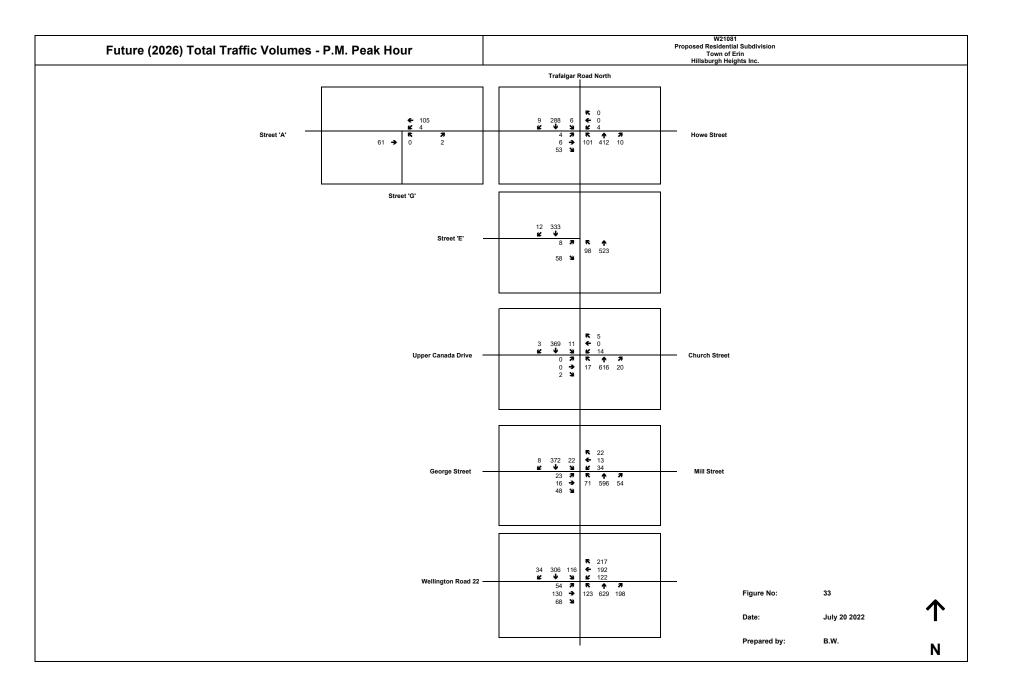
For the Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North and Upper Canada Drive/Church Street at Trafalgar Road North intersections, the signal timing plans and the lane configurations used in the Future (2026) Total Background Traffic Analysis are used in the Future (2026) Total Traffic Analysis.

Proposed Street 'A'/Howe Street at Trafalgar Road North was analyzed as an unsignalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Proposed Street 'E' at Trafalgar Road North was analyzed as an un-signalized intersection with a stop-control at the eastbound approach. The lane configuration used in the analysis comprises a shared through-left turning lane at the northbound approach; a shared left-right turning lane at the eastbound approach; and a shared through-right turning lane at the southbound approach.

Proposed Street 'A' at proposed Street 'G' was analyzed as a single lane roundabout that is yield-controlled at all approaches. The lane configuration used in the analysis comprises a shared left-right turning lane at the northbound approach; a shared throughright turning lane at the eastbound approach; and a shared through-left turning lane at the westbound approach.





7.2 Future (2026) Total Traffic Analysis (Cont'd)

The results of the analysis are summarized in **Table 13**. The related calculations are provided in **Appendix E**.

Table 13: Future (2020)	Turning Lane	20101		Peak Hou	r		P.M. I	Peak Hou	r
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
	Overall	1.07	D	37.0	n/a	1.38	Ε	72.0	n/a
Trafalgar Road North	EB Approach	0.61	C	23.3	64.7	0.59	С	24.5	56.0
at	WB Approach	1.07	F	95.1	108.6	1.07	F	84.6	158.0
Wellington Road 22	NBL	0.19	В	15.1	10.4	0.39	В	18.0	27.8
Ũ	NB TR	0.45	В	15.0	47.9	1.10	F	84.3	230.1
(Signalized)	SBL	0.72	С	30.8	68.1	1.38	F	250.8	48.4
	SB TR	0.77	C	26.0	107.6	0.45	В	16.2	58.8
Trafalgar Road North	Overall	0.29	Α	5.0	n/a	0.13	Α	2.5	n/a
at	EB Approach	0.29	В	11.7	9.5	0.13	В	12.9	3.6
Howe Street/	WB Approach	0.08	В	14.7	2.1	0.02	D	27.0	0.6
Proposed Street 'A'	NB Approach	0.06	А	2.8	1.5	0.09	А	2.4	2.3
(Un-signalized)	SB Approach	0.00	А	0.2	0.1	0.01	А	0.2	0.2
Trafalgar Road North	Overall	0.23	Α	1.8	n/a	0.50	Α	6.2	n/a
at	EB Approach	0.05	С	17.7	1.3	0.41	D	32.0	15.1
George Street/	WB Approach	0.23	С	19.2	7.0	0.50	F	52.3	19.1
Mill Street	NB Approach	0.01	А	0.5	0.3	0.07	А	1.7	1.7
(Un-signalized)	SB Approach	0.00	А	0.0	0.0	0.03	А	0.8	0.6
Trafalgar Road North	Overall	0.12	Α	1.3	n/a	0.09	Α	0.8	n/a
at	EB Approach	0.11	C	17.5	2.9	0.00	В	10.4	0.1
Upper Canada Drive/	WB Approach	0.12	C	15.3	3.3	0.09	C	23.2	2.3
Church Street	NB Approach	0.00	Α	0.1	0.1	0.02	А	0.4	0.4
(Un-signalized)	SB Approach	0.00	А	0.0	0.0	0.01	А	0.4	0.3

 Table 13: Future (2026) Total Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

Note 2: Signalized intersections are based on existing signal timing plans.

7.2 Future (2026) Total Traffic Analysis (Cont'd)

Table 15. Future (2020)										
	A.M. Peak Hour					P.M. Peak Hour				
Intersection	Approach				95th				95th	
		V/C	LOS	Delay ¹	Queue (m)	V/C	LOS	Delay ¹	Queue (m)	
Trafalgar Road North	Overall	0.27	Α	2.0	n/a	0.22	Α	2.2	n/a	
at	EB Approach	0.19	В	13.0	5.7	0.14	В	13.0	3.8	
Proposed Street 'E'	NB Approach	0.03	A	1.1	0.7	0.09	А	2.3	2.4	
(Un-signalized)	SB Approach	0.27	А	0.0	0.0	0.22	А	0.0	0.0	
Street 'A'	Overall	0.20	A	4.8	n/a	0.11	Α	4.0	n/a	
at	EB Approach	0.20	A	5.1	7.0	0.06	A	3.8	0.0	
Proposed Street 'G'	WB Approach	0.11	А	4.3	0.0	0.11	А	4.2	0.0	
(Roundabout)	NB Approach	0.03	А	4.4	0.0	0.00	А	3.4	0.0	

 Table 13: Future (2026) Total Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

7.2 Future (2026) Total Traffic Analysis (Cont'd)

Trafalgar Road North at Wellington Road 22

The analysis of the Future (2026) Total Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "D" during the A.M. Peak Hour and at a Level of Service "E" during the P.M. Peak Hour. With the inclusion of site-generated trips, impacts to the intersection are low during the A.M. Peak Hour and high during P.M. Peak Hour.

The westbound approach will continue to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 during the A.M. Peak Hour and will begin to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 during the P.M. Peak Hour.

During the P.M. Peak Hour, the shared through-right turning lane at the northbound approach and the left turning lane at the southbound approach will begin to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0. In addition, during the A.M. and P.M. Peak Hours, the queue lengths at the southbound left turning lane may begin to result in a spillback of vehicles into the adjacent lane.

Trafalgar Road North at Howe Street/proposed Street 'A'

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are moderate during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "D" or better during the P.M. Peak Hour.

7.2 Future (2026) Total Traffic Analysis (Cont'd)

Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are low during the A.M. Peak Hour and moderate during P.M. Peak Hour.

During the P.M. Peak Hour, the westbound approach will begin to operate at a Level of Service "F" with an average delay of 52.3 seconds per vehicle. All of the other turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and continue to operate at a Level of Service "C" or better during the P.M. Peak Hour.

Trafalgar Road North at proposed Street 'E'

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will operate at a Level of Service "A" during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements will operate at a Level of Service "B" or better.

7.2 Future (2026) Total Traffic Analysis (Cont'd)

Proposed Street 'A' at proposed Street 'B'/proposed Street 'G'

The analysis of the Future (2026) Total Traffic Conditions indicates that all of the approaches at the roundabout will operate at a Level of Service "A" during the A.M. and P.M. Peak Hours.

7.2.1 Future (2026) Total Traffic Analysis – Right-Turn Lane Warrant Analysis

For the southbound right turning movements at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a right-turn lane warrant analysis was conducted using the principles provided in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways⁷. Based on the procedure to the right-turn lane warrant analysis, a right-turning lane should be considered when traffic volumes are 60 vehicles per hour or higher. With the southbound right turning movements at the concerned intersections operating with 12 vehicles per vehicle or less, right turning lanes are not warranted.

7.2.2 Future (2026) Total Traffic Analysis – Left-Turn Lane Warrant Analysis

For the northbound left turning movement at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a left-turn lane warrant analysis was undertaken. The analysis followed the procedure specified in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways.

At approximately 200 metres north of the proposed Residential Subdivision, travelling in the southbound direction, the posted speed limit on Trafalgar Road North changes from 60 km/h to 40 km/h. Therefore, a design speed of 70 km/h was assumed.

⁷ Geometric Design Standards for Ontario Highways, Ministry of Transportation Ontario.

7.2.2 Future (2026) Total Traffic Analysis – Left-Turn Lane Warrant Analysis

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, the analysis determined that a left-turning lane at the northbound approach is warranted during the P.M. Peak Hour.

For the Trafalgar Road North at Howe Street/proposed Street 'A' intersection, in order to match the construction of a northbound left-turning lane, a southbound left-turning lane will be recommended to create a balanced intersection.

The results of the analysis are provided in Figures 34 and 35.

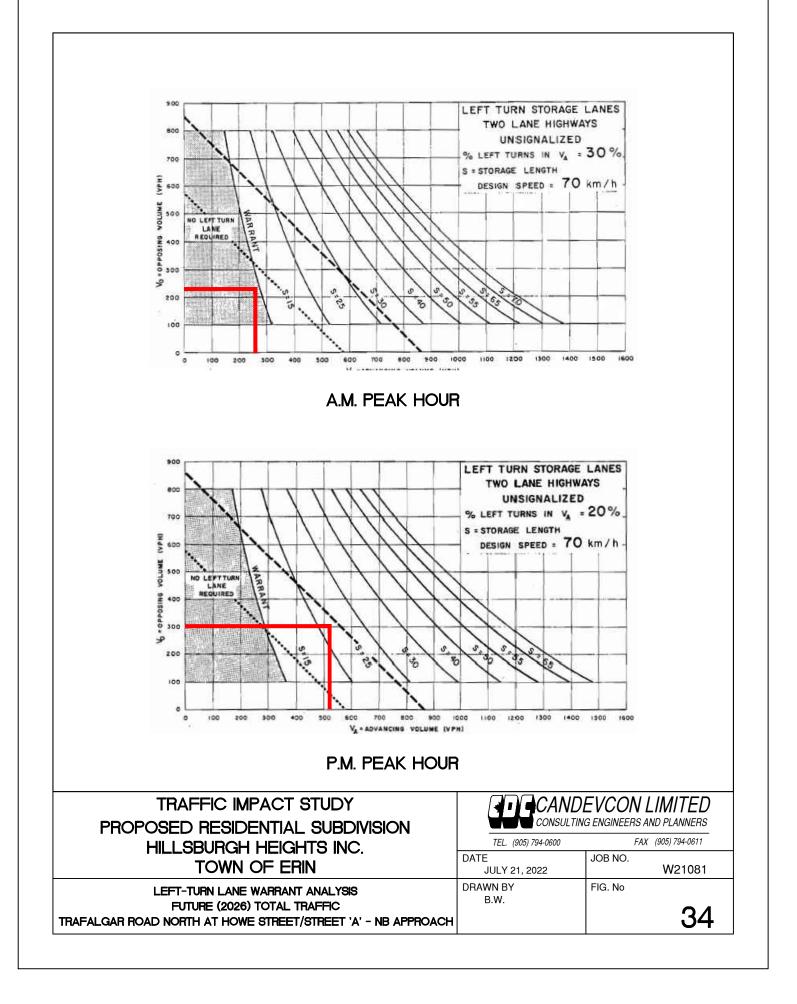
7.2.3 Future (2026) Total Traffic Analysis – Signal Warrant Analysis

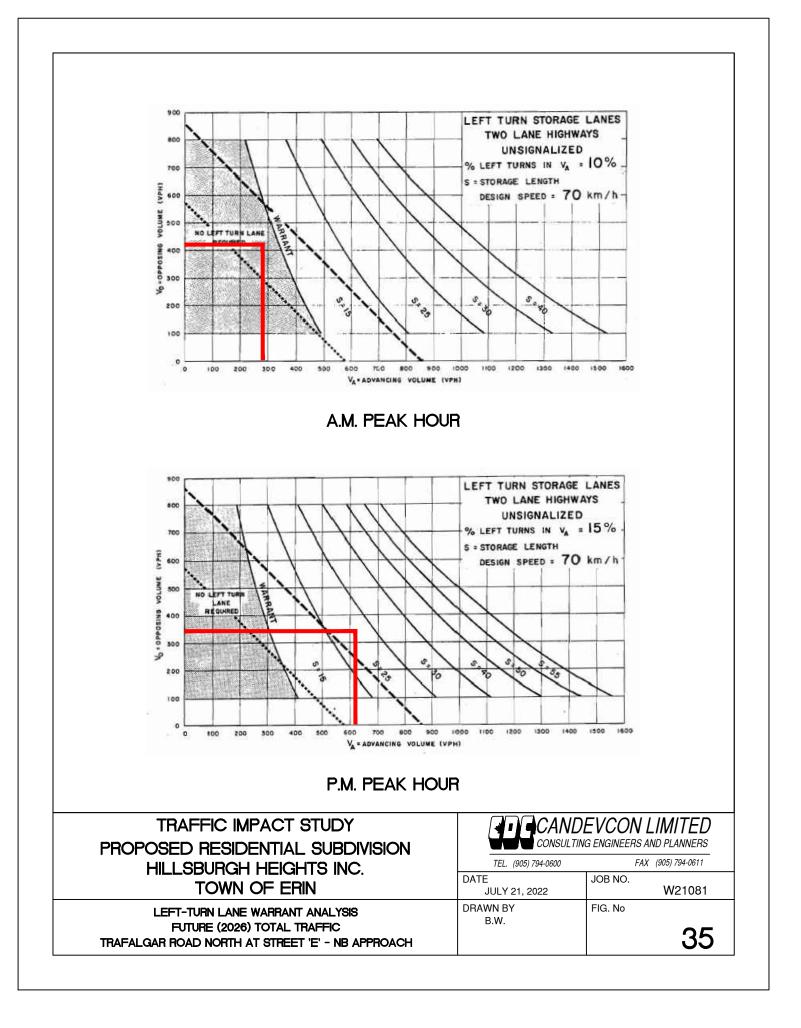
The Future (2026) Total Traffic Analysis indicates that the westbound approach for the Trafalgar Road North at George Street/Mill Street intersection will operate at a Level of Service "F" with an average delay of 52.3 seconds per vehicle during the P.M. Peak Hour.

Therefore, a signal warrant analysis was undertaken since the intersection is currently unsignalized. The analysis followed the procedures specified in Book 12 Justification 7 in the Ontario Traffic Manual and is provided in **Appendix G**⁸.

The signal warrant analysis indicates that the Trafalgar Road North at George Street/Mill Street intersection does not warrant traffic signals. Although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, an average delay of 52.3 seconds per vehicle is acceptable during the Weekday Peak Periods.

⁸ Ontario Traffic Manual Book 12 – Traffic Signals, Ministry of Transportation Ontario, March 2012.





7.2.4 Future (2026) Total Traffic Analysis – Recommended Improvements

For the Trafalgar Road North at Wellington Road 22 intersection, there are critical turning movements during the A.M. and P.M. Peak Hours.

In order to address the critical turning movements, the following improvements are recommended.

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a left turning lane at the eastbound approach with 35 metres of storage,
- Include a left turning lane at the westbound approach with 45 metres of storage,
- Extend the storage of the southbound left turning lane to 65 metres.

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, improvements were made based on the results of the left-turn lane warrant analysis. The left-turn lane warrant analysis indicates that a northbound left turning lane is warranted during the P.M. Peak Hour. For the Trafalgar Road North at Howe Street/proposed Street 'A' intersection, to avoid issues with safety due to the construction of a northbound left-turning lane, a southbound left-turning lane will be constructed.

The following improvements are recommended for the concerned intersections.

Proposed Street 'A'/Howe Street at Trafalgar Road North

• Include a left turning lane at the northbound and southbound approaches with 15 metres of storage.

Proposed Street 'E' at Trafalgar Road North

• Include a left turning lane at the northbound approach with 15 metres of storage.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 52.3 seconds per vehicle, it is considered acceptable for Peak Period conditions.

7.2.4 Future (2026) Total Traffic Analysis – Recommended Improvements (Cont'd)

The traffic conditions with the recommended improvements are summarized in **Table 14.** The related calculations are provided in **Appendix E**.

Table 14. Future (2020)	Turning Lane			Peak Hou	-			Peak Hou	r
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
	Overall	0.89	С	28.1	n/a	1.00	D	44.3	n/a
	EBL	0.14	С	23.1	10.0	0.70	Е	75.4	31.7
Trafalgar Road North	EB TR	0.89	D	50.1	80.0	0.51	С	30.7	52.7
at	WBL	0.72	С	34.1	36.2	0.54	D	39.4	40.8
	WB TR	0.42	В	15.5	28.5	0.99	Е	71.3	135.7
Wellington Road 22	NBL	0.19	В	14.0	9.5	0.31	В	15.8	27.1
(Signalized)	NB TR	0.47	В	13.8	42.4	1.00	D	52.8	245.9
	SBL	0.73	С	29.8	60.9	0.70	С	32.0	21.7
	SB TR	0.80	C	26.7	109.7	0.35	В	10.2	48.1
	Overall	0.29	Α	4.9	n/a	0.27	А	2.0	n/a
Trafalgar Road North	EB Approach	0.29	В	11.7	9.5	0.13	В	12.9	3.6
at	WB Approach	0.08	В	14.7	2.1	0.02	D	26.8	0.6
Howe Street/	NBL	0.06	А	7.9	1.5	0.09	А	8.2	2.3
Proposed Street 'A'	NB TR	0.11	А	0.0	0.0	0.27	А	0.0	0.0
(Un-signalized)	SBL	0.00	А	7.6	0.1	0.01	А	8.3	0.2
	SB TR	0.14	А	0.0	0.0	0.19	А	0.0	0.0
Trafalgar Road North	Overall	0.27	Α	1.9	n/a	0.33	Α	1.6	n/a
C	EB Approach	0.19	В	13.0	5.7	0.14	В	13.0	3.8
at	NBL	0.03	А	8.4	0.7	0.09	А	8.3	2.4
Proposed Street 'E'	NBT	0.16	А	0.0	0.0	0.33	А	0.0	0.0
(Un-signalized)	SB Approach	0.27	А	0.0	0.0	0.22	А	0.0	0.0

 Table 14: Future (2026) Total Traffic – Level of Service – with Improvements

Note 1: Delays are measured in seconds per vehicle.

7.2.4 Future (2026) Total Traffic Analysis – Recommended Improvements (Cont'd)

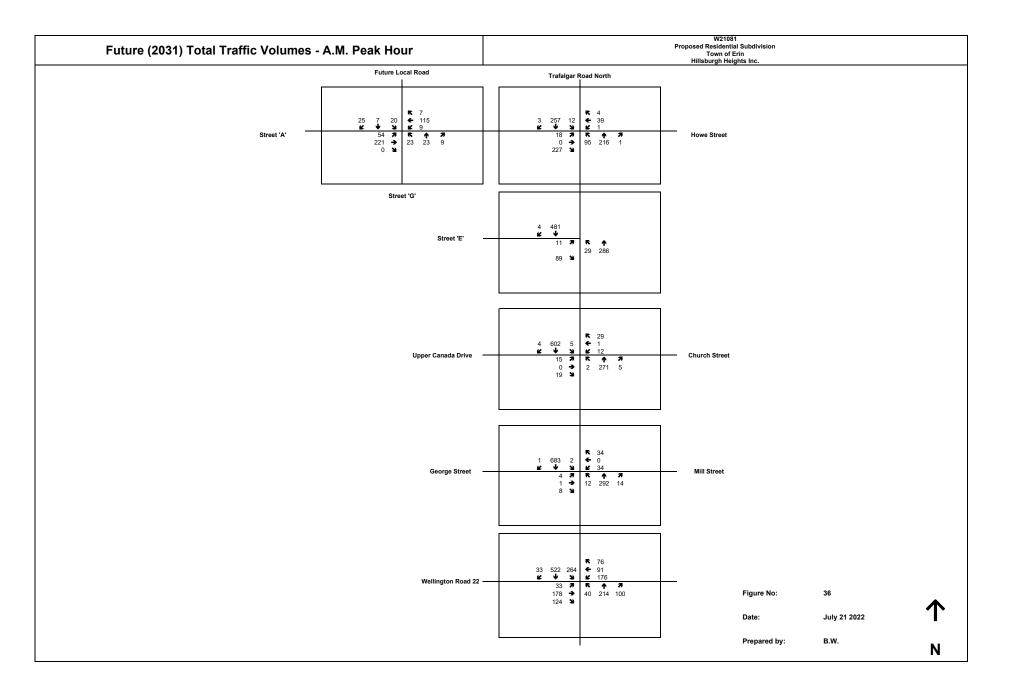
With the recommended improvements, the intersection of Trafalgar Road North at Wellington Road 22 will operate at a Level of Service C during the A.M. Peak Hour and a Level of Service "D" during the P.M. Peak Hour. All of the turning movements will operate at a Level of Service "D" or better during the A.M. Peak Hour and a Level of Service "E" or better during the P.M. Peak Hour.

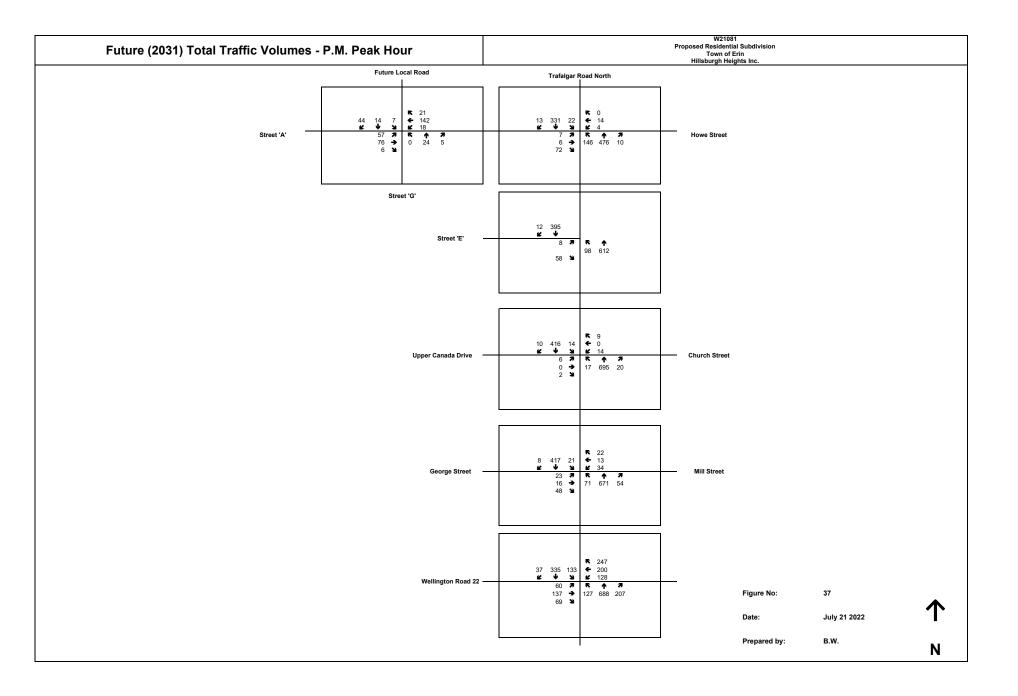
The Trafalgar Road North at Howe Street/proposed Street 'A' intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours. All of the turning movements will operate at a Level of Service "B" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

The Trafalgar Road North at proposed Street 'E' intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours. All of the turning movements will operate at a Level of Service "B" or better during the A.M. and P.M. Peak Hours.

7.3 Future (2031) Total Traffic

The Future (2031) Total Traffic is based on the Future (2031) Total Background Traffic Volumes plus the Site-Generated Traffic Volumes from the Subject Property and the Site-Generated Traffic Volumes from the anticipated development that is immediately north of the Subject Property and that is owned by the applicant. (Figures 14 and 15) The Future (2031) Total Traffic Volumes are provided in Figures 36 and 37.





7.4 Future (2031) Total Traffic Analysis

For the Future (2031) Total Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

For the Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North, Howe Street/proposed Street 'A' at Trafalgar Road North and proposed Street 'E' at Trafalgar Road North intersections, the signal timing plans and the lane configurations used in the Future (2026) Total Traffic Analysis are used in the Future (2031) Total Traffic Analysis.

The proposed Street 'A' at proposed Street 'G'/future Local Road intersection was analyzed as a single lane roundabout that is yield-controlled at all approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

The results of the analysis are summarized in **Table 15.** The related calculations are provided in **Appendix E**.

7.4 Future (2031) Total Traffic Analysis (Cont'd)

Table 15: Future (2031)	Turning Lane			Peak Hou	r		P.M. 1	Peak Hou	r
Intersection	/Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
	Overall	1.16	D	46.4	n/a	1.59	F	99.0	n/a
Trafalgar Road North	EB Approach	0.65	C	24.6	69.6	0.66	С	27.7	62.2
at	WB Approach	1.16	F	126.8	118.8	1.16	F	117.3	175.8
Wellington Road 22	NBL	0.23	В	16.5	11.1	0.43	В	19.5	30.0
(Signalized)	NB TR	0.49	В	15.8	53.3	1.19	F	119.2	256.6
(Signalized)	SBL	0.85	D	44.1	83.2	1.59	F	333.1	54.8
	SB TR	0.84	C	30.3	136.2	0.49	В	16.9	65.4
Trafalgar Road North	Overall	0.37	A	5.8	n/a	0.23	A	4.0	n/a
at	EB Approach	0.37	В	13.3	13.7	0.23	C	16.6	7.0
Howe Street/	WB Approach	0.14	C	17.6	3.7	0.14	Е	37.0	3.9
Proposed Street 'A'	NB Approach	0.08	А	2.9	2.0	0.13	А	3.2	3.7
(Un-signalized)	SB Approach	0.01	А	0.4	0.2	0.02	А	0.7	0.6
Trafalgar Road North	Overall	0.26	Α	1.8	n/a	0.62	Α	7.6	n/a
at	EB Approach	0.05	С	18.9	1.3	0.49	E	41.9	19.4
George Street/	WB Approach	0.26	C	22.3	8.2	0.62	F	75.0	24.9
Mill Street	NB Approach	0.01	А	0.5	0.4	0.07	А	1.7	1.8
(Un-signalized)	SB Approach	0.00	А	0.0	0.0	0.03	А	0.7	0.6
Trafalgar Road North	Overall	0.17	Α	1.6	n/a	0.12	Α	1.1	n/a
at	EB Approach	0.17	C	22.2	4.7	0.04	D	25.6	1.1
Upper Canada Drive/	WB Approach	0.15	C	16.7	4.0	0.12	D	25.9	3.2
Church Street	NB Approach	0.00	Α	0.1	0.1	0.02	Α	0.4	0.4
(Un-signalized)	SB Approach	0.00	А	0.1	0.1	0.02	А	0.5	0.4

Table 15: Future (2031) Total Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

Note 2: Signalized intersections are based on existing signal timing plans.

7.4 Future (2031) Total Traffic Analysis (Cont'd)

			A.M. Peak Hour P.M. Peak Hou						r
Intersection	Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
Trafalgar Road North	Overall	0.31	Α	1.9	n/a	0.26	Α	2.2	n/a
at	EB Approach	0.22	В	14.1	6.5	0.16	В	14.4	4.5
Proposed Street 'E'	NB Approach	0.03	А	1.1	0.8	0.10	А	2.3	2.5
(Un-signalized)	SB Approach	0.31	А	0.0	0.0	0.26	А	0.0	0.0
Proposed Street 'A'	Overall	0.28	Α	5.5	n/a	0.19	Α	4.9	n/a
at	EB Approach	0.28	А	6.1	7.0	0.14	А	4.7	0.0
Proposed Street 'G'/	WB Approach	0.14	А	4.9	7.0	0.19	А	5.4	7.0
future Local Road	NB Approach	0.08	А	5.3	0.0	0.03	А	4.1	0.0
(Roundabout)	SB Approach	0.06	А	4.4	0.0	0.08	А	4.6	0.0

 Table 15: Future (2031) Total Traffic – Level of Service

Note 1: Delays are measured in seconds per vehicle.

7.4 Future (2031) Total Traffic Analysis (Cont'd)

Trafalgar Road North at Wellington Road 22

The analysis of the Future (2031) Total Traffic Conditions indicates that the signalized intersection will continue to operate at a Level of Service "D" during the A.M. Peak Hour and will begin to operate at a Level of Service "F" during the P.M. Peak Hour. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. Peak Hour and high during P.M. Peak Hour.

The westbound approach will continue to operate at a Level of Service "F' with a volume over capacity ratio that is greater than 1.0 during the A.M. Peak Hour and will begin to operate at a Level of Service "F' with a volume over capacity ratio that is greater than 1.0 during the P.M. Peak Hour.

During the P.M. Peak Hour, the shared through-right turning lane at the northbound approach will continue to operate with a volume over capacity ratio that is greater than 1.0 and will begin to operate at a Level of Service "F".

The left turning lane at the southbound approach will continue to operate at a Level of Service "F" and will begin to operate with a volume over capacity ratio that is greater than 1.0. The queue lengths at the southbound left turning lane may begin to result in a spill back of vehicles into the adjacent lane during the A.M. and P.M. Peak Hours.

7.4 Future (2031) Total Traffic Analysis (Cont'd)

Trafalgar Road North at Howe Street/proposed Street 'A'

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are high during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "E" or better during the P.M. Peak Hour.

Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. Peak Hour and moderate during P.M. Peak Hour.

During the P.M. Peak Hour, the westbound approach will begin to operate at a Level of Service "F" with an average delay of 75.0 seconds per vehicle. All of the other turning movements will continue to operate at a Level of Service "C" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "E" or better during the P.M. Peak Hour.

7.4 Future (2031) Total Traffic Analysis (Cont'd)

Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

Trafalgar Road North at proposed Street 'E'

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection are minimal during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements will continue to operate at a Level of Service "B" or better.

Proposed Street 'A' at proposed Street 'G'/future Local Road

The analysis of the Future (2031) Total Traffic Conditions indicates that all of the approaches at the roundabout will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

7.4.1 Future (2031) Total Traffic Analysis – Right-Turn Lane Warrant Analysis

For the southbound right turning movements at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a right-turn lane warrant analysis was conducted using the principles provided in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways. Based on the procedure to the right-turn lane warrant analysis, a right-turning lane should be considered when traffic volumes are 60 vehicles per hour or higher. With the southbound right turning movements at the concerned intersections operating with 13 vehicles per vehicle or less, right turning lanes are not warranted.

7.4.2 Future (2031) Total Traffic Analysis – Signal Warrant Analysis

The Future (2031) Total Traffic Analysis indicates that the westbound approach for the Trafalgar Road North at George Street/Mill Street intersection will operate at a Level of Service "F" with an average delay of 75.0 seconds per vehicle during the P.M. Peak Hour.

Therefore, a signal warrant analysis was undertaken since the intersection is currently unsignalized. The analysis followed the procedures specified in Book 12 Justification 7 in the Ontario Traffic Manual and is provided in **Appendix G**.

The signal warrant analysis indicates that the Trafalgar Road North at George Street/Mill Street intersection does not warrant traffic signals. Although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, an average delay of 75.0 seconds per vehicle is acceptable during the Weekday Peak Periods.

7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements

For the Trafalgar Road North at Wellington Road 22 intersection, there are critical turning movements as a result of background traffic growth from the 2026 horizon year.

In order to address the critical turning movements, the following improvements are recommended beyond the recommendations made for the 2026 horizon:

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a right turning lane at the northbound approach with 20 metres of storage,
- Extend the storage for the left turning lane at the southbound approach to 70 metres,
- Include a right turning lane at the westbound approach with 35 metres of storage.

The traffic conditions with the recommended improvements are summarized in **Table 16.** The related calculations are provided in **Appendix E**.

7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements (Cont'd)

Table 16: Future (2031)		Level		Peak Hou	^	ovennen.		Peak Hou	r
Intersection	Turning Lane /Approach	V/C	LOS	Delay ¹	95th Queue (m)	V/C	LOS	Delay ¹	95th Queue (m)
	Overall	0.86	C	28.3	n/a	0.82	В	19.7	n/a
	EBL	0.15	С	23.9	11.4	0.24	С	20.9	15.3
	EB TR	0.86	D	45.6	79.2	0.54	С	21.3	36.9
Trafalgar Road North	WBL	0.79	D	40.6	43.5	0.54	С	28.5	29.9
at	WBT	0.18	В	17.6	20.0	0.51	С	24.3	39.9
Wellington Road 22	WBR	0.17	Α	5.4	8.1	0.57	В	15.0	32.3
(Signalized)	NBL	0.22	В	15.7	10.6	0.35	В	12.4	21.3
(Signalized)	NBT	0.35	В	14.6	36.1	0.82	С	22.8	135.6
	NBR	0.16	А	3.1	7.1	0.26	А	5.1	16.5
	SBL	0.72	С	28.0	68.1	0.78	D	47.3	44.6
	SB TR	0.85	С	31.0	130.0	0.45	В	11.4	47.8
	Overall	0.37	Α	5.6	n/a	0.31	А	3.2	n/a
Trafalgar Road North	EB Approach	0.37	В	13.3	13.7	0.23	С	16.6	7.0
at	WB Approach	0.14	C	17.6	3.7	0.14	Е	36.9	3.9
Howe Street/	NBL	0.08	А	8.0	2.0	0.13	А	8.5	3.7
Proposed Street 'A'	NB TR	0.13	А	0.0	0.0	0.31	А	0.0	0.0
(Un-signalized)	SBL	0.01	А	7.7	0.2	0.02	А	8.5	0.6
	SB TR	0.16	А	0.0	0.0	0.22	А	0.0	0.0
Trafalgar Road North	Overall	0.31	Α	1.8	n/a	0.39	A	1.5	n/a
at	EB Approach	0.22	В	14.1	6.5	0.16	В	14.4	4.5
Proposed Street 'E'	NBL	0.03	А	8.6	0.8	0.10	А	8.6	2.5
*	NBT	0.18	А	0.0	0.0	0.39	А	0.0	0.0
(Un-signalized)	SB Approach	0.31	А	0.0	0.0	0.26	А	0.0	0.0

 Table 16: Future (2031) Total Traffic – Level of Service – with Improvements

Note 1: Delays are measured in seconds per vehicle.

7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements (Cont'd)

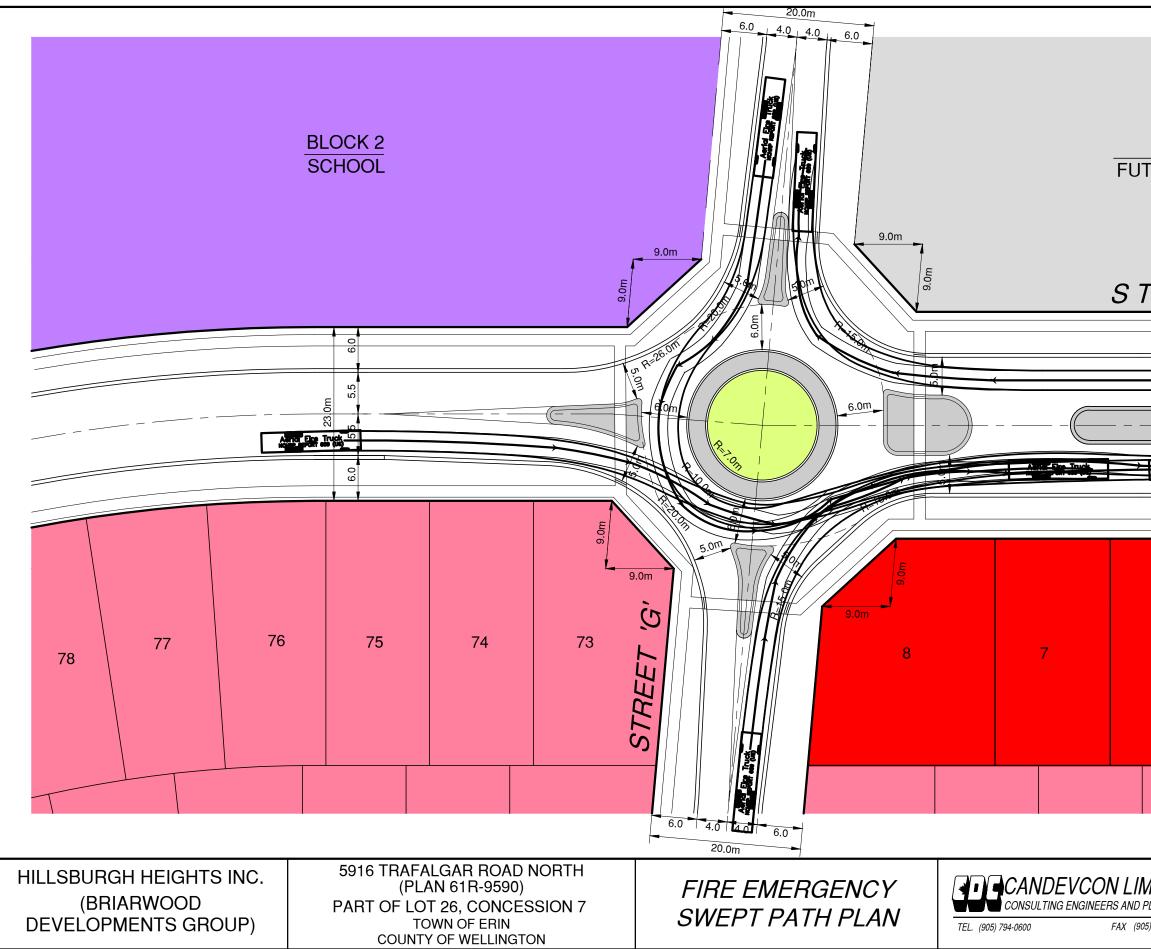
For Trafalgar Road North at Wellington Road 22, the intersection will operate at a Level of Service "C" during the A.M. Peak Hour and a Level of Service "B" during the P.M. Peak Hour. All of the turning movements will operate at a Level of Service "D" or better during the A.M. and P.M. Peak Hours.

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, with the improvements recommended for the 2026 horizon year, all of the turning movements will continue to operate at an acceptable Levels of Service during the A.M. and P.M. Peak Hours.

For the Trafalgar Road North at George Street/Mill Street intersection, although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, with an average delay of 75.0 seconds per vehicle, it is considered acceptable for Peak Period conditions.

8. REVIEW OF ROUNDABOUT – SWEPT PATH ANALYSIS

Using the preliminary design of the roundabout intersection at proposed Street 'A' at proposed Street 'G'/future Local Road, the geometry of the roundabout was analyzed for fire emergency vehicles. Vehicle swept paths have been analyzed in AutoTURN software and are provided in **Figure RA-1**. The vehicle swept paths demonstrate that the proposed geometry and right of way is acceptable.



BLOCK 5A FUTURE DEVELOPMENT (COMMERCIAL)

STREET 'A'

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9. SIGHT DISTANCE ANALYSIS

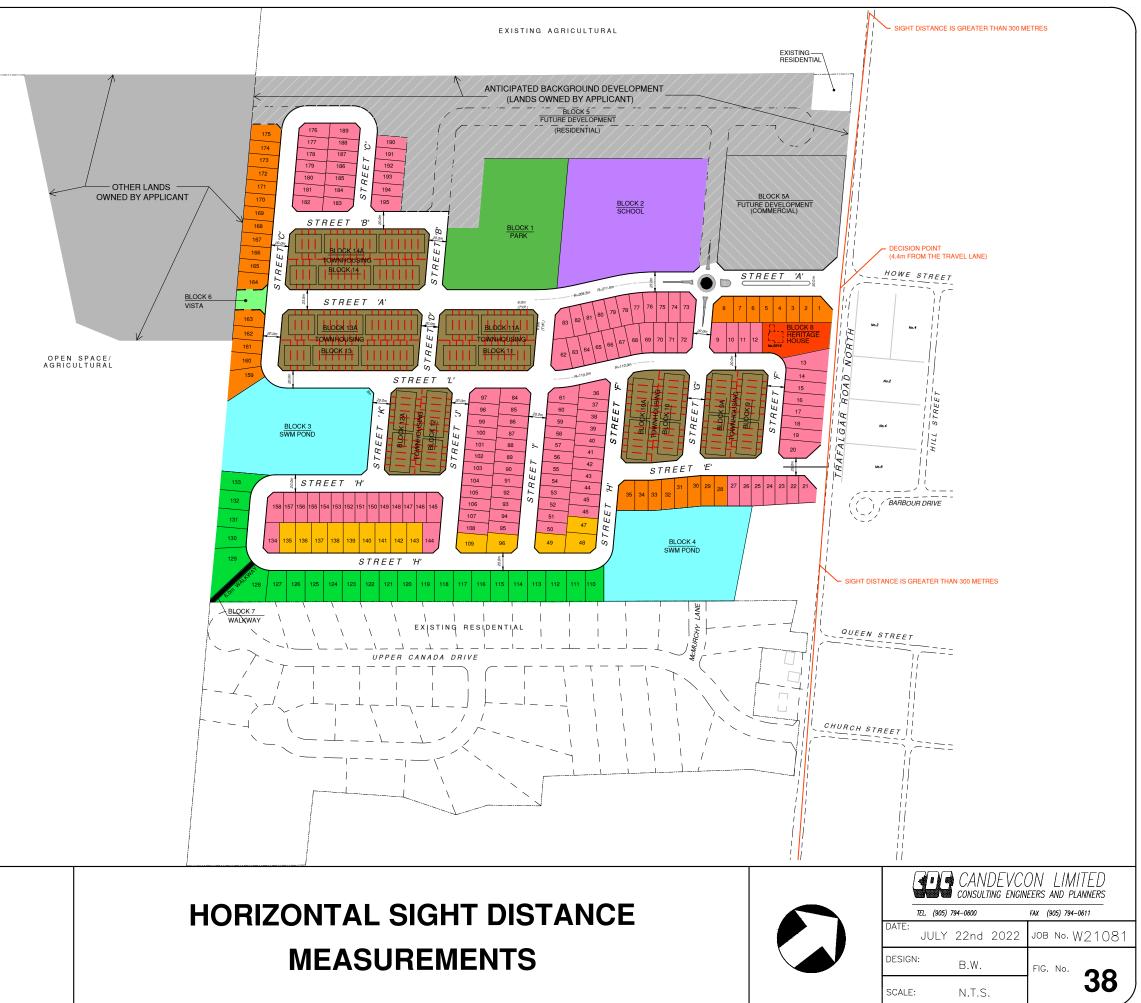
For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, the sight distances for vehicles exiting from the Subject Subdivision were reviewed. To evaluate the sight distances for the proposed accesses, the principles were taken from the Transportation Association of Canada's Geometric Design Guide for Canadian Roads⁹.

To determine the sight distances from the vertical plane, CANDEVCON LIMITED conducted a site visit on April 26, 2022. Pictures taken during the site visit that illustrate the site distances from the proposed accesses are provided in **Appendix H**. For the sight distances from the horizontal plane, issues are minimal since the alignment of Trafalgar Road North is relatively straight. To demonstrate that the sight distances provided from the horizontal plane will exceed the sight distances required, the sight distances for vehicles exiting Street 'A' are provided in **Figure 38**.

At approximately 200 metres north of the proposed Residential Subdivision, travelling in the southbound direction, the posted speed limit on Trafalgar Road North changes from 60 km/h to 40 km/h. Therefore, for vehicles leaving the Subject Subdivision by making a right-turn, the design speed for vehicles on Trafalgar Road North is 70 km/h, conservatively. For vehicles leaving the Subject Subdivision by making a left-turn or a through movement, the design speed for vehicles on Trafalgar Road North is 60 km/h.

Based on the results of the analysis, the sight distances provided exceed the sight distances required. The findings of the sight distance analysis are provided in **Table 17**.

⁹ Geometric Design Guide for Canadian Roads, Transportation Association of Canada, June 2017.



TRAFFIC IMPACT STUDY HILLSBURGH HEIGHTS INC. PROPOSED RESIDENTIAL SUBDIVISION 5916 TRAFALGAR ROAD NORTH TOWN OF ERIN

9. SIGHT DISTANCE ANALYSIS (CONT'D)

			Sight Dista	nce Provided
Departing From	Turning Movement	Sight Distance Required		
			Vertical	Horizontal
	EBL	133m (Note 1)	220m	>300m
Street 'A'	EBT	117m (Note 1)	220m	>300m
	EBR	126m (Note 2)	240m	>300m
Street 'E'	EBL	133m (Note 1)	>300m	>300m
	EBR	126m (Note 2)	220m	>300m

Table 17: The Required and Provided Sight Distances

Note 1: The design speed for Trafalgar Road North is 60 km/h. Note 2: The design speed for Trafalgar Road North is 70 km/h.

10. SUMMARY

The proposed Residential Subdivision is expected to generate a total of 527 trips during the A.M. Peak Hour (218 inbound trips and 309 outbound trips) and 368 trips during the P.M. Peak Hour (220 inbound trips and 148 outbound trips). During the A.M. and P.M. Peak Hours, traffic impacts from the trips generated by the proposed Residential Subdivision are moderate.

Vehicle access to the proposed Residential Subdivision from Trafalgar Road North is provided via the proposed Street 'E' that is located at the southeast corner of the Subject Property and the proposed Street 'A' that aligns with Howe Street to form a four legged intersection. Between the 2026 and 2031 horizon years, it is anticipated that the future Residential Subdivision owned by the applicant that is immediately north of the Subject Subdivision will be fully built-out and occupied. At the time this report was prepared, details to the future Residential Subdivision are preliminary. However, for the roundabout within the Subject Subdivision that is immediately west of the proposed Street 'A'/Howe Street at Trafalgar Road North intersection, the anticipated development will provide a local road that aligns with Street 'G' at Street 'A' to form the north leg.

The lands that are immediately west of the Subject Subdivision that are owned by the applicant lie outside of the Hillsburgh Urban Boundary and are designated under Agricultural and Greenland. The future development potential for these lands, since they lie outside the Urban Boundary, will only be recognized once the lands are brought into the Urban Boundary, which could take up to 30 years. Therefore, the potential development will be built-out and occupied after the 2031 horizon year.

The following recommendations should be considered for the full build-out 2026 horizon year:

Trafalgar Road North at Wellington Road 22

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a left turning lane at the eastbound approach with 35 metres of storage,
- Include a left turning lane at the westbound approach with 45 metres of storage,
- Extend the storage of the southbound left turning lane to 65 metres.

10. SUMMARY (CONT'D)

Proposed Street 'A'/Howe Street at Trafalgar Road North

- An un-signalized intersection with stop-controls at the eastbound and westbound approaches,
- A left turning lane with 15 metres of storage and a shared through-right turning lane at the northbound and southbound approaches,
- A shared left-through-right turning lane at the eastbound and westbound approaches.

Proposed Street 'E' at Trafalgar Road North

- An un-signalized intersection with a stop-control at the eastbound approach,
- A left turning lane with 15 metres of storage and a through lane at the northbound approach,
- A shared left-right turning lane at the eastbound approach,
- A shared through-right turning lane at the southbound approach.

Proposed Street 'A' at proposed Street 'G'

• A single lane roundabout intersection that is yield-controlled at all approaches.

With the exception of the Trafalgar Road North at George Street/Mill Street intersection, all of intersections will have turning movements that operate at a Level of Service "E" or better.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 52.3 seconds per vehicle, it is considered acceptable for Peak Period conditions.

10. SUMMARY (CONT'D)

To address the growth in background traffic, the following recommendations should be considered for the five (5) year post build-out 2031 horizon year (beyond the improvements as recommended for the 2026 horizon):

Trafalgar Road North at Wellington Road 22

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a right turning lane at the northbound approach with 20 metres of storage,
- Extend the storage for the left turning lane at the southbound approach to 70 metres,
- Include a right turning lane at the westbound approach with 35 metres of storage.

Proposed Street 'A' at proposed Street 'G'/future Local Road

• A north leg to the intersection that is a part of the anticipated development (lands owned by applicant) that is immediately north will be constructed.

With the exception of the Trafalgar Road North at George Street/Mill Street intersection, all of intersections will have turning movements that operate at a Level of Service "E" or better.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 75.0 seconds per vehicle, it is considered acceptable for Peak Period conditions.

In addition, the preliminary design of the roundabout at proposed Street 'A' at proposed Street 'G'/future Local Road demonstrates that adequate circulation will be provided for fire emergency vehicles.

10. SUMMARY (CONT'D)

Based on the analysis outlined in the Study, with the implementation of the recommendations as outlined, all the key intersections will operate at acceptable levels of service during the Weekday A.M. and P.M. Peak Hours under the 2026 and 2031 horizon years.

This Report was prepared by:

CANDEVCON LIMITED



Brian Wong, P. Eng. Intermediate Transportation Engineer



David Lee, P. Eng. Project Manager

APPENDIX A

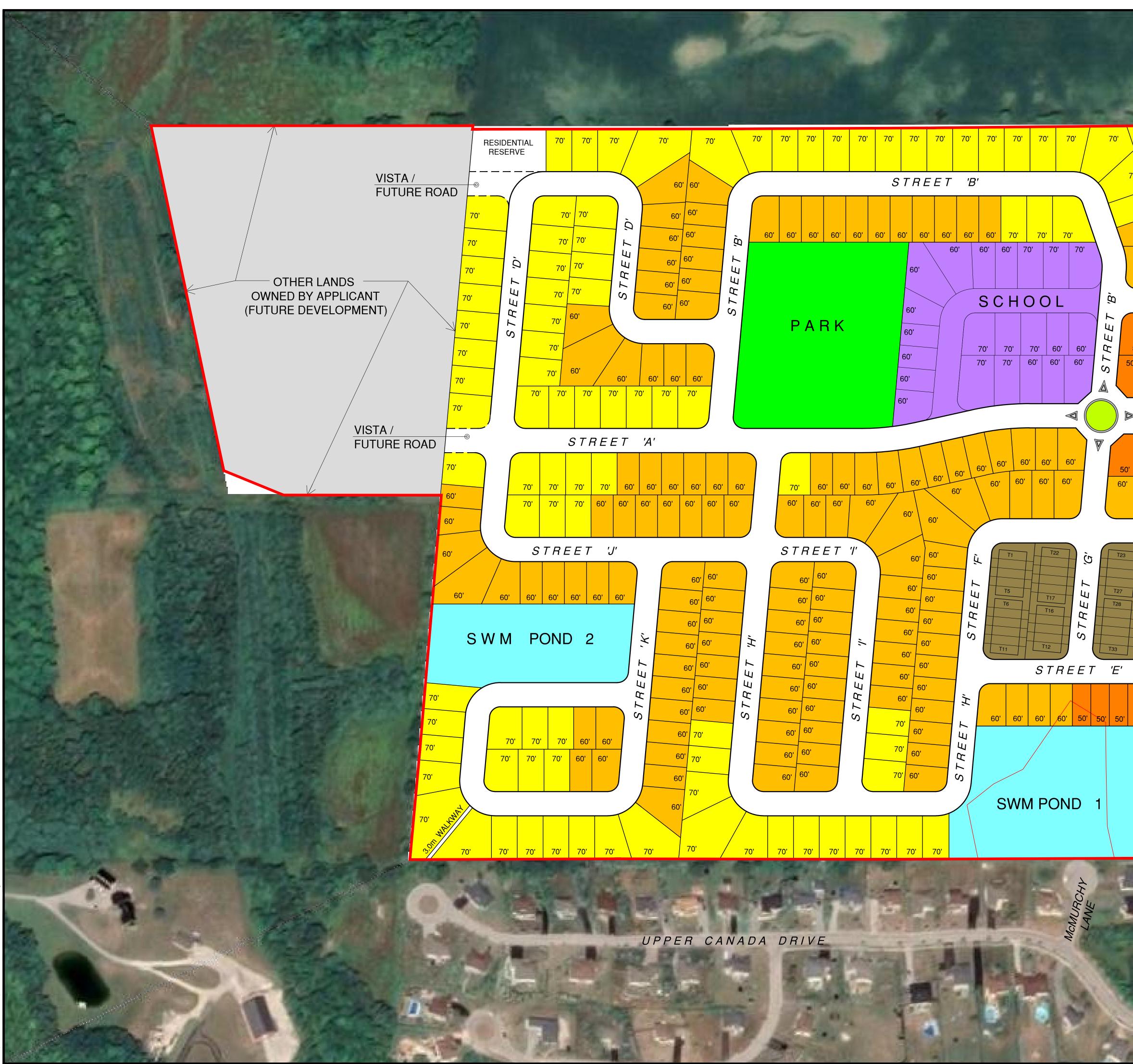
TERMS OF REFERENCE

Traffic Impact Study – Terms of Reference

- a) Assemble, review and confirm background data (i.e. traffic volume/flow on the adjacent road network during weekday peak hours) available from official sources, existing road geometry and access locations.
- b) Gather/conduct turning movement counts (if necessary) for the Howe Street at Trafalgar Road North intersection during the Weekday AM and Weekday PM Peak Hours. (We may need to use historical traffic counts given the current covid-19 situation.)
- c) Establish existing traffic patterns and historic travel growth rates for the study area.
- d) Consult with the County of Wellington and the Town of Erin to confirm data as required (i.e. growth trends, other proposed development timing etc.), issues/developments to be addressed and any anticipated future road improvements.
- e) Confirm with the County of Wellington and the Town of Erin for any future planned road improvements in the area.
- f) Assess total future trips generated by the proposed Residential Subdivision during the Weekday AM and Weekday PM Peak Hours.
- g) Develop the trip distribution and traffic assignment for the proposed Residential Subdivision during the Weekday AM and Weekday PM Peak Hours.
- h) Establish the five (5) year time horizon post full build-out of the proposed Residential Subdivision to forecast future peak periods of street traffic.
- i) Analyze peak period traffic operations at the following key site access points. (To be confirmed with the County of Wellington and the Town of Erin)
 - Street 'A'/Howe Street at Trafalgar Road North,
 - Street 'F' at Trafalgar Road North.
- j) Complete traffic operations and volume-capacity analyses using the Synchro 9.0 software.
- k) Assess existing and future total background and total traffic operations (five (5) year horizon post development) at the proposed key access points mentioned above.
- Prepare a report to summarize the findings of the traffic impact analysis, as well as to recommend any improvements required to mitigate the traffic impacts (if any). Submit the final report to the County of Wellington and the Town of Erin for review/comments.

Traffic Impact Study – Terms of Reference (Cont'd)

m) Provide and circulate copies of the final report to all applicable approval authorities (first submission only).



221 WEST - Files W21081-HILLSBURGH-BRIARWOOD-HOMES/W21081-DP-1-PRELIMINARY-DEVELOPMENT-PLAN-LAND-HILLSBURGH-OCTOBER-5-2021.dw

	LAND USE RESIDENTIAL	AREA 23.31 ha. 57.60		CENTAGE 57.8%
	PARK 1	2.03 ha. 5.01	Ac.	5.0%
	SWM POND 1 SWM POND 2	1.71 ha. 4.23 1.09 ha. 2.69		6.9%
and the second s	SCHOOL ROADS / MISC.	2.23 ha. 5.51 10.03 ha. 24.79		5.5% 24.8%
70' 60'	TOTAL	40.40 ha. 99.83	Ac.	100%
	LOT FRONTAGE TYPE DEPTH		NUMBER OF LOTS	NUMBER OF UNITS
50'	T 7.5mx35.0	3.31 AC	-	44
70' ^{70'} È <u>50'</u>	50' 15.2mx35.0n	10.74 ha	44	44
	60' 18.3mx35.0n	¹ 26.54 Ac	147	147
60' 70' 70' U 50'	70' 21.3mx35.0m T O T A L	23.31 ha	97 288	97 332
	TOTAL	57.60 Ac	200	002
50' 50' 50' 50' 50' 50'				
50' 50' 50' 50' 50' 50' 2				
STREET 'A'				
STREET 'A' HOWE ST.				
U U U U U U U U U U U U U U U U U U U				
' 50' 50' 50' 50' 50' 60' 60'				
60' 60' <u>60'</u>				
(H)				
	$\overline{\mathbb{A}}$			
		SCRIPTION		DATE BY
BARBOUR DR.		NDEVCC		
50' 50' 50' 50' 50' 50' F	9358 GOREWAY DRIN	ULTING ENGINE /E BRAM	IPTON, ONTA	RIO L6P 0M7
	TEL. (905) 794-0600		FAX (9	05) 794-0611
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the second second second	CHECKED BY: D.K.H.	DRAWI		
CHURCH STREET	SCALE: 1:1500		P-	- 1,
	DATE: OCT., 5th 2	021		

David Lee

From:	Joe Mullan <mullan@ainleygroup.com></mullan@ainleygroup.com>
Sent:	October-21-21 1:42 PM
То:	Brian Wong
Cc:	Nick Colucci; Angela Sciberras; Tanjot Bal; Pasquale Costanzo; David Lee; Diarmuid Horgan
Subject:	W21081 - 5916 Trafalgar Road North - Terms of Reference (Town of Erin)
Attachments:	Erin Development Ownership Map.pdf

Hi Brian:

We have review your proposed Terms of Reference for the Hillsburgh Heights (Briarwood) development and we provide the following comments:

- 1. Given the size of the development, we concur with the minimum of two access street from the Development onto Trafalgar Road. These access streets should be designed in accordance with the TAC Manual in relation to intersection spacing and corner clearance requirements etc.
- 2. Please utilize the Institute of Transportation Engineers Trip Generation Manual 10th Edition for site trip estimate and using traffic count data and Transportation Tomorrow Survey data for trip distribution.
- 3. Given the size of the development, future horizons should include build out year of any phases if applicable, plus the full build out year of the development, along with five and ten years post full build out.
- 4. It is unlikely that there is any traffic data/turning movements for Trafalgar Road and the existing intersections through Hillsburgh, therefore, we require that Traffic Counts/turning movement data be obtained for the 3 hrs Weekday AM period (7am to 10am) and 3 hrs Weekday PM from 3pm to 6pm). This will account for any possible changes that have occurred to people's work schedules because of the pandemic, whereby the peak hour may occur later in the morning or earlier in the afternoon.
- 5. With respect to future growth within Hillsburgh, there are four other major developments that are proposing to develop within the next number of years. Given that Trafalgar Road is the main spine of Hillsburgh, all new developments will impact the amount of traffic utilizing Trafalgar Road; therefore, these new Developments need to be accounted for in the TIS for your clients development.

The details we have associated with these future developments in Hillsburgh are noted below and the locations of each are shown on the attached map

#	Development Name	Number of Proposed SDE's
2	Carson Reid Homes Ltd	182
3	Thomasfield Homes Ltd	210
4	Dominion Packers & Realties (Tavares)	700
16	Chantler	213

6. Given the potential impact of these developments collectively (1,625 SDE's) on Trafalgar Road, please include the following intersections in the traffic counts and associated analysis within the TIS for your clients development.

- Trafalgar Road / Upper Canada Drive / Church Street
- Trafalgar Road / Mill Street/George Street
- Trafalgar Road / County 22

Should you have any questions regarding this information please do not hesitate to contact me

Regards,

J. A. Mullan, P.Eng. President & CEO



Cell: (705) 718-7230

WWW.AINLEYGROUP.COM

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From: Brian Wong <<u>brian@candevcon.com</u>>
Sent: Wednesday, October 6, 2021 10:12 AM
To: Nick Colucci <<u>nick.colucci@erin.ca</u>>
Cc: Joe Mullan <<u>mullan@ainleygroup.com</u>>; David Lee <<u>david@candevcon.com</u>>; Diarmuid Horgan
<<u>dhorgan@candevcon.com</u>>
Subject: W21081 - 5916 Trafalgar Road North - Terms of Reference (Town of Erin)

Good Morning Nick,

We are preparing a Traffic Impact Study for a proposed Residential Subdivision that is immediately west of Trafalgar Road North and north of Upper Canada Drive. Please find the Terms of Reference and the latest Preliminary Development Plan attached for your review and comment. In the meantime, can you please provide the Traffic Impact Study and/or the Site Plan for any anticipated background developments within the vicinity of the proposed Residential Subdivision.

If you require any further information, please do not hesitate to contact me.

Brian Wong, P.Eng.

Intermediate Transportation Engineer

CANDEVCON LIMITED CONSULTING ENGINEERS & PLANNERS GTA WEST OFFICE (CORPORATE) 9358 Goreway Drive Brampton, Ontario, L6P 0M7 (905)794-0600 OFFICE (905)794-0611 FAX E-mail: brian@candevcon.com

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David Lee

From:	Kooistra, Tim <tkooistra@dillon.ca></tkooistra@dillon.ca>
Sent:	October-12-21 1:22 PM
То:	Brian Wong
Cc:	David Lee; Diarmuid Horgan; Pasquale Costanzo
Subject:	Re: FW: W21081 - 5916 Trafalgar Road North - Terms of Reference (County of Wellington)

Hi Brian,

As promised, I am following up with regard to the proposed residential subdivision located at 5916 Trafalgar Road North (Wellington Road 24) within the Town of Erin and located immediately north of the community of Hillsburgh. As I noted during our phone call, Dillon Consulting Limited has been retained by the County of Wellington to review the proposed scope of work for traffic impact studies that may impact the County road network and associated intersections. As a result, this response is being provided on behalf of the County of Wellington for your consideration.

The required transportation impact study will need to consider the following:

- The Existing and/or Future Operational analysis at the intersections of:
 - Wellington Road 24 (Trafalgar Road) and Howe Street / future Street 'A' currently unsignalized
 - o Wellington Road 24 and Upper Canada Drive / Church Street currently unsignalized
 - Wellington Road 22 and Wellington Road 24 signalized (signal timing is attached)
- Turning movement data will need to be collected at each of these three study area intersections.
- Future Operational analysis at:
 - the proposed internal Street 'A' & Street 'B' intersection future roundabout.
 - The proposed Wellington Road 24 (Trafalgar Road) and future Street 'E'
- Use a 2.0% per annum growth rate to forecast the traffic volumes to various horizon years including:
 - 2021 (Existing)
 - 2030 (Buildout)
 - 2035 (5 years following build-out)
- The report should include a discussion as to whether or not a local road connection to McMurchy Lane and Upper Canada Drive could be introduced rather than connecting Street 'E' to Wellington Road 24.
- The trip generation and future traffic volumes will need to explicitly consider that a school will be constructed on the school block as discussed.
- Due to the vertical profile of Wellington Road 24, a safety assessment will need to be completed at both locations. As you can see across the corridor from where Street 'E' was constructed, Barbour Drive features a cul-de-sac and no direct connection.
- Due to the vertical profile along Wellington Road 24 fronting the proposed residential development, sightline analysis needs to be completed at the locations of the two intersections are being proposed to connect to Wellington Road 24 (future Street 'A' & future Street 'E'). Based on available speeds found along this portion of the corridor, a 70 km/h design speed (posted + 30 km/h) should be used.
- The need for both a northbound left-turn lane and a southbound right-turn lane at the Howe Street / future Street 'A' intersection and the future Street 'E' intersection need to be explicitly assessed utilizing a 70 km/h design speed.

Lastly, any background developments that may impact future traffic volumes in the study area (along Wellington Road 24) will need to be identified by Town of Erin staff.

As always, please let me know if you have any questions or comments on this matter.

Thanks,

Tim

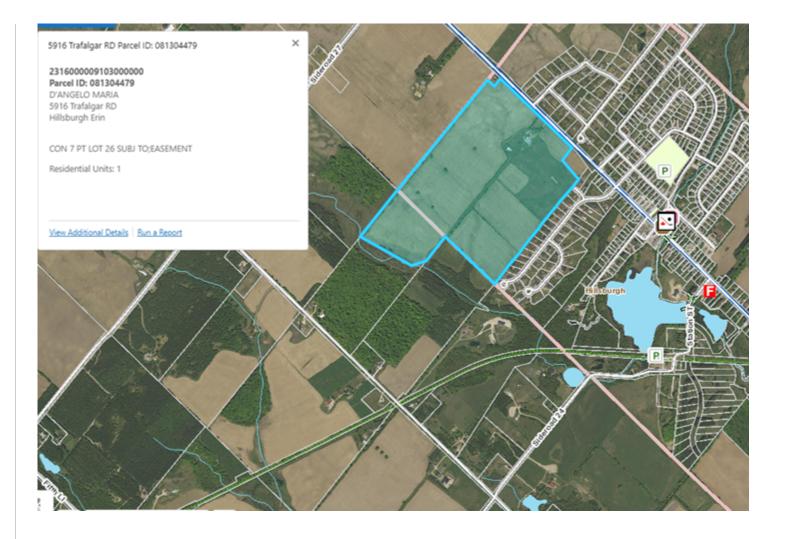
Tim Kooistra, C.E.T. Dillon Consulting Limited 130 Dufferin Avenue Suite 1400 London, Ontario, N6A 5R2
T - 519.438.1288 ext. 1330 F - 519.672.8209 M - 519.851.5403
TKooistra@dillon.ca www.dillon.ca

On Thu, Oct 7, 2021 at 9:02 AM Pasquale Costanzo <<u>pasqualec@wellington.ca</u>> wrote:

Hi Tim,

Could you review the attached terms of reference for a proposed subdivision at the north end of Hillsburgh and provide any comments.

Thank you



Pasquale Costanzo, C.E.T., CMMII Infrastructure Specialist

Technical Services Supervisor

County of Wellington, Roads Division

T 519.837.2601 x 2250

E pasqualec@wellington.ca

From: Brian Wong <<u>brian@candevcon.com</u>> Sent: Wednesday, October 6, 2021 10:20 AM To: Pasquale Costanzo <<u>pasqualec@wellington.ca</u>> Cc: David Lee <<u>david@candevcon.com</u>>; Diarmuid Horgan <<u>dhorgan@candevcon.com</u>> Subject: W21081 - 5916 Trafalgar Road North - Terms of Reference (County of Wellington) CAUTION: This email originated from outside the organization. Do not click links or open attachments unless you know the contents to be safe.

Good Morning Pasquale,

We are preparing a Traffic Impact Study for a proposed Residential Subdivision that is immediately west of Trafalgar Road North and north of Upper Canada Drive. Please find the Terms of Reference and the latest Preliminary Development Plan attached for your review and comment. In the meantime, can you please let me know whether the County has any recent turning movement counts for the intersection of Trafalgar Road North at Howe Street and can you please provide me with growth projections for Trafalgar Road North. The horizon year is anticipated for 2030.

If you require any further information, please do not hesitate to contact me.

Brian Wong, P.Eng.

Intermediate Transportation Engineer

CANDEVCON LIMITED

CONSULTING ENGINEERS & PLANNERS

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APPENDIX B

TURNING MOVEMENT COUNTS



Project #21-219 - Candevcon Limited

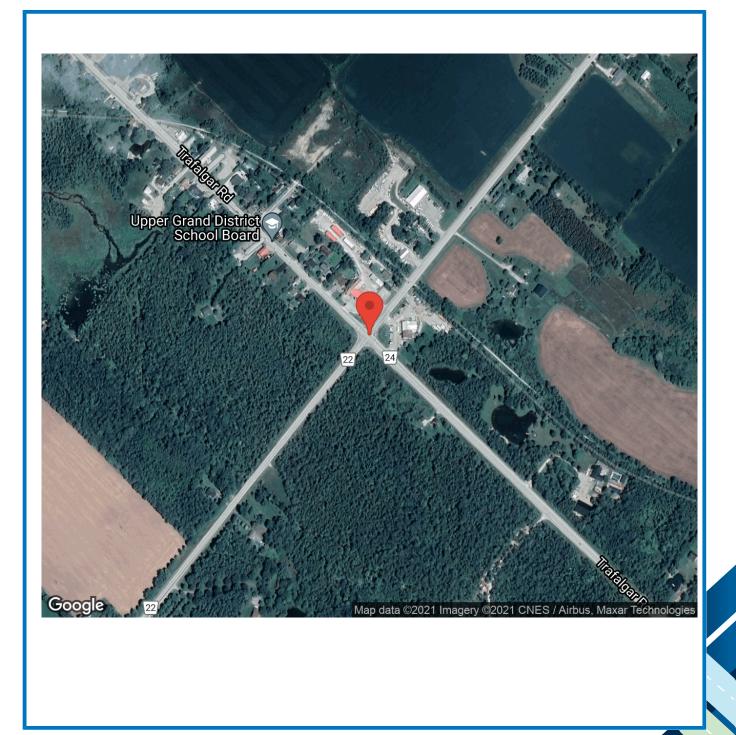
Intersection Count Report

Intersection:	Trafalgar Rd N & Wellington Rd 22				
Municipality:	Erin				
Count Date:	Oct 28, 2021				
Site Code:	2121900003				
Count Categories:	Cars, Trucks, Bicycles, Pedestrians				
Count Period:	07:00-10:00, 15:00-18:00				
Weather:	Clear				



Traffic Count Map

Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Municipality:	Erin
Count Date:	Oct 28, 2021





Traffic Count Summary

Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Municipality:	Erin
Count Date:	Oct 28, 2021

Trafalgar Rd N - Traffic Summary

		North Approach Totals						South Approach Totals					
		Include	s Cars, 1	Frucks, B	icycles		Includes Cars, Trucks, Bicycles						
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	53	176	24	0	253	0	3	85	27	0	115	0	368
08:00 - 09:00	59	148	19	0	226	0	12	110	41	0	163	0	389
09:00 - 10:00	33	117	21	0	171	0	5	124	25	0	154	0	325
					В	REAK							
15:00 - 16:00	60	137	25	0	222	0	23	257	46	0	326	0	548
16:00 - 17:00	50	138	22	0	210	0	22	283	68	0	373	0	583
17:00 - 18:00	21	136	22	0	179	0	20	296	59	0	375	0	554
GRAND TOTAL	276	852	133	0	1261	0	85	1155	266	0	1506	0	2767



Traffic Count Summary

Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Municipality:	Erin
Count Date:	Oct 28, 2021

Wellington Rd 22 - Traffic Summary

		East Approach Totals						West Approach Totals						
		Include	s Cars, 1	ſ <mark>rucks, B</mark> i	cycles		Includes Cars, Trucks, Bicycles							
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total	
07:00 - 08:00	34	34	19	0	87	0	15	36	23	0	74	0	161	
08:00 - 09:00	43	41	27	0	111	0	26	55	26	0	107	0	218	
09:00 - 10:00	26	32	42	0	100	0	13	33	7	0	53	0	153	
					В	REAK								
15:00 - 16:00	38	55	94	0	187	0	24	28	15	0	67	0	254	
16:00 - 17:00	43	62	81	0	186	0	44	54	11	0	109	0	295	
17:00 - 18:00	32	56	70	0	158	0	43	33	4	0	80	0	238	
GRAND TOTAL	216	280	333	0	829	0	165	239	86	0	490	0	1319	



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars				TI	rucks				Bi	cycles			
Start Time	•	1		1	Total	•	1		n	Total	-	1		1	Total	Total Peds
07:00	5	40	0	0	45	0	0	0	0	0	0	0	0	0	0	0
07:15	8	39	9	0	56	1	0	1	0	2	0	0	0	0	0	0
07:30	15	33	9	0	57	1	6	2	0	9	0	0	0	0	0	0
07:45	18	51	3	0	72	5	7	0	0	12	0	0	0	0	0	0
08:00	9	34	2	0	45	4	3	1	0	8	0	0	0	0	0	0
08:15	11	38	4	0	53	4	6	1	0	11	0	0	0	0	0	0
08:30	13	21	7	0	41	1	3	1	0	5	0	0	0	0	0	0
08:45	12	40	3	0	55	5	3	0	0	8	0	0	0	0	0	0
09:00	10	20	3	0	33	1	1	0	0	2	0	0	0	0	0	0
09:15	10	31	5	0	46	0	6	0	0	6	0	0	0	0	0	0
09:30	7	29	5	0	41	0	2	1	0	3	0	0	0	0	0	0
09:45	4	25	7	0	36	1	3	0	0	4	0	0	0	0	0	0
SUBTOTAL	122	401	57	0	580	23	40	7	0	70	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars				T	rucks				Bi	cycles			
Start Time	4	1		9	Total	4	1		9	Total	4	1		9	Total	Total Peds
15:00	14	31	1	0	46	6	6	0	0	12	0	0	0	0	0	0
15:15	14	29	11	0	54	4	0	0	0	4	0	0	0	0	0	0
15:30	8	21	3	0	32	5	4	2	0	11	0	0	0	0	0	0
15:45	6	34	7	0	47	3	12	1	0	16	0	0	0	0	0	0
16:00	9	33	5	0	47	1	2	0	0	3	0	0	0	0	0	0
16:15	15	33	4	0	52	3	2	0	0	5	0	0	0	0	0	0
16:30	9	26	9	0	44	0	1	0	0	1	0	0	0	0	0	0
16:45	13	40	4	0	57	0	1	0	0	1	0	0	0	0	0	0
17:00	7	36	7	0	50	0	0	0	0	0	0	0	0	0	0	0
17:15	6	36	6	0	48	1	2	0	0	3	0	0	0	0	0	0
17:30	2	22	5	0	29	2	3	0	0	5	0	0	0	0	0	0
17:45	2	37	4	0	43	1	0	0	0	1	0	0	0	0	0	0
SUBTOTAL	105	378	66	0	549	26	33	3	0	62	0	0	0	0	0	0
GRAND TOTAL	227	779	123	0	1129	49	73	10	0	132	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

			-				_					-•				
			Cars	_			T	rucks	_			BI	cycles	_		
Start Time	•	1		J.	Total	- 1	1		J.	Total	- 📲 -	1		J.	Total	Total Peds
07:00	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0
07:15	0	19	3	0	22	0	6	2	0	8	0	0	0	0	0	0
07:30	1	26	6	0	33	0	4	2	0	6	0	0	0	0	0	0
07:45	2	21	13	0	36	0	4	1	0	5	0	0	0	0	0	0
08:00	2	18	12	0	32	0	6	0	0	6	0	0	0	0	0	0
08:15	4	15	11	0	30	0	5	1	0	6	0	0	0	0	0	0
08:30	2	25	6	0	33	1	3	2	0	6	0	0	0	0	0	0
08:45	3	34	7	0	44	0	4	2	0	6	0	0	0	0	0	0
09:00	1	21	6	0	28	0	2	0	0	2	0	0	0	0	0	0
09:15	2	25	5	0	32	0	9	1	0	10	0	0	0	0	0	0
09:30	0	24	4	0	28	0	7	1	0	8	0	0	0	0	0	0
09:45	2	29	6	0	37	0	7	2	0	9	0	0	0	0	0	0
SUBTOTAL	19	262	79	0	360	1	57	14	0	72	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				T	rucks				Bi	cycles			
Start Time	•	1		9	Total	-	1		2	Total	-	1		1	Total	Total Peds
15:00	4	43	11	0	58	1	6	1	0	8	0	0	0	0	0	0
15:15	6	74	7	0	87	1	4	0	0	5	0	0	0	0	0	0
15:30	7	53	12	0	72	2	3	2	0	7	0	0	0	0	0	0
15:45	2	67	12	0	81	0	7	1	0	8	0	0	0	0	0	0
16:00	5	59	10	0	74	0	5	3	0	8	0	0	0	0	0	0
16:15	6	74	18	0	98	3	2	0	0	5	0	0	0	0	0	0
16:30	4	69	14	0	87	1	3	0	0	4	0	0	0	0	0	0
16:45	3	70	23	0	96	0	1	0	0	1	0	0	0	0	0	0
17:00	7	81	13	0	101	1	3	0	0	4	0	0	0	0	0	0
17:15	5	79	13	0	97	1	3	1	0	5	0	0	0	0	0	0
17:30	3	55	16	0	74	0	4	0	0	4	0	0	0	0	0	0
17:45	3	69	16	0	88	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	55	793	165	0	1013	10	43	8	0	61	0	0	0	0	0	0
GRAND TOTAL	74	1055	244	0	1373	11	100	22	0	133	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

East Approach - Wellington Rd 22

		(Cars				TI	rucks				Bi	cycles			
Start Time	•	1		1	Total	-	1	•	n	Total	•	1		1	Total	Total Peds
07:00	0	6	0	0	6	0	0	0	0	0	0	0	0	0	0	0
07:15	10	8	4	0	22	0	0	3	0	3	0	0	0	0	0	0
07:30	9	8	5	0	22	2	1	1	0	4	0	0	0	0	0	0
07:45	11	11	6	0	28	2	0	0	0	2	0	0	0	0	0	0
08:00	14	7	6	0	27	5	0	4	0	9	0	0	0	0	0	0
08:15	9	11	7	0	27	2	1	1	0	4	0	0	0	0	0	0
08:30	5	10	5	0	20	1	1	1	0	3	0	0	0	0	0	0
08:45	4	10	2	0	16	3	1	1	0	5	0	0	0	0	0	0
09:00	6	7	10	0	23	2	1	1	0	4	0	0	0	0	0	0
09:15	7	5	8	0	20	0	1	1	0	2	0	0	0	0	0	0
09:30	3	7	7	0	17	0	1	1	0	2	0	0	0	0	0	0
09:45	5	6	11	0	22	3	4	3	0	10	0	0	0	0	0	0
SUBTOTAL	83	96	71	0	250	20	11	17	0	48	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

East Approach - Wellington Rd 22

			Cars				TI	rucks				Bi	cycles			
Start Time	-	1		9	Total		1		1	Total	4	+	•	9	Total	Total Peds
15:00	5	8	24	0	37	0	0	1	0	1	0	0	0	0	0	0
15:15	10	19	25	0	54	0	0	3	0	3	0	0	0	0	0	0
15:30	9	12	16	0	37	0	0	5	0	5	0	0	0	0	0	0
15:45	14	15	19	0	48	0	1	1	0	2	0	0	0	0	0	0
16:00	8	19	16	0	43	0	0	2	0	2	0	0	0	0	0	0
16:15	11	17	19	0	47	1	0	0	0	1	0	0	0	0	0	0
16:30	8	10	16	0	34	0	0	2	0	2	0	0	0	0	0	0
16:45	14	16	26	0	56	1	0	0	0	1	0	0	0	0	0	0
17:00	12	14	23	0	49	0	1	0	0	1	0	0	0	0	0	0
17:15	9	14	19	0	42	0	0	1	0	1	0	0	0	0	0	0
17:30	6	17	18	0	41	0	0	0	0	0	0	0	0	0	0	0
17:45	5	10	9	0	24	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	111	171	230	0	512	2	2	15	0	19	0	0	0	0	0	0
GRAND TOTAL	194	267	301	0	762	22	13	32	0	67	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

West Approach - Wellington Rd 22

		(Cars				TI	rucks				Bi	cycles			
Start Time	-	1		1	Total	•	1		n	Total	•	1		1	Total	Total Peds
07:00	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
07:15	1	8	8	0	17	2	0	0	0	2	0	0	0	0	0	0
07:30	1	14	5	0	20	1	0	1	0	2	0	0	0	0	0	0
07:45	10	14	7	0	31	0	0	0	0	0	0	0	0	0	0	0
08:00	3	19	9	0	31	0	1	0	0	1	0	0	0	0	0	0
08:15	6	8	5	0	19	1	0	0	0	1	0	0	0	0	0	0
08:30	4	10	6	0	20	0	1	0	0	1	0	0	0	0	0	0
08:45	9	16	6	0	31	3	0	0	0	3	0	0	0	0	0	0
09:00	1	7	3	0	11	2	0	1	0	3	0	0	0	0	0	0
09:15	5	9	2	0	16	0	1	0	0	1	0	0	0	0	0	0
09:30	2	4	0	0	6	0	0	0	0	0	0	0	0	0	0	0
09:45	3	12	1	0	16	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	45	121	54	0	220	9	3	2	0	14	0	0	0	0	0	0



Trafalgar Rd N & Wellington Rd 22
2121900003
Erin
Oct 28, 2021

West Approach - Wellington Rd 22

		(Cars				T	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	1		9	Total	-	1		J.	Total	Total Peds
15:00	4	5	3	0	12	0	0	2	0	2	0	0	0	0	0	0
15:15	7	7	1	0	15	1	0	0	0	1	0	0	0	0	0	0
15:30	5	8	4	0	17	0	0	0	0	0	0	0	0	0	0	0
15:45	6	7	3	0	16	1	1	2	0	4	0	0	0	0	0	0
16:00	12	12	3	0	27	1	0	1	0	2	0	0	0	0	0	0
16:15	11	12	1	0	24	0	0	0	0	0	0	0	0	0	0	0
16:30	6	14	3	0	23	0	0	0	0	0	0	0	0	0	0	0
16:45	14	15	2	0	31	0	1	1	0	2	0	0	0	0	0	0
17:00	9	8	0	0	17	0	0	0	0	0	0	0	0	0	0	0
17:15	9	8	2	0	19	0	0	0	0	0	0	0	0	0	0	0
17:30	12	4	0	0	16	0	1	1	0	2	0	0	0	0	0	0
17:45	13	12	0	0	25	0	0	1	0	1	0	0	0	0	0	0
SUBTOTAL	108	112	22	0	242	3	3	8	0	14	0	0	0	0	0	0
GRAND TOTAL	153	233	76	0	462	12	6	10	0	28	0	0	0	0	0	0



Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Count Date:	Oct 28, 2021

** Signalized Intersection **

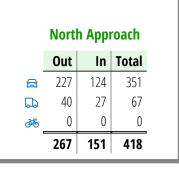
Peak Hour Diagram

Clear

Specified Pe	eriod	One Hour Peak							
From:	07:00:00	From:	07:30:00						
To:	10:00:00	To:	08:30:00						

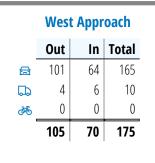
Weather conditions:

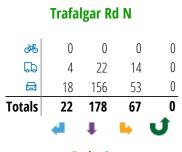
Major Road: Trafalgar Rd N runs N/S



Wellington Rd 22

	Totals	æ		්
7	0	0	0	0
4	22	20	2	0
-	56	55	1	0
4	27	26	1	0





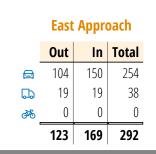




Peds: 0



	4	t	•	J.				
Totals	9	99	46	0				
	9	80	42	0				
D	0	19	4	0				
æ	0	0	0	0				
Trafalgar Rd N								



Wellington Rd 22

	Totals		G	්
C	0	0	0	0
t	30	24	6	0
-	39	37	2	0
	54	43	11	0

	South Approach										
	Out	In	Total								
	131	225	356								
B	23	34	57								
ණ්	0	0	0								
	154	259	413								

🖻 - Cars

🗔 - Trucks

Peds: 0

💑 - Bicycles

Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Count Date:	Oct 28, 2021
Period:	07:00 - 10:00

Peak Hour Data (07:30 - 08:30)

		North Approach South Approach Trafalgar Rd N Trafalgar Rd N							N	East Ap /ellingt	oproach con Rd 2	1 22		West Approach Wellington Rd 22						Total					
Start Time	4	t		J	Peds	Total	•	t	•	ŋ	Peds	Total	•	t		ŋ	Peds	Total	•	t		ŋ	Peds	Total	Vehicl es
07:30	16	39	11	0	0	66	1	30	8	0	0	39	11	9	6	0	0	26	2	14	6	0	0	22	153
07:45	23	58	3	0	0	84	2	25	14	0	0	41	13	11	6	0	0	30	10	14	7	0	0	31	186
08:00	13	37	3	0	0	53	2	24	12	0	0	38	19	7	10	0	0	36	3	20	9	0	0	32	159
08:15	15	44	5	0	0	64	4	20	12	0	0	36	11	12	8	0	0	31	7	8	5	0	0	20	151
Grand Total	67	178	22	0	0	267	9	99	46	0	0	154	54	39	30	0	0	123	22	56	27	0	0	105	649
Approach %	25.1	66.7	8.2	0		-	5.8	64.3	29.9	0		-	43.9	31.7	24.4	0		-	21	53.3	25.7	0		-	
Totals %	10.3	27.4	3.4	0		41.1	1.4	15.3	7.1	0		23.7	8.3	6	4.6	0		19	3.4	8.6	4.2	0		16.2	
PHF	0.73	0.77	0.5	0		0.79	0.56	0.83	0.82	0		0.94	0.71	0.81	0.75	0		0.85	0.55	0.7	0.75	0		0.82	0.87
Cars	53	156	18	0		227	9	80	42	0		131	43	37	24	0		104	20	55	26	0		101	563
% Cars	79.1	87.6	81.8	0		85	100	80.8	91.3	0		85.1	79.6	94.9	80	0		84.6	90.9	98.2	96.3	0		96.2	86.7
Trucks	14	22	4	0		40	0	19	4	0		23	11	2	6	0		19	2	1	1	0		4	86
% Trucks	20.9	12.4	18.2	0		15	0	19.2	8.7	0		14.9	20.4	5.1	20	0		15.4	9.1	1.8	3.7	0		3.8	13.3
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Count Date:	Oct 28, 2021

** Signalized Intersection **

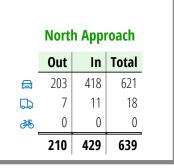
Peak Hour Diagram

Specified Pe	eriod	One Hour P	eak
From:	15:00:00	From:	16:15:00
To:	18:00:00	To:	17:15:00

Weather conditions:

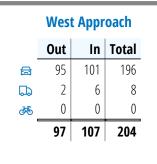
Clear

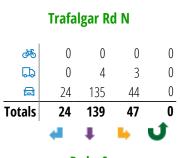
Major Road: Trafalgar Rd N runs N/S



Wellington Rd 22

	Totals	ß		්
7	0	0	0	0
1	40	40	0	0
-	50	49	1	0
4	7	6	1	0









Peds: 0



	•	t	•	J.
Totals	25	303	68	0
æ	20	294	68	0
<u>ل</u>	5	9	0	0
æ	0	0	0	0
	Trafa	lgar R	d N	

	East Approach												
	Out	In	Total										
⊟	186	161	347										
	5	4	9										
්	0	0	0										
	191	165	356										

Wellington Rd 22

	Totals		G	්
C	0	0	0	0
t	86	84	2	0
-	58	57	1	0
	47	45	2	0

	Sout	h Appı	roach
	Out	In	Total
	382	186	568
B	14	7	21
ණ්	0	0	0
	396	193	589

🖻 - Cars

🖵 - Trucks

Peds: 0

💑 - Bicycles

Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Wellington Rd 22
Site Code:	2121900003
Count Date:	Oct 28, 2021
Period:	15:00 - 18:00

Peak Hour Data (16:15 - 17:15)

			North A Trafalg					5	outh A Trafalg	approac ar Rd N	:h N			v	East Ap /ellingt	oproact on Rd :	า 22			V	West Ap Vellingt	oproach on Rd 2	1 22		Total
Start Time	4	t		ŋ	Peds	Total	4	t		J	Peds	Total	-	t		ŋ	Peds	Total	•	t		ŋ	Peds	Total	Vehicl es
16:15	18	35	4	0	0	57	9	76	18	0	0	103	12	17	19	0	0	48	11	12	1	0	0	24	232
16:30	9	27	9	0	0	45	5	72	14	0	0	91	8	10	18	0	0	36	6	14	3	0	0	23	195
16:45	13	41	4	0	0	58	3	71	23	0	0	97	15	16	26	0	0	57	14	16	3	0	0	33	245
17:00	7	36	7	0	0	50	8	84	13	0	0	105	12	15	23	0	0	50	9	8	0	0	0	17	222
Grand Total	47	139	24	0	0	210	25	303	68	0	0	396	47	58	86	0	0	191	40	50	7	0	0	97	894
Approach %	22.4	66.2	11.4	0		-	6.3	76.5	17.2	0		-	24.6	30.4	45	0		-	41.2	51.5	7.2	0		-	
Totals %	5.3	15.5	2.7	0		23.5	2.8	33.9	7.6	0		44.3	5.3	6.5	9.6	0		21.4	4.5	5.6	0.8	0		10.9	
PHF	0.65	0.85	0.67	0		0.91	0.69	0.9	0.74	0		0.94	0.78	0.85	0.83	0		0.84	0.71	0.78	0.58	0		0.73	0.91
Cars	44	135	24	0		203	20	294	68	0		382	45	57	84	0		186	40	49	6	0		95	866
% Cars	93.6	97.1	100	0		96.7	80	97	100	0		96.5	95.7	98.3	97.7	0		97.4	100	98	85.7	0		97.9	96.9
Trucks	3	4	0	0		7	5	9	0	0		14	2	1	2	0		5	0	1	1	0		2	28
% Trucks	6.4	2.9	0	0		3.3	20	3	0	0		3.5	4.3	1.7	2.3	0		2.6	0	2	14.3	0		2.1	3.1
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Project #21-219 - Candevcon Limited

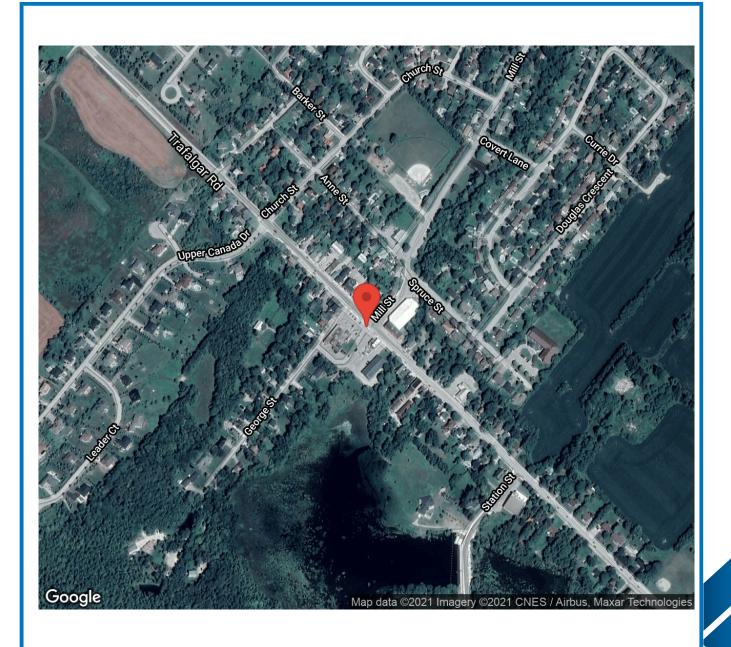
Intersection Count Report

Intersection:	Trafalgar Rd N & Mill St-George St						
Municipality:	Erin						
Count Date:	Oct 28, 2021						
Site Code:	2121900004						
Count Categories:	Cars, Trucks, Bicycles, Pedestrians						
Count Period:	07:00-10:00, 15:00-18:00						
Weather:	Clear						



Traffic Count Map

Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Municipality:	Erin
Count Date:	Oct 28, 2021





Traffic Count Summary

Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Municipality:	Erin
Count Date:	Oct 28, 2021

Trafalgar Rd N - Traffic Summary

		North	Appr	oach T	otals		South Approach Totals						
		Include	s Cars, 1	Frucks, B	icycles			Include	s Cars, 1	rucks, B	icycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	2	219	3	0	224	0	4	119	4	0	127	1	351
08:00 - 09:00	8	176	0	0	184	0	14	120	10	0	144	1	328
09:00 - 10:00	5	127	3	0	135	0	18	134	15	0	167	0	302
					В	REAK							
15:00 - 16:00	15	167	5	0	187	1	61	224	40	0	325	0	512
16:00 - 17:00	11	156	4	0	171	0	51	286	38	0	375	0	546
17:00 - 18:00	14	124	2	0	140	0	50	284	41	0	375	2	515
GRAND TOTAL	55	969	17	0	1041	1	198	1167	148	0	1513	4	2554



Traffic Count Summary

Intersection:	Trafalgar R
Site Code:	212190000
Municipality:	Erin
Count Date:	Oct 28, 202

Trafalgar Rd N & Mill St-George St 2121900004 Erin Oct 28, 2021

Mill St - Traffic Summary

		East	Appro	ach To	tals								
		Include	s Cars, 1	Frucks, Bi	icycles			Include	s Cars, 1	Frucks, Bi	cycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	17	0	7	0	24	0	2	2	8	0	12	0	36
08:00 - 09:00	29	3	7	0	39	3	7	1	8	0	16	0	55
09:00 - 10:00	19	5	5	0	29	0	2	2	23	0	27	0	56
	-				В	REAK							
15:00 - 16:00	24	10	13	0	47	0	20	9	44	0	73	0	120
16:00 - 17:00	25	13	13	0	51	3	20	8	30	0	58	5	109
17:00 - 18:00	14	6	8	0	28	0	14	16	29	0	59	2	87
GRAND TOTAL	128	37	53	0	218	6	65	38	142	0	245	7	463



Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Municipality:	Erin
Count Date:	Oct 28, 2021

North Approach - Trafalgar Rd N

			C				τ.					D:				
			Cars	-			11	rucks	-			BI	cycles	-		
Start Time	•			n	Total	•			J.	Total	•			n.	Total	Total Peds
07:00	1	25	2	0	28	0	0	0	0	0	0	0	0	0	0	0
07:15	0	57	0	0	57	0	8	0	0	8	0	0	0	0	0	0
07:30	1	52	0	0	53	0	6	0	0	6	0	0	0	0	0	0
07:45	0	59	1	0	60	0	12	0	0	12	0	0	0	0	0	0
08:00	1	43	0	0	44	0	7	0	0	7	0	0	0	0	0	0
08:15	2	32	0	0	34	0	7	0	0	7	0	0	0	0	0	0
08:30	2	35	0	0	37	0	10	0	0	10	0	0	0	0	0	0
08:45	3	36	0	0	39	0	6	0	0	6	0	0	0	0	0	0
09:00	0	24	1	0	25	0	6	0	0	6	0	0	0	0	0	0
09:15	2	27	1	0	30	1	3	0	0	4	0	0	0	0	0	0
09:30	2	30	1	0	33	0	7	0	0	7	0	0	0	0	0	0
09:45	0	27	0	0	27	0	3	0	0	3	0	0	0	0	0	0
SUBTOTAL	14	447	6	0	467	1	75	0	0	76	0	0	0	0	0	0



Trafalgar Rd N & Mill St-George St
2121900004
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars				T	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	1		9	Total	-	1		9	Total	Total Peds
15:00	3	38	2	0	43	0	9	0	0	9	0	0	0	0	0	0
15:15	3	35	0	0	38	0	5	0	0	5	0	0	0	0	0	0
15:30	5	22	0	0	27	0	6	0	0	6	0	0	0	0	0	1
15:45	4	40	3	0	47	0	12	0	0	12	0	0	0	0	0	0
16:00	6	44	3	0	53	0	4	0	0	4	0	0	0	0	0	0
16:15	0	45	0	0	45	0	2	0	0	2	0	0	0	0	0	0
16:30	3	31	0	0	34	0	1	0	0	1	0	0	0	0	0	0
16:45	2	28	1	0	31	0	1	0	0	1	0	0	0	0	0	0
17:00	4	35	0	0	39	0	1	0	0	1	0	0	0	0	0	0
17:15	7	35	0	0	42	0	1	0	0	1	0	0	0	0	0	0
17:30	3	27	1	0	31	0	1	0	0	1	0	0	0	0	0	0
17:45	0	23	1	0	24	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	40	403	11	0	454	0	44	0	0	44	0	0	0	0	0	1
GRAND TOTAL	54	850	17	0	921	1	119	0	0	120	0	0	0	0	0	1



Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Municipality:	Erin
Count Date:	Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				Tr	rucks				Ri	cycles			
Start Time	-	1		9	Total	4	1		9	Total	-	1		9	Total	Total Peds
07:00	0	17	0	0	17	0	0	0	0	0	0	0	0	0	0	0
07:15	0	28	2	0	30	0	6	0	0	6	0	0	0	0	0	1
07:30	0	27	0	0	27	0	6	0	0	6	0	0	0	0	0	0
07:45	4	28	1	0	33	0	7	1	0	8	0	0	0	0	0	0
08:00	2	21	6	0	29	0	10	0	0	10	0	0	0	0	0	0
08:15	3	19	0	0	22	0	8	0	0	8	0	0	0	0	0	0
08:30	4	26	1	0	31	0	5	0	0	5	0	0	0	0	0	1
08:45	5	25	3	0	33	0	6	0	0	6	0	0	0	0	0	0
09:00	7	22	4	0	33	0	7	0	0	7	0	0	0	0	0	0
09:15	5	23	2	0	30	1	8	2	0	11	0	0	0	0	0	0
09:30	2	28	0	0	30	0	5	0	0	5	0	0	0	0	0	0
09:45	3	30	6	0	39	0	11	1	0	12	0	0	0	0	0	0
SUBTOTAL	35	294	25	0	354	1	79	4	0	84	0	0	0	0	0	2



Trafalgar Rd N & Mill St-George St
2121900004
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

	Cars						T	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	1		1	Total	•	1		1	Total	Total Peds
15:00	12	46	10	0	68	0	8	0	0	8	0	0	0	0	0	0
15:15	19	49	7	0	75	0	6	0	0	6	0	0	0	0	0	0
15:30	14	50	8	0	72	1	5	2	0	8	0	0	0	0	0	0
15:45	15	56	12	0	83	0	4	1	0	5	0	0	0	0	0	0
16:00	11	61	15	0	87	0	9	0	0	9	0	0	0	0	0	0
16:15	13	75	14	0	102	2	1	0	0	3	0	0	0	0	0	0
16:30	15	73	3	0	91	0	5	0	0	5	0	0	0	0	0	0
16:45	10	61	6	0	77	0	1	0	0	1	0	0	0	0	0	0
17:00	15	74	17	0	106	0	2	0	0	2	0	0	0	0	0	0
17:15	11	80	9	0	100	0	5	0	0	5	0	0	0	0	0	0
17:30	11	56	7	0	74	0	3	0	0	3	0	0	0	0	0	2
17:45	13	61	8	0	82	0	3	0	0	3	0	0	0	0	0	0
SUBTOTAL	159	742	116	0	1017	3	52	3	0	58	0	0	0	0	0	2
GRAND TOTAL	194	1036	141	0	1371	4	131	7	0	142	0	0	0	0	0	4



Trafalgar Rd N & Mill St-George St
2121900004
Erin
Oct 28, 2021

East Approach - Mill St

			Cars				T	rucks				Bi	cycles			
Start Time	-	1		1	Total	-	1		n	Total	•	1		1	Total	Total Peds
07:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
07:15	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0
07:30	8	0	3	0	11	0	0	0	0	0	0	0	0	0	0	0
07:45	3	0	4	0	7	0	0	0	0	0	0	0	0	0	0	0
08:00	10	0	3	0	13	2	0	1	0	3	0	0	0	0	0	0
08:15	3	0	1	0	4	1	0	0	0	1	0	0	0	0	0	0
08:30	7	2	0	0	9	1	0	0	0	1	0	0	0	0	0	3
08:45	5	1	1	0	7	0	0	1	0	1	0	0	0	0	0	0
09:00	6	0	2	0	8	0	0	0	0	0	0	0	0	0	0	0
09:15	5	2	1	0	8	0	0	0	0	0	0	0	0	0	0	0
09:30	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0
09:45	4	2	2	0	8	1	0	0	0	1	0	0	0	0	0	0
SUBTOTAL	60	8	17	0	85	5	0	2	0	7	0	0	0	0	0	3



Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Municipality:	Erin
Count Date:	Oct 28, 2021

East Approach - Mill St

		1	Cars				TI	rucks				Bi	cycles			
Start Time	•	1		1	Total	-	1		n	Total	•	1		1	Total	Total Peds
15:00	6	1	3	0	10	1	0	0	0	1	0	0	0	0	0	0
15:15	8	5	2	0	15	0	0	0	0	0	0	0	0	0	0	0
15:30	1	2	3	0	6	1	0	0	0	1	0	0	0	0	0	0
15:45	4	2	5	0	11	3	0	0	0	3	0	0	0	0	0	0
16:00	6	3	6	0	15	0	0	0	0	0	0	0	0	0	0	0
16:15	6	5	2	0	13	1	0	0	0	1	0	0	0	0	0	3
16:30	7	1	5	0	13	0	0	0	0	0	0	0	0	0	0	0
16:45	5	4	0	0	9	0	0	0	0	0	0	0	0	0	0	0
17:00	4	1	3	0	8	0	0	0	0	0	0	0	0	0	0	0
17:15	3	3	3	0	9	0	0	0	0	0	0	0	0	0	0	0
17:30	1	1	2	0	4	0	0	0	0	0	0	0	0	0	0	0
17:45	6	1	0	0	7	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	57	29	34	0	120	6	0	0	0	6	0	0	0	0	0	3
GRAND TOTAL	117	37	51	0	205	11	0	2	0	13	0	0	0	0	0	6



Trafalgar Rd N & Mill St-George St
2121900004
Erin
Oct 28, 2021

West Approach - George St

			Cars				T	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	1		9	Total	-	1		1	Total	Total Peds
07:00	1	1	3	0	5	0	0	0	0	0	0	0	0	0	0	0
07:15	1	1	3	0	5	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
08:30	3	1	4	0	8	0	0	0	0	0	0	0	0	0	0	0
08:45	4	0	2	0	6	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	9	0	9	0	0	0	0	0	0	0	0	0	0	0
09:15	0	1	5	0	6	1	0	2	0	3	0	0	0	0	0	0
09:30	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0
09:45	1	1	4	0	6	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	10	5	36	0	51	1	0	3	0	4	0	0	0	0	0	0



Trafalgar Rd N & Mill St-George St
2121900004
Erin
Oct 28, 2021

West Approach - George St

			Cars				T	rucks				Bi	cycles			
Start Time		t		9	Total	-	+		1	Total	-	+		9	Total	Total Peds
15:00	9	1	14	0	24	0	0	1	0	1	0	0	0	0	0	0
15:15	5	1	9	0	15	0	0	0	0	0	0	0	0	0	0	0
15:30	1	2	10	0	13	0	0	0	0	0	2	0	0	0	2	0
15:45	2	5	10	0	17	1	0	0	0	1	0	0	0	0	0	0
16:00	4	2	11	0	17	0	0	0	0	0	2	0	0	0	2	0
16:15	4	6	8	0	18	0	0	0	0	0	0	0	0	0	0	3
16:30	4	0	7	0	11	2	0	0	0	2	0	0	0	0	0	2
16:45	4	0	4	0	8	0	0	0	0	0	0	0	0	0	0	0
17:00	2	10	11	0	23	0	0	0	0	0	0	0	0	0	0	0
17:15	9	2	10	0	21	0	0	0	0	0	0	0	0	0	0	0
17:30	1	2	3	0	6	0	0	0	0	0	0	0	0	0	0	2
17:45	2	2	5	0	9	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	47	33	102	0	182	3	0	1	0	4	4	0	0	0	4	7
GRAND TOTAL	57	38	138	0	233	4	0	4	0	8	4	0	0	0	4	7



Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Count Date:	Oct 28, 2021

** Unsignalized Intersection **

Peak Hour Diagram

Specified Pe	eriod	One Hour Peak						
From:	07:00:00	From:	07:15:00					
To:	10:00:00	To:	08:15:00					

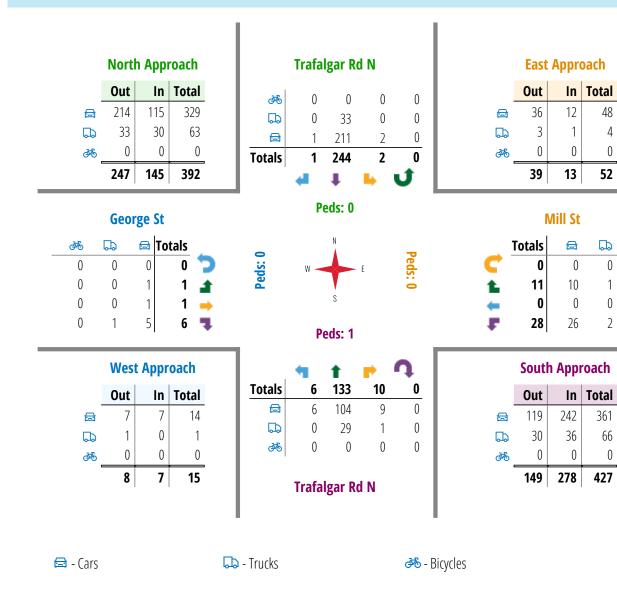
Weather conditions:

Clear

Major Road: Trafalgar Rd N runs N/S

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Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Count Date:	Oct 28, 2021
Period:	07:00 - 10:00

Peak Hour Data (07:15 - 08:15)

		ľ	North A Trafalg	oproac ar Rd N	ih N			S	outh A Trafalg	approac ar Rd N	:h N				East Ap Mil	oproacł II St	ı			١	Nest Aj Geor	oproacł ge St	1		Total Vehicl
Start Time	•	1		9	Peds	Total	•	1		J	Peds	Total	•	t		J	Peds	Total	•	1		J	Peds	Total	es
07:15	0	65	0	0	0	65	0	34	2	0	1	36	5	0	0	0	0	5	1	1	3	0	0	5	111
07:30	1	58	0	0	0	59	0	33	0	0	0	33	8	0	3	0	0	11	0	0	2	0	0	2	105
07:45	0	71	1	0	0	72	4	35	2	0	0	41	3	0	4	0	0	7	0	0	0	0	0	0	120
08:00	1	50	0	0	0	51	2	31	6	0	0	39	12	0	4	0	0	16	0	0	1	0	0	1	107
Grand Total	2	244	1	0	0	247	6	133	10	0	1	149	28	0	11	0	0	39	1	1	6	0	0	8	443
Approach %	0.8	98.8	0.4	0		-	4	89.3	6.7	0		-	71.8	0	28.2	0		-	12.5	12.5	75	0		-	
Totals %	0.5	55.1	0.2	0		55.8	1.4	30	2.3	0		33.6	6.3	0	2.5	0		8.8	0.2	0.2	1.4	0		1.8	
PHF	0.5	0.86	0.25	0		0.86	0.38	0.95	0.42	0		0.91	0.58	0	0.69	0		0.61	0.25	0.25	0.5	0		0.4	0.92
Cars	2	211	1	0		214	6	104	9	0		119	26	0	10	0		36	1	1	5	0		7	376
% Cars	100	86.5	100	0		86.6	100	78.2	90	0		79.9	92.9	0	90.9	0		92.3	100	100	83.3	0		87.5	84.9
Trucks	0	33	0	0		33	0	29	1	0		30	2	0	1	0		3	0	0	1	0		1	67
% Trucks	0	13.5	0	0		13.4	0	21.8	10	0		20.1	7.1	0	9.1	0		7.7	0	0	16.7	0		12.5	15.1
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					1	-					0	-					0	-	1
% Peds					0	-					100	-					0	-					0	-	



Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Count Date:	Oct 28, 2021

Peak Hour Diagram

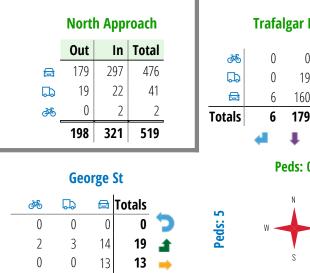
Specified Pe	eriod	One Hour Peak						
From:	15:00:00	From:	15:45:00					
To:	18:00:00	To:	16:45:00					

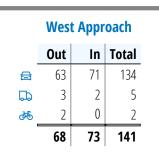
Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Trafalgar Rd N runs N/S

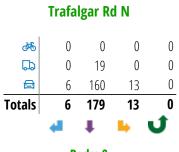




36

36

3







Peds: 3



	•	t	•	ŋ			
Totals	56	284	45	0			
⊟	54	265	44	0			
B	2	19	1	0			
්රි	0	0	0	0			
Trafalgar Rd N							

	East Approach										
	Out	In	Total								
	52	70	122								
B	4	1	5								
Ā	0	0	0								
	56	71	127								

Mill St Totals Ð G ക് 0 0 0 0 18 18 0 0 0 11 0 11 27 23 4 0

	South Approach										
	Out	In	Total								
	363	219	582								
D	22	23	45								
æ	0	0	0								
	385	242	627								



0

0

🗔 - Trucks

💑 - Bicycles

Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Mill St-George St
Site Code:	2121900004
Count Date:	Oct 28, 2021
Period:	15:00 - 18:00

Peak Hour Data (15:45 - 16:45)

		Ņ	lorth A Trafalg	Approac gar Rd N	ih N			S	outh A Trafalg	pproac ar Rd N	h I				East Ap Mi	oproach Il St	1				West Aj Geor	oproac ge St	h		Total Vehicl
Start Time	•	1	•	J	Peds	Total	•	1	•	J	Peds	Total	•	1	•	J	Peds	Total	•	1	•	9	Peds	Total	es
15:45	4	52	3	0	0	59	15	60	13	0	0	88	7	2	5	0	0	14	3	5	10	0	0	18	179
16:00	6	48	3	0	0	57	11	70	15	0	0	96	6	3	6	0	0	15	6	2	11	0	0	19	187
16:15	0	47	0	0	0	47	15	76	14	0	0	105	7	5	2	0	3	14	4	6	8	0	3	18	184
16:30	3	32	0	0	0	35	15	78	3	0	0	96	7	1	5	0	0	13	6	0	7	0	2	13	157
Grand Total	13	179	6	0	0	198	56	284	45	0	0	385	27	11	18	0	3	56	19	13	36	0	5	68	707
Approach %	6.6	90.4	3	0		-	14.5	73.8	11.7	0		-	48.2	19.6	32.1	0		-	27.9	19.1	52.9	0		-	
Totals %	1.8	25.3	0.8	0		28	7.9	40.2	6.4	0		54.5	3.8	1.6	2.5	0		7.9	2.7	1.8	5.1	0		9.6	
PHF	0.54	0.86	0.5	0		0.84	0.93	0.91	0.75	0		0.92	0.96	0.55	0.75	0		0.93	0.79	0.54	0.82	0		0.89	0.95
Cars	13	160	6	0		179	54	265	44	0		363	23	11	18	0		52	14	13	36	0		63	657
% Cars	100	89.4	100	0		90.4	96.4	93.3	97.8	0		94.3	85.2	100	100	0		92.9	73.7	100	100	0		92.6	92.9
Trucks	0	19	0	0		19	2	19	1	0		22	4	0	0	0		4	3	0	0	0		3	48
% Trucks	0	10.6	0	0		9.6	3.6	6.7	2.2	0		5.7	14.8	0	0	0		7.1	15.8	0	0	0		4.4	6.8
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	2	0	0	0		2	2
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	10.5	0	0	0		2.9	0.3
Peds					0	-					0	-					3	-					5	-	8
% Peds					0	-					0	-					37.5	-					62.5	-	



Project #21-219 - Candevcon Limited

St

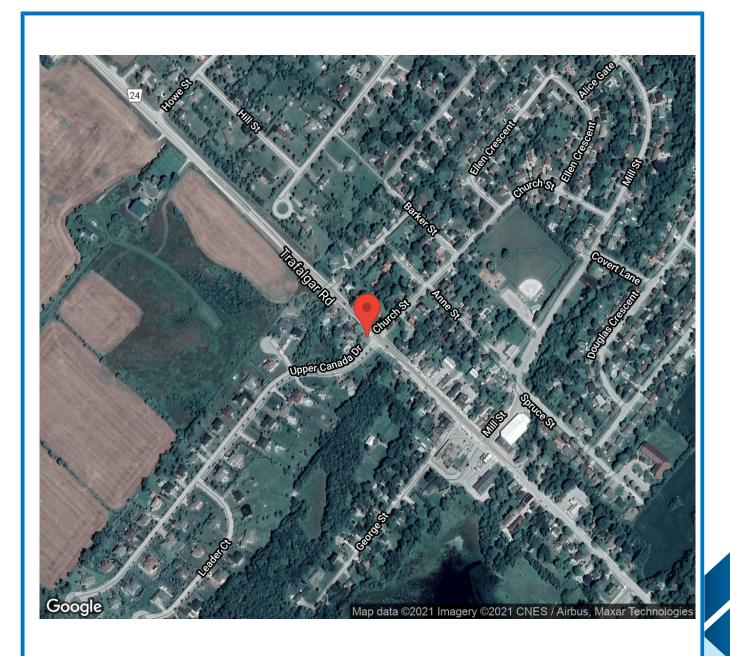
Intersection Count Report

Intersection:	Trafalgar Rd N & Upper Canada Dr-Church
Municipality:	Erin
Count Date:	Oct 28, 2021
Site Code:	2121900002
Count Categories:	Cars, Trucks, Bicycles, Pedestrians
Count Period:	07:00-10:00, 15:00-18:00
Weather:	Clear



Traffic Count Map

Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021



Traffic Count Summary



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

Trafalgar Rd N - Traffic Summary

		North	Appr	oach T	otals								
		Include	s Cars, 1	Frucks, Bi	cycles			Include	s Cars, 1	Frucks, B	icycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	2	200	1	0	203	0	1	120	4	0	125	4	328
08:00 - 09:00	1	142	1	0	144	2	7	107	5	0	119	0	263
09:00 - 10:00	4	103	1	0	108	0	1	118	6	0	125	2	233
					В	REAK							
15:00 - 16:00	4	179	3	0	186	0	8	245	15	0	268	1	454
16:00 - 17:00	6	190	2	0	198	1	16	325	10	0	351	5	549
17:00 - 18:00	7	139	2	0	148	0	13	300	9	0	322	1	470
GRAND TOTAL	24	953	10	0	987	3	46	1215	49	0	1310	13	2297

Traffic Count Summary



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

Church St - Traffic Summary

		East	Appro	ach To	tals		West Approach Totals						
		Include	s Cars, 1	Trucks, Bi	icycles			Include	s Cars, 1	Frucks, Bi	cycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	8	0	2	0	10	0	4	0	13	0	17	0	27
08:00 - 09:00	7	1	5	0	13	0	3	0	10	0	13	5	26
09:00 - 10:00	8	0	4	0	12	1	2	0	8	0	10	1	22
					В	REAK							
15:00 - 16:00	10	7	5	0	22	0	3	3	6	0	12	0	34
16:00 - 17:00	6	1	3	0	10	1	1	1	2	0	4	0	14
17:00 - 18:00	8	1	4	0	13	0	6	0	6	0	12	0	25
GRAND TOTAL	47	10	23	0	80	2	19	4	45	0	68	6	148



Trafalgar Rd N & Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars				T	rucks				Bi	cycles			
Start Time	-	t	P	9	Total	-	1	P	0	Total	-	1		9	Total	Total Peds
07:00	0	40	0	0	40	0	4	0	0	4	0	0	0	0	0	0
07:15	2	42	0	0	44	0	4	0	0	4	0	0	0	0	0	0
07:30	0	41	0	0	41	0	5	0	0	5	0	0	0	0	0	0
07:45	0	51	1	0	52	0	13	0	0	13	0	0	0	0	0	0
08:00	0	33	0	0	33	0	7	0	0	7	0	0	0	0	0	0
08:15	0	28	1	0	29	0	6	0	0	6	0	0	0	0	0	0
08:30	0	27	0	0	27	1	7	0	0	8	0	0	0	0	0	1
08:45	0	28	0	0	28	0	6	0	0	6	0	0	0	0	0	1
09:00	0	14	0	0	14	0	4	0	0	4	0	0	0	0	0	0
09:15	1	23	0	0	24	0	6	0	0	6	0	0	0	0	0	0
09:30	1	25	1	0	27	0	6	0	0	6	0	0	0	0	0	0
09:45	2	22	0	0	24	0	3	0	0	3	0	0	0	0	0	0
SUBTOTAL	6	374	3	0	383	1	71	0	0	72	0	0	0	0	0	2



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars				TI	rucks				Bi	cycles			
Start Time	-	+		9	Total	-	+		9	Total	-	+	•	9	Total	Total Peds
15:00	4	31	1	0	36	0	6	0	0	6	0	0	0	0	0	0
15:15	0	37	0	0	37	0	5	0	0	5	0	0	0	0	0	0
15:30	0	32	1	0	33	0	7	0	0	7	0	0	0	0	0	0
15:45	0	50	1	0	51	0	11	0	0	11	0	0	0	0	0	0
16:00	0	48	0	0	48	0	4	0	0	4	0	0	0	0	0	0
16:15	4	50	0	0	54	0	2	0	0	2	0	0	0	0	0	0
16:30	1	34	0	0	35	0	1	0	0	1	0	0	0	0	0	0
16:45	1	50	2	0	53	0	1	0	0	1	0	0	0	0	0	1
17:00	4	32	0	0	36	0	1	0	0	1	0	0	0	0	0	0
17:15	1	38	1	0	40	0	0	0	0	0	0	0	0	0	0	0
17:30	2	37	0	0	39	0	4	0	0	4	0	0	0	0	0	0
17:45	0	27	1	0	28	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	17	466	7	0	490	0	42	0	0	42	0	0	0	0	0	1
GRAND TOTAL	23	840	10	0	873	1	113	0	0	114	0	0	0	0	0	3



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				TI	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	t		n	Total	-	+		J.	Total	Total Peds
07:00	1	18	0	0	19	0	3	0	0	3	0	0	0	0	0	0
07:15	0	19	2	0	21	0	9	0	0	9	0	0	0	0	0	2
07:30	0	29	1	0	30	0	6	0	0	6	0	0	0	0	0	2
07:45	0	30	1	0	31	0	6	0	0	6	0	0	0	0	0	0
08:00	2	19	0	0	21	0	9	0	0	9	0	0	0	0	0	0
08:15	1	18	0	0	19	0	7	1	0	8	0	0	0	0	0	0
08:30	2	21	1	0	24	0	4	0	0	4	0	0	0	0	0	0
08:45	2	25	1	0	28	0	4	2	0	6	0	0	0	0	0	0
09:00	0	14	0	0	14	0	7	0	0	7	0	0	0	0	0	0
09:15	0	19	0	0	19	0	9	2	0	11	0	0	0	0	0	0
09:30	1	28	2	0	31	0	5	1	0	6	0	0	0	0	0	1
09:45	0	25	1	0	26	0	11	0	0	11	0	0	0	0	0	1
SUBTOTAL	9	265	9	0	283	0	80	6	0	86	0	0	0	0	0	6



Trafalgar Rd N & Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				T	rucks				Bi	cycles			
Start Time	-	1		9	Total	-	1		1	Total	•	1		9	Total	Total Peds
15:00	1	52	0	0	53	0	8	0	0	8	0	0	0	0	0	0
15:15	5	54	2	0	61	0	6	0	0	6	0	0	0	0	0	0
15:30	0	55	4	0	59	1	4	1	0	6	0	0	0	0	0	1
15:45	1	62	8	0	71	0	4	0	0	4	0	0	0	0	0	0
16:00	2	73	1	0	76	0	9	1	0	10	0	0	0	0	0	5
16:15	6	81	2	0	89	0	1	0	0	1	0	0	0	0	0	0
16:30	5	78	5	0	88	0	7	0	0	7	0	0	0	0	0	0
16:45	3	75	1	0	79	0	1	0	0	1	0	0	0	0	0	0
17:00	3	85	3	0	91	0	2	0	0	2	0	0	0	0	0	1
17:15	2	86	0	0	88	0	4	0	0	4	0	0	0	0	0	0
17:30	5	63	1	0	69	1	2	0	0	3	0	0	0	0	0	0
17:45	2	56	5	0	63	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	35	820	32	0	887	2	50	2	0	54	0	0	0	0	0	7
GRAND TOTAL	44	1085	41	0	1170	2	130	8	0	140	0	0	0	0	0	13



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

East Approach - Church St

		(Cars				Ti	rucks				Bi	cycles			
Start Time	-	1		1	Total	•	1		1	Total	•	1		1	Total	Total Peds
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
07:30	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0
07:45	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0
08:00	2	0	2	0	4	0	1	0	0	1	0	0	0	0	0	0
08:15	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0
08:30	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
09:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
09:15	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0
09:30	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
09:45	4	0	1	0	5	0	0	0	0	0	0	0	0	0	0	1
SUBTOTAL	23	0	11	0	34	0	1	0	0	1	0	0	0	0	0	1



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Municipality:	Erin
Count Date:	Oct 28, 2021

East Approach - Church St

			Cars				T	rucks				Bi	cycles			
Start Time	-	1		1	Total	-	1		1	Total	•	1		1	Total	Total Peds
15:00	2	7	1	0	10	0	0	0	0	0	0	0	0	0	0	0
15:15	1	0	2	0	3	1	0	0	0	1	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	5	0	2	0	7	1	0	0	0	1	0	0	0	0	0	0
16:00	4	0	2	0	6	0	0	0	0	0	0	0	0	0	0	1
16:15	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:45	0	1	1	0	2	0	0	0	0	0	0	0	0	0	0	0
17:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:15	3	1	2	0	6	0	0	0	0	0	0	0	0	0	0	0
17:30	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0
17:45	1	0	2	0	3	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	22	9	12	0	43	2	0	0	0	2	0	0	0	0	0	1
GRAND TOTAL	45	9	23	0	77	2	1	0	0	3	0	0	0	0	0	2



Trafalgar Rd N & Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

West Approach - Upper Canada Dr

		(Cars					Bi	cycles							
Start Time	- 🖷 -	1		9	Total	- 🖷 -	1		1	Total	•	1		1	Total	Total Peds
07:00	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
07:30	1	0	3	0	4	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	8	0	8	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0
08:15	1	0	3	0	4	0	0	1	0	1	0	0	0	0	0	0
08:30	1	0	3	0	4	0	0	0	0	0	0	0	0	0	0	5
08:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30	1	0	4	0	5	0	0	0	0	0	0	0	0	0	0	1
09:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	9	0	30	0	39	0	0	1	0	1	0	0	0	0	0	6



Trafalgar Rd N & Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

West Approach - Upper Canada Dr

			Cars				T	rucks				Bi	cycles			
Start Time	•	1		1	Total	-	1		1	Total	-	1		1	Total	Total Peds
15:00	1	3	0	0	4	1	0	0	0	1	0	0	0	0	0	0
15:15	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
15:45	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	1	1	1	0	3	0	0	0	0	0	0	0	0	0	0	0
17:00	1	0	3	0	4	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
17:30	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
17:45	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	8	4	14	0	26	2	0	0	0	2	0	0	0	0	0	0
GRAND TOTAL	17	4	44	0	65	2	0	1	0	3	0	0	0	0	0	6



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church
Site Code:	2121900002
Count Date:	Oct 28, 2021

Peak Hour Diagram

Specified Pe	eriod	One Hour P	eak
From:	07:00:00	From:	07:15:00
To:	10:00:00	To:	08:15:00

Weather conditions:

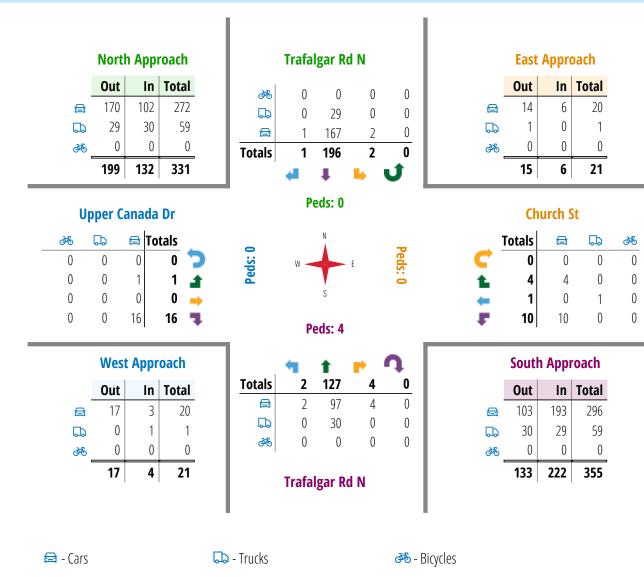
St

Clear

CIE

** Unsignalized Intersection **

Major Road: Trafalgar Rd N runs N/S



Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Count Date:	Oct 28, 2021
Period:	07:00 - 10:00

Peak Hour Data (07:15 - 08:15)

		ſ	North A Trafalg	pproac ar Rd N	ih N			S	outh A Trafalg	pproac ar Rd N	h I				East Ap Chui	oproach rch St	۱				West Aj Jpper Ca				Total Vehicl
Start Time	1	1	•	J	Peds	Total	•	1	•	J	Peds	Total	1	1	•	J	Peds	Total	•	1		J	Peds	Total	es
07:15	2	46	0	0	0	48	0	28	2	0	2	30	2	0	1	0	0	3	0	0	2	0	0	2	83
07:30	0	46	0	0	0	46	0	35	1	0	2	36	3	0	1	0	0	4	1	0	3	0	0	4	90
07:45	0	64	1	0	0	65	0	36	1	0	0	37	3	0	0	0	0	3	0	0	8	0	0	8	113
08:00	0	40	0	0	0	40	2	28	0	0	0	30	2	1	2	0	0	5	0	0	3	0	0	3	78
Grand Total	2	196	1	0	0	199	2	127	4	0	4	133	10	1	4	0	0	15	1	0	16	0	0	17	364
Approach %	1	98.5	0.5	0		-	1.5	95.5	3	0		-	66.7	6.7	26.7	0		-	5.9	0	94.1	0		-	
Totals %	0.5	53.8	0.3	0		54.7	0.5	34.9	1.1	0		36.5	2.7	0.3	1.1	0		4.1	0.3	0	4.4	0		4.7	
PHF	0.25	0.77	0.25	0		0.77	0.25	0.88	0.5	0		0.9	0.83	0.25	0.5	0		0.75	0.25	0	0.5	0		0.53	0.81
Cars	2	167	1	0		170	2	97	4	0		103	10	0	4	0		14	1	0	16	0		17	304
% Cars	100	85.2	100	0		85.4	100	76.4	100	0		77.4	100	0	100	0		93.3	100	0	100	0		100	83.5
Trucks	0	29	0	0		29	0	30	0	0		30	0	1	0	0		1	0	0	0	0		0	60
% Trucks	0	14.8	0	0		14.6	0	23.6	0	0		22.6	0	100	0	0		6.7	0	0	0	0		0	16.5
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					4	-					0	-					0	-	4
% Peds					0	-					100	-					0	-					0	-	



Intersection:	Trafalgar Rd N & Upper Canada Dr-Church
Site Code:	2121900002
Count Date:	Oct 28, 2021

Peak Hour Diagram

Specified Pe	eriod	One Hour P	eak
From:	15:00:00	From:	15:45:00
To:	18:00:00	To:	16:45:00

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Trafalgar Rd N runs N/S

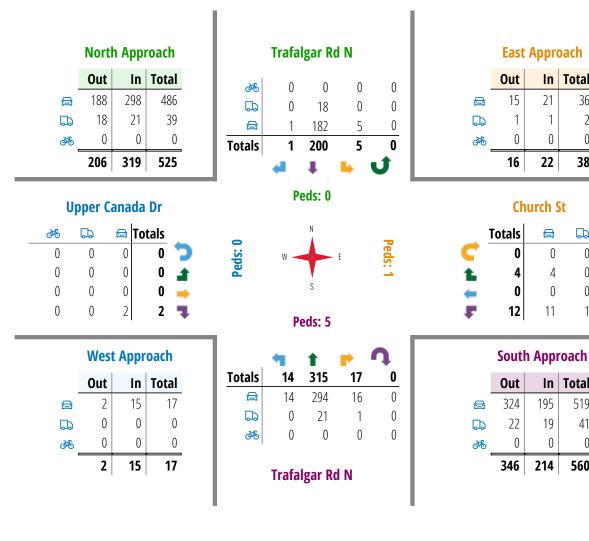
In Total

G

æ

⊟

In Total



St



🗔 - Trucks

💑 - Bicycles

Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Upper Canada Dr-Church St
Site Code:	2121900002
Count Date:	Oct 28, 2021
Period:	15:00 - 18:00

Peak Hour Data (15:45 - 16:45)

		r	North A Trafalg	pproac ar Rd N	:h I			S	outh A Trafalg	pproac ar Rd N	h I				East Ap Chur	oproach rch St	1			ι	West Aj Jpper Ca	oproach anada [ı Dr		Total Vehicl
Start Time	•	1	•	J	Peds	Total	1	1	•	J	Peds	Total	1	1	•	J	Peds	Total	1	1	•	J	Peds	Total	es
15:45	0	61	1	0	0	62	1	66	8	0	0	75	6	0	2	0	0	8	0	0	1	0	0	1	146
16:00	0	52	0	0	0	52	2	82	2	0	5	86	4	0	2	0	1	6	0	0	1	0	0	1	145
16:15	4	52	0	0	0	56	6	82	2	0	0	90	1	0	0	0	0	1	0	0	0	0	0	0	147
16:30	1	35	0	0	0	36	5	85	5	0	0	95	1	0	0	0	0	1	0	0	0	0	0	0	132
Grand Total	5	200	1	0	0	206	14	315	17	0	5	346	12	0	4	0	1	16	0	0	2	0	0	2	570
Approach %	2.4	97.1	0.5	0		-	4	91	4.9	0		-	75	0	25	0		-	0	0	100	0		-	
Totals %	0.9	35.1	0.2	0		36.1	2.5	55.3	3	0		60.7	2.1	0	0.7	0		2.8	0	0	0.4	0		0.4	
PHF	0.31	0.82	0.25	0		0.83	0.58	0.93	0.53	0		0.91	0.5	0	0.5	0		0.5	0	0	0.5	0		0.5	0.97
Cars	5	182	1	0		188	14	294	16	0		324	11	0	4	0		15	0	0	2	0		2	529
% Cars	100	91	100	0		91.3	100	93.3	94.1	0		93.6	91.7	0	100	0		93.8	0	0	100	0		100	92.8
Trucks	0	18	0	0		18	0	21	1	0		22	1	0	0	0		1	0	0	0	0		0	41
% Trucks	0	9	0	0		8.7	0	6.7	5.9	0		6.4	8.3	0	0	0		6.3	0	0	0	0		0	7.2
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					5	-					1	-					0	-	6
% Peds					0	-					83.3	-					16.7	-					0	-	



Project #21-219 - Candevcon Limited

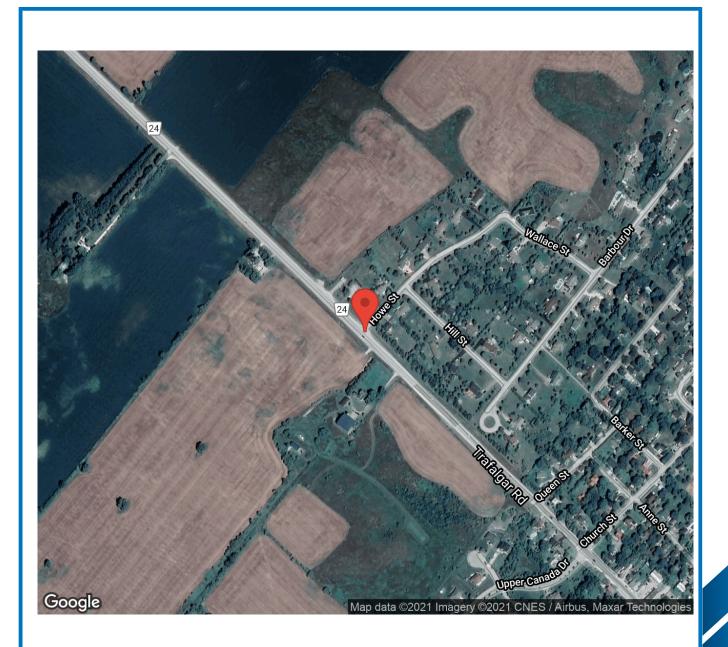
Intersection Count Report

Intersection:	Trafalgar Rd N & Howe St
Municipality:	Erin
Count Date:	Oct 28, 2021
Site Code:	2121900001
Count Categories:	Cars, Trucks, Bicycles, Pedestrians
Count Period:	07:00-10:00, 15:00-18:00
Weather:	Clear



Traffic Count Map

ntersection:	Trafalgar Rd N & Howe St
Site Code:	2121900001
Municipality:	Erin
Count Date:	Oct 28, 2021





Traffic Count Summary

Intersection:	Trafalgar Rd N & Howe St
Site Code:	2121900001
Municipality:	Erin
Count Date:	Oct 28, 2021

Trafalgar Rd N - Traffic Summary

		North	Appr	oach T	otals			South Approach Totals						
		Include	s Cars, 1	Trucks, Bi	cycles									
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total	
07:00 - 08:00	0	155	0	0	155	0	0	119	1	0	120	0	275	
08:00 - 09:00	3	166	0	0	169	0	0	125	1	0	126	0	295	
09:00 - 10:00	0	112	0	0	112	0	0	132	4	0	136	0	248	
					В	REAK								
15:00 - 16:00	7	194	0	0	201	0	0	250	5	0	255	0	456	
16:00 - 17:00	5	183	0	0	188	0	0	325	5	0	330	0	518	
17:00 - 18:00	3	163	0	0	166	0	0	308	8	0	316	0	482	
GRAND TOTAL	18	973	0	0	991	0	0	1259	24	0	1283	0	2274	



Traffic Count Summary

Intersection:	Trafalgar Rd N & Howe St
Site Code:	2121900001
Municipality:	Erin
Count Date:	Oct 28, 2021

Howe St - Traffic Summary

		East	Appro	ach To	tals								
		Include	s Cars, 1	Frucks, Bi	cycles								
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	4	0	2	0	6	0	0	0	0	0	0	0	6
08:00 - 09:00	1	0	3	0	4	0	0	0	0	0	0	0	4
09:00 - 10:00	3	0	1	0	4	0	0	0	0	0	0	0	4
					В	REAK							
15:00 - 16:00	7	0	4	0	11	0	0	0	0	0	0	0	11
16:00 - 17:00	2	0	2	0	4	0	0	0	0	0	0	0	4
17:00 - 18:00	3	0	2	0	5	0	0	0	0	0	0	0	5
GRAND TOTAL	20	0	14	0	34	0	0	0	0	0	0	0	34



Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Care				т					D	inveloe			
			Cars	-			1	rucks	-			Б	icycles	-		
Start Time	Ţ			n	Total	•			n	Total	- 🐂 -			U	Total	Total Peds
07:00	0	35	0	0	35	0	4	0	0	4	0	0	0	0	0	0
07:15	0	40	0	0	40	0	6	0	0	6	0	0	0	0	0	0
07:30	0	34	0	0	34	0	5	0	0	5	0	0	0	0	0	0
07:45	0	25	0	0	25	0	6	0	0	6	0	0	0	0	0	0
08:00	2	33	0	0	35	0	5	0	0	5	0	0	0	0	0	0
08:15	0	33	0	0	33	0	7	0	0	7	0	0	0	0	0	0
08:30	0	36	0	0	36	0	8	0	0	8	0	0	0	0	0	0
08:45	1	38	0	0	39	0	6	0	0	6	0	0	0	0	0	0
09:00	0	20	0	0	20	0	7	0	0	7	0	0	0	0	0	0
09:15	0	20	0	0	20	0	4	0	0	4	0	0	0	0	0	0
09:30	0	28	0	0	28	0	6	0	0	6	0	0	0	0	0	0
09:45	0	23	0	0	23	0	4	0	0	4	0	0	0	0	0	0
SUBTOTAL	3	365	0	0	368	0	68	0	0	68	0	0	0	0	0	0



Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

North Approach - Trafalgar Rd N

			Cars			Trucks						Bi	cycles			
Start Time	-	1		1	Total	-	t		9	Total	-	t		9	Total	Total Peds
15:00	3	41	0	0	44	0	7	0	0	7	0	0	0	0	0	0
15:15	3	42	0	0	45	0	4	0	0	4	0	0	0	0	0	0
15:30	1	25	0	0	26	0	6	0	0	6	0	0	0	0	0	0
15:45	0	58	0	0	58	0	11	0	0	11	0	0	0	0	0	0
16:00	1	42	0	0	43	0	4	0	0	4	0	0	0	0	0	0
16:15	2	56	0	0	58	0	2	0	0	2	0	0	0	0	0	0
16:30	2	34	0	0	36	0	1	0	0	1	0	0	0	0	0	0
16:45	0	44	0	0	44	0	0	0	0	0	0	0	0	0	0	0
17:00	1	39	0	0	40	0	1	0	0	1	0	0	0	0	0	0
17:15	2	44	0	0	46	0	1	0	0	1	0	0	0	0	0	0
17:30	0	40	0	0	40	0	3	0	0	3	0	0	0	0	0	0
17:45	0	34	0	0	34	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	15	499	0	0	514	0	41	0	0	41	0	0	0	0	0	0
GRAND TOTAL	18	864	0	0	882	0	109	0	0	109	0	0	0	0	0	0



Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				Т	rucks				Bi	cycles			
Start Time	-	1	P	9	Total	-	t	P	J.	Total	-	1		J.	Total	Total Peds
07:00	0	20	0	0	20	0	3	0	0	3	0	0	0	0	0	0
07:15	0	22	0	0	22	0	9	0	0	9	0	0	0	0	0	0
07:30	0	32	1	0	33	0	6	0	0	6	0	0	0	0	0	0
07:45	0	23	0	0	23	0	4	0	0	4	0	0	0	0	0	0
08:00	0	26	1	0	27	0	9	0	0	9	0	0	0	0	0	0
08:15	0	23	0	0	23	0	7	0	0	7	0	0	0	0	0	0
08:30	0	27	0	0	27	0	3	0	0	3	0	0	0	0	0	0
08:45	0	26	0	0	26	0	4	0	0	4	0	0	0	0	0	0
09:00	0	24	0	0	24	0	9	0	0	9	0	0	0	0	0	0
09:15	0	23	0	0	23	0	6	1	0	7	0	0	0	0	0	0
09:30	0	32	0	0	32	0	5	0	0	5	0	0	0	0	0	0
09:45	0	23	3	0	26	0	10	0	0	10	0	0	0	0	0	0
SUBTOTAL	0	301	5	0	306	0	75	1	0	76	0	0	0	0	0	0



Traffic Count Data

Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

South Approach - Trafalgar Rd N

			Cars				Т	rucks				Ri	cycles			
Start Time	-	1		9	Total	-	t	P	9	Total	-	1		n	Total	Total Peds
15:00	0	58	0	0	58	0	10	0	0	10	0	0	0	0	0	0
15:15	0	55	0	0	55	0	6	0	0	6	0	0	0	0	0	0
15:30	0	53	1	0	54	0	7	1	0	8	0	0	0	0	0	0
15:45	0	57	2	0	59	0	4	1	0	5	0	0	0	0	0	0
16:00	0	77	1	0	78	0	7	0	0	7	0	0	0	0	0	0
16:15	0	78	3	0	81	0	1	0	0	1	0	0	0	0	0	0
16:30	0	74	1	0	75	0	7	0	0	7	0	0	0	0	0	0
16:45	0	80	0	0	80	0	1	0	0	1	0	0	0	0	0	0
17:00	0	76	4	0	80	0	1	0	0	1	0	0	0	0	0	0
17:15	0	89	2	0	91	0	5	0	0	5	0	0	0	0	0	0
17:30	0	66	1	0	67	0	2	0	0	2	0	0	0	0	0	0
17:45	0	66	1	0	67	0	3	0	0	3	0	0	0	0	0	0
SUBTOTAL	0	829	16	0	845	0	54	2	0	56	0	0	0	0	0	0
GRAND TOTAL	0	1130	21	0	1151	0	129	3	0	132	0	0	0	0	0	0



Traffic Count Data

Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

East Approach - Howe St

			Cars				T	rucks				Bi	icycles			
Start Time	-	1		9	Total	- 🖷	1		1	Total	-	1		1	Total	Total Peds
07:00	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
07:30	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
08:30	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	6	0	6	0	12	2	0	0	0	2	0	0	0	0	0	0



Traffic Count Data

Trafalgar Rd N & Howe St
2121900001
Erin
Oct 28, 2021

East Approach - Howe St

			Cars				T	rucks				B	icycles			
Start Time	-	1		9	Total	-	1		9	Total	- 🖷	1	-	1	Total	Total Peds
15:00	1	0	2	0	3	0	0	0	0	0	0	0	0	0	0	0
15:15	3	0	2	0	5	0	0	0	0	0	0	0	0	0	0	0
15:30	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0
15:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
17:15	2	0	1	0	3	0	0	0	0	0	0	0	0	0	0	0
17:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	12	0	8	0	20	0	0	0	0	0	0	0	0	0	0	0
GRAND TOTAL	18	0	14	0	32	2	0	0	0	2	0	0	0	0	0	0



Intersection:Trafalgar Rd N & Howe StSite Code:2121900001Count Date:Oct 28, 2021

Peak Hour Diagram

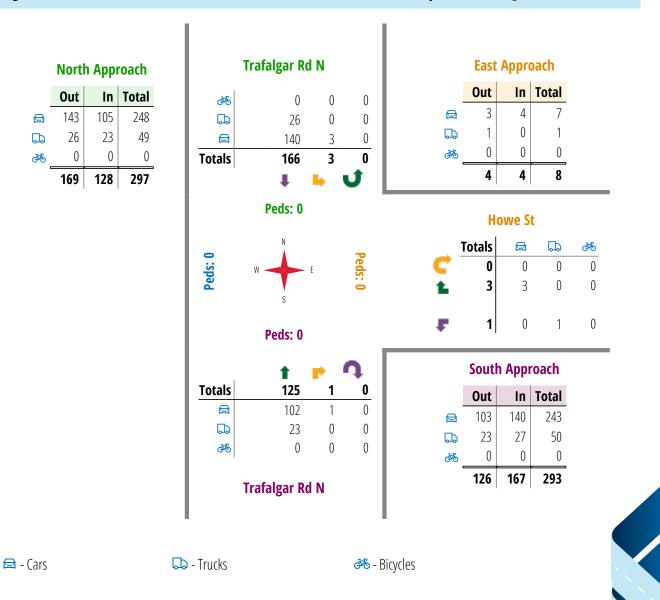
Specified Pe	eriod	One Hour P	eak
From:	07:00:00	From:	08:00:00
To:	10:00:00	To:	09:00:00

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Trafalgar Rd N runs N/S



Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Howe St
Site Code:	2121900001
Count Date:	Oct 28, 2021
Period:	07:00 - 10:00

Peak Hour Data (08:00 - 09:00)

		I	North A Trafalg	approac ar Rd N	:h N			S	outh A Trafalg	approac ar Rd N	h I				East Ap Hov	oproach ve St	1		West Approach						Total Vehicl
Start Time	•	1	•	9	Peds	Total	•	1	•	ŋ	Peds	Total	•	1		ŋ	Peds	Total	•	1	•	9	Peds	Total	es
08:00	2	38		0	0	40		35	1	0	0	36	0		1	0	0	1					0		77
08:15	0	40		0	0	40		30	0	0	0	30	1		0	0	0	1					0		71
08:30	0	44		0	0	44		30	0	0	0	30	0		2	0	0	2					0		76
08:45	1	44		0	0	45		30	0	0	0	30	0		0	0	0	0					0		75
Grand Total	3	166		0	0	169		125	1	0	0	126	1		3	0	0	4					0	0	299
Approach %	1.8	98.2		0		-		99.2	0.8	0		-	25		75	0		-						-	
Totals %	1	55.5		0		56.5		41.8	0.3	0		42.1	0.3		1	0		1.3						0	
PHF	0.38	0.94		0		0.94		0.89	0.25	0		0.88	0.25		0.38	0		0.5						0	0.97
Cars	3	140		0		143		102	1	0		103	0		3	0		3						0	249
% Cars	100	84.3		0		84.6		81.6	100	0		81.7	0		100	0		75						0	83.3
Trucks	0	26		0		26		23	0	0		23	1		0	0		1						0	50
% Trucks	0	15.7		0		15.4		18.4	0	0		18.3	100		0	0		25		_				0	16.7
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Intersection:Trafalgar Rd N & Howe StSite Code:2121900001Count Date:Oct 28, 2021

Peak Hour Diagram

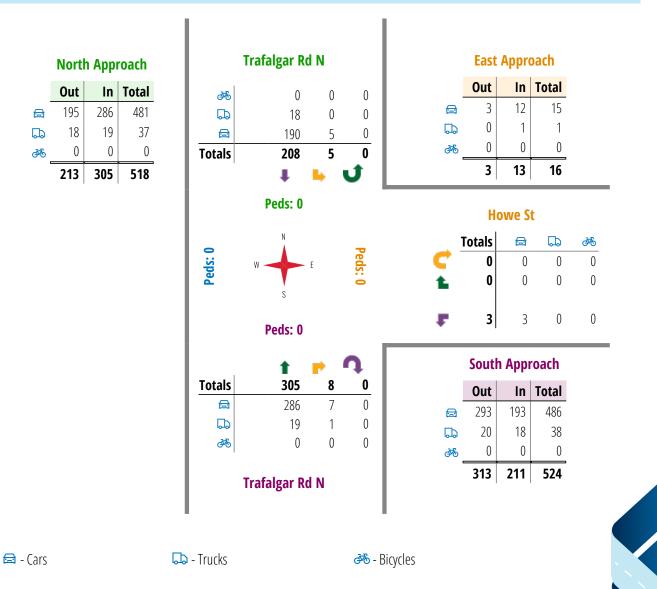
Specified Pe	eriod	One Hour P	eak
From:	15:00:00	From:	15:45:00
To:	18:00:00	To:	16:45:00

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Trafalgar Rd N runs N/S



Comments



Peak Hour Summary

Intersection:	Trafalgar Rd N & Howe St
Site Code:	2121900001
Count Date:	Oct 28, 2021
Period:	15:00 - 18:00

Peak Hour Data (15:45 - 16:45)

		١	North A Trafalg	approac ar Rd N	:h N			S	outh A Trafalg	pproac ar Rd N	h I		East Approach Howe St							West Approach					
Start Time	•	1	•	9	Peds	Total	•	1	•	J	Peds	Total	•	1	•	9	Peds	Total	•	1	•	9	Peds	Total	Vehicl es
15:45	0	69		0	0	69		61	3	0	0	64	1		0	0	0	1					0		134
16:00	1	46		0	0	47		84	1	0	0	85	0		0	0	0	0					0		132
16:15	2	58		0	0	60		79	3	0	0	82	1		0	0	0	1					0		143
16:30	2	35		0	0	37		81	1	0	0	82	1		0	0	0	1					0		120
Grand Total	5	208		0	0	213		305	8	0	0	313	3		0	0	0	3					0	0	529
Approach %	2.3	97.7		0		-		97.4	2.6	0		-	100		0	0		-						-	
Totals %	0.9	39.3		0		40.3		57.7	1.5	0		59.2	0.6		0	0		0.6						0	
PHF	0.63	0.75		0		0.77		0.91	0.67	0		0.92	0.75		0	0		0.75						0	0.92
Cars	5	190		0		195		286	7	0		293	3		0	0		3						0	491
% Cars	100	91.3		0		91.5		93.8	87.5	0		93.6	100		0	0		100						0	92.8
Trucks	0	18		0		18		19	1	0		20	0		0	0		0						0	38
% Trucks	0	8.7		0		8.5		6.2	12.5	0		6.4	0		0	0		0						0	7.2
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	

APPENDIX C

SIGNAL TIMING PLANS

Configuration

				Con	trolle	er Se	quence	Pric	ority			
	1	2	3	4	5	6	7	8	9	10	11	12
Ring 1 Phases Ring 2 Phases								0 0		0 0	0 0	0 0
	-	0	2	4	-	C	Phase	0	0	1.0		1.0
	T	2	3	4	5	6	7	8	9	10	11	12
In Use							•		•	•	•	•
Exclusive Ped Direction	•	•	•	•	•	•	•	•	•	•	•	•

		Overlap						
	А	В	С	D				
Direction								

Load Switch Channel/Driver Group Assign (Info Only):

Load					Signal							
Switch						Driver	Group					
(MMU)						Phase/						
Channel						Ovlap	Ped					
1 .	•					1						
2.	•					2						
з.	•					3						
4.	•					4						
5.	•					5						
6.	•		•	•		6						
7.	•		•	•		7	•					
8.	•		•	•		8						
9.	•		•	•		2	Х					
10 .	•		•	•		4	Х					
11 .	•		•	•		6	Х					
12 .	•		•	•		8	Х					
13 .						A						
14 .	•		•	•		В						
15 .	•	•		•		С						
16 .	•	•				D						

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13 Configuration Continued _____ Enable BIU: 1 2 3 4 5 6 7 8 Terminal/Facilities. Detector Rack. Type 2 Runs as Type 1. . . MMU Disable. X Diagnostic Enable. Peer-Peer Comm Enable. . . 1 2 3 4 5 6 7 8 9 10 Port 2: Port 2 Protocol Terminal Port 2 Enable YES AB3418 Address. 0 AB3418 Group Address. . . . 0 AB3418 Response Delay 0 AB3418 Single Flag Enable . . . NO AB3418 Drop-Out Time. . . . 0 AB3418 TOD SF Select. . . . 0 Data, Parity, Stop. 8, 0, 1 Port 3: Port 3 Protocol Telemetry Port 3 Enable NO Telemetry Address 0 System Detector 9-16 Address. . 0 Telemetry Response Delay. . . . 7800 AB3418 Address. 0 AB3418 Group Address. . . . 0 AB3418 Response Delay 0 AB3418 Single Flag Enable . . . NO AB3418 Drop-Out Time. . . . 0 AB3418 TOD SF Select. . . . 0 Data, Parity, Stop. 8, 0, 1

Configuration Continued

Event Enabling

Alarm Enabling

Critical RFE'S ((MMU	/TF)	•	•	•	•	•	Х
Non-Critical RFE	I'S	(DE	T/	ΤE	ST)	•	•	Х
Detector Errors		•	•	•	•	•	•	•	Х
Coordination Err	ors	•	•	•	•	•	•	•	Х
MMU Flash Faults	5	•	•	•	•	•	•	•	Х
Local Flash Faul	ts.	•	•	•	•	•	•	•	Х
Preempt		•	•	•	•	•	•	•	Х
Power On/Off		•	•	•	•	•	•	•	Х
Low Battery		•	•	•	•	•	•	•	Х

ALARM	1										•	Х
ALARM	2	•						•			•	Х
ALARM	3	•		•		•	•	•			•	Х
ALARM	4	•						•			•	
ALARM	5	•	•	•	•	•	•	•	•	•	•	•
ALARM	6	•		•	•	•	•	•	•	•	•	•
ALARM	7											
ALARM	8	•		•	•	•	•	•	•	•	•	•
ALARM	9	•		•	•	•	•	•	•	•	•	•
ALARM	10											
ALARM	11	•		•	•	•	•	•	•	•	•	•
ALARM	12											
ALARM	13											
ALARM	14									•		
ALARM	15	•								•		
ALARM	16											

Supervisor Access Code. . . **** Data Change Access Code . . ****

MMU Compatibility Program (Info Only)

Channel		Is	Allow	red to	Time	With C	hannel
	16 15	14 13	12 1	1 10	9 8	76	5 4 3 2
1			•	• •	• •		
2			•	• •	• •	• •	
3			•	• •	• •		• •
4			•				•
5			•				
6			•			•	
7							
8			•				
9			•	• •			
10			•				
11			•				
12							
13							
14							
15							

Version Info:		
Software Assy.	Part No.	Version
Boot	27831	2.83
Program	45561	7.9
Application		. 3
Help	27891	6.33
Configuration	27918	C000 F

By-Phase Timing Data

	1	2	3	4	5	Ph 6	ase 7	8	9	10	11	12
Direction	Ţ	Z	2	4	5	0	/	0	9	10		ΤZ
Minimum Green	5	30	5	15	5	30	5	15	5	5	5	5
Bike Min Green	0	0	0	0	0	0	0	0	0	0	0	0
Cond Serv Min Grn	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	5	0	5	0	5	0	5	0	10	0	10
Ped Clearance	0	7	0	7	0	7	0	7	0	16	0	16
Veh Extension	5.0	5.0	5.0	4.0	5.0	5.0	5.0	4.0	5.0	5.0	5.0	5.0
Alt Veh Exten	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Extension	0	0	0	0	0	0	0	0	0	0	0	0
Max 1	35	35	35	27	35	35	35	27	35	35	35	35
Max 2	40	15	40	40	40	15	40	40	40	40	40	40
Max 3	0	0	0	0	0	0	0	0	0	0	0	0
Det. Fail Max	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Change	3.0	5.5	3.0	5.5	3.0	5.5	3.0	5.5	3.0	3.0	3.0	3.0
Red Clearance	1.0	2.0	1.0	2.0	1.0	2.0	1.0	2.0	1.0	1.0	1.0	1.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act. B4 Init	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Actuation	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Initial	30	30	30	30	30	30	30	30	30	30	30	30
Time B4 Reduction	0	0	0	0	0	0	0	0	0	0	0	0
Cars Waiting	0	0	0	0	0	0	0	0	0	0	0	0
Time To Reduce	0	0	0	0	0	0	0	0	0	0	0	0
Minimum Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

No-Serve Phases

				Phase	Ca	nnot	Serv	ve W:	ith	Phase	Ð	
Phase		12	11	10	9	8	7	6	5	4	3	2
1	•		•	•	•		•	•	•	•	•	•
2	•		•	•	•		•	•	•	•	•	
3	•		•	•	•		•	•	•	•		
4	•	•	•	•	•	•	•	•	•			
5	•	•	•	•	•	•	•	•				
6	•	•	•	•	•	•	•					
7	•		•	•	•							
8	•	•	•	•	•							
9	•		•	•								
10	•		•									
11	•	•										

Ped Carryover

Ped Start Phase	Carry Over Phase
1	0
2	0
3	0
4	0
5	0
6	0
7	0
8	0
9	0
10	0
11	0
12	0

Vehicle/Ped Phase as Overlap

Veh

	Ped Phase As Overlap												
Ped			(Consi	ists	of H	Ped H	Phase	es				
Ovlap													
Phase	1	2	3	4	5	6	7	8	9	10	11	12	
1	•	•	•	•	•	•			•	•	•	•	
2	•	•	•	•	•					•	•		
3	•	•	•	•	•	•			•	•	•	•	
4	•	•	•	•			•	•	•			•	
5	•	•	•	•	•				•	•	•	•	
6	•	•	•	•			•	•	•			•	
7	•	•	•	•			•						
8	•		•	•			•	•			•		
9	•	•	•	•			•						
10	•		•	•			•	•			•		
11	•		•							•			
12	•	•	•	•						•	•		

Veh Phase As Overlap Consists of Veh Phases

Ovlap												
Phase	1	2	3	4	5	6	7	8	9	10	11	12
1	Х	•		•	•	•		•	•	•	•	
2	•	Х	•	•	•		•	•	•	•	•	•
3	•	•	Х	•	•		•	•	•	•	•	•
4	•	•	•	Х	•		•	•	•	•	•	•
5	•	•	•		Х	•		•	•			•
6	•	•	•	•	•	Х	•	•	•	•	•	•
7	•	•	•	•	•		Х	•	•	•	•	•
8	•	•	•			•		Х	•			•
9	•	•	•	•	•		•	•	Х	•	•	•
10	•	•	•	•	•	•	•	•	•	Х	•	•
11	•	•		•	•	•		•	•	•	Х	
12	•	•	•	•	•	•	•	•	•	•	•	Х

Overlap Data 													
Overlap A	Phase:	1	2	З	4	5	6	7	8	9	10	11	12
Standard		-	_		•	•	-		•				
Protected		•				•		•					•
Permitted					•								
Enable Lag					•	•							
Enable Lead								•					
Spare													
Spare Advance Green Ti	mer				0.0	•	•	•	•	•	•	•	•
Lag/Lead Timers					Green		Yel	Llow		Red			
Lag/Lead limers		•••	•••	•	0.0		0.	. 0		0.0			
Overlap B													
Standard													
Protected					•					•			
Permitted					•					•			
Enable Lag								•	•	•	•	•	•
Enable Lead								•	•	•	•	•	
Spare							•	•	•	•	•	•	•
Advance Green Ti	mer	•••											
- (-) -					Green								
Lag/Lead Timers		•••	• •	•	0.0		0.	. 0		0.0			
Overlap C	Phase:	1	2	3	4	5	6	7	8	9	10	11	12
Standard		•	•	•	•	•	•	•	•		•		
Protected					•	•	•	•	•		•	•	
Permitted		•	•		•	•		•	•	•	•	•	
Enable Lag					•	•	•	•	•		•	•	
Enable Lead					•	•	•	•	•		•	•	
Spare								•	•		•	•	
Advance Green Ti													
Lag/Lead Timers				•	Green 0.0		Yel 0.			Red 0.0			
5.													
Overlap D	Phase:	1	2	3	4	5	6	7	8	9	10	11	12
Standard	••••	•	•	•	•	•	•	•	•	•	•	•	•
Protected	••••	•	•	•	•	•	•	•	•	•	•	•	•
Permitted	••••	•	•	•	•	•	•	•	•	•	•	•	•
Enable Lag	• • • •	•	•	•	•	•	•	•	•	•	•	•	•
Enable Lead	• • • •	•	•	•	•	•	•	•	•	•	•	•	•
Spare	• • • •	•	•	•	•	•	•	•	•	•	•	•	•
Advance Green Ti	mer	•••	•••	•	0.0			_					
					Green			llow		Red			
Lag/Lead Timers					0.0		Ο.	. 0		0.0			

Power Start, Remote Flash

						Ph	ase										
	1	2	3	4	5	6	7	8	9	10	11	12					
Power Start	•	Х				Х		•									
External Start	•	Х	•	•	•	Х	•	•	•	•	•	•					
Into Remote Flash		Х				Х											
Exit Remote Flash		Х				Х							О	ver	lap	,	
Remote Flash Yellow.	•	•		•	•		•	•	•		•	•	А	В	C	D	
Flash Together												Х		Х		Х	
Initialization Interval	1:																
Power Start		•	Yel	low													
External Start	•••	•	Yel	low													
			0														
Power Start All Red Tir																	
Power Start Flash Time	• •	•	0														
Remote Flash Options:																	
Remote Flash Options.																	
Out of Flash Yellow .			N	0													
Out of Flash All Red.			Ν	0													
Minimum Recall			N	0													
Alternate Flash			N	0													
Flash Thru Load Switche			N	0													

Flash Thru Load Switches. . NO Cycle Through Phases. . . NO Option Data

	Phase												
		1	2	3	4	5	6	7	8	9	10	11	12
Guaranteed Passage .	•	•	•	•	•	•	•	•	•	•	•	•	•
Call To NonActuated 1	•		Х	•	•	•	Х	•	•				•
Call To NonActuated 2	•			•	Х		•	•	Х				•
Dual Entry	•		Х	•	Х	•	Х	•	Х		Х		Х
Conditional Service .	•	Х		Х	•	Х	•	Х	•	Х		Х	•
Conditional Reservice	•			•	•		•	•	•				•
Actuated Rest in Walk	•			•	•		•	•	•				•
Flashing Walk	•	•	•	•	•	•	•	•	•	•		•	•

Enable Programm	able Options
Dual Entry ON	Backup Protection Group 1 OFF
Conditional Service OFF	Backup Protection Group 2 OFF
Ped Clearance Protection OFF	Backup Protection Group 3 OFF
Special Preempt Overlap Flash . OFF	Simultaneous Gap Group 1 OFF
Cond Service Det Cross Switch . OFF	Simultaneous Gap Group 2 OFF
Lock Detectors in Red Only OFF	Simultaneous Gap Group 3 OFF

 Five Section Left Turn Control

 Phases: 5-2
 7-4
 1-6
 3-8
 11-10
 9-12

 Left Turn Head.
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Recall Data, Dimming

	Phase												
		1	2	3	4	5	6	7	8	9	10	11	12
Locking Detector	•	•	•	•	•	•	•			•	•	•	•
Vehicle Recall	•	•	Х	•	•	•	Х			•	•	•	•
Pedestrian Recall		•		•						•		•	
Recall To Max		•	•	•	•					•		•	
Soft Recall		•	•	•	•					•		•	
Don't Rest Here		•	•	•	•					•		•	
Ped Dark if No Call .	•	•	•	•	•	•	•	•	•	•	•	•	•

Dimming:

Dimming:																
	Load Switch															
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	
Green/Walk NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	
Yellow/Ped Clear. NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	
Red/Don't Walk NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	

Detector Type/Timers

	Locking	Log	Tim	ers	Don't Res	ρt	
Det.	Memory		Extend	Delay	Extend	00	Туре
1	NO	NO	0.0	0		0	- Normal
2	NO	NO	0.0	0	•	0	- Normal
3	NO	NO	0.0	0	•	0	- Normal
4	NO	NO	0.0	7	•	1	- Extend/Delay
5	NO	NO	0.0	0		0	- Normal
6	NO	NO	0.0	0		0	- Normal
7	NO	NO	0.0	0	•	0	- Normal
8	NO	NO	0.0	7		1	- Extend/Delay
9	NO	NO	0.0	0		0	- Normal
10	NO	NO	0.0	0	•	0	- Normal
11	NO	NO	0.0	0		0	- Normal
12	NO	NO	0.0	0		0	- Normal
13	NO	NO	0.0	0		0	- Normal
14	NO	NO	0.0	0	•	0	- Normal
15	NO	NO	0.0	0	•	0	- Normal
16	NO	NO	0.0	0	•	0	- Normal
17	NO	NO	0.0	0	•	0	- Normal
18	NO	NO	0.0	0	•	0	- Normal
19	NO	NO	0.0	0	•	0	- Normal
20	NO	NO	0.0	0	•	0	- Normal
21	NO	NO	0.0	0	•	0	- Normal
22	NO	NO	0.0	0	•	0	- Normal
23	NO	NO	0.0	0	•	0	- Normal
24	NO	NO	0.0	0	•	0	- Normal
25	NO	NO	0.0	0	•	0	- Normal
26	NO	NO	0.0	0	•	0	- Normal
27	NO	NO	0.0	0	•	0	- Normal
28	NO	NO	0.0	0	•	0	- Normal
29	NO	NO	0.0	0	•	0	- Normal
30	NO	NO	0.0	0	•	0	- Normal
31	NO	NO	0.0	0	•	0	- Normal
32	NO	NO	0.0	0	•	0	- Normal

Detector Names

Det	1:	Detector	1	Det	17:	Detector	17
Det	2:	Detector	2	Det	18:	Detector	18
Det	3:	Detector	3	Det	19:	Detector	19
Det	4:	Detector	4	Det	20:	Detector	20
Det	5:	Detector	5	Det	21:	Detector	21
Det	6:	Detector	6	Det	22:	Detector	22
Det	7:	Detector	7	Det	23:	Detector	23
Det	8:	Detector	8	Det	24:	Detector	24
Det	9:	Detector	9	Det	25:	Detector	25
Det	10:	Detector	10	Det	26:	Detector	26
Det	11:	Detector	11	Det	27:	Detector	27
Det	12:	Detector	12	Det	28:	Detector	28
Det	13:	Detector	13	Det	29:	Detector	29
Det	14:	Detector	14	Det	30:	Detector	30
Det	15:	Detector	15	Det	31:	Detector	31
Det	16:	Detector	16	Det	32:	Detector	32

Detector Type/Timers

33	NO	NO	0.0	0	•	0 - Normal
34	NO	NO	0.0	0	•	0 - Normal
35	NO	NO	0.0	0	•	0 - Normal
36	NO	NO	0.0	0	•	0 - Normal
37	NO	NO	0.0	0	•	0 - Normal
38	NO	NO	0.0	0	•	0 - Normal
39	NO	NO	0.0	0	•	0 - Normal
40	NO	NO	0.0	0	•	0 - Normal
41	NO	NO	0.0	0		0 - Normal
42	NO	NO	0.0	0	•	0 - Normal
43	NO	NO	0.0	0		0 - Normal
44	NO	NO	0.0	0		0 - Normal
45	NO	NO	0.0	0	•	0 - Normal
46	NO	NO	0.0	0	•	0 - Normal
47	NO	NO	0.0	0		0 - Normal
48	NO	NO	0.0	0	•	0 - Normal
49	NO	NO	0.0	0	•	0 - Normal
50	NO	NO	0.0	0	•	0 - Normal
51	NO	NO	0.0	0	•	0 - Normal
52	NO	NO	0.0	0	•	0 - Normal
53	NO	NO	0.0	0	•	0 - Normal
54	NO	NO	0.0	0	•	0 - Normal
55	NO	NO	0.0	0	•	0 - Normal
56	NO	NO	0.0	0	•	0 - Normal
57	NO	NO	0.0	0	•	0 - Normal
58	NO	NO	0.0	0	•	0 - Normal
59	NO	NO	0.0	0	•	0 - Normal
60	NO	NO	0.0	0	•	0 - Normal
61	NO	NO	0.0	0	•	0 - Normal
62	NO	NO	0.0	0	•	0 - Normal
63	NO	NO	0.0	0	•	0 - Normal
64	NO	NO	0.0	0	•	0 - Normal

Detector Names

Det	33:	Detector	33	Det 49: Detector	49
Det	34:	Detector	34	Det 50: Detector	50
Det	35:	Detector	35	Det 51: Detector	51
Det	36:	Detector	36	Det 52: Detector	52
Det	37:	Detector	37	Det 53: Detector	53
Det	38:	Detector	38	Det 54: Detector	54
Det	39:	Detector	39	Det 55: Detector	55
Det	40:	Detector	40	Det 56: Detector	56
Det	41:	Detector	41	Det 57: Detector	57
Det	42:	Detector	42	Det 58: Detector	58
Det	43:	Detector	43	Det 59: Detector	59
Det	44:	Detector	44	Det 60: Detector	60
Det	45:	Detector	45	Det 61: Detector	61
Det	46:	Detector	46	Det 62: Detector	62
Det	47:	Detector	47	Det 63: Detector	63
Det	48:	Detector	48	Det 64: Detector	64

Detector Phase Assignment

						Pha	se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
1	Х				•		•	•	•	•	•	
2		Х					•	•	•	•	•	
2 3 4			Х				•	•	•	•	•	
4	•			Х	•		•	•	•	•	•	
5	•				Х		•	•	•	•	•	
6	•	•	•	•	•	Х	•	•	•	•	•	•
7							Х	•	•	•	•	
8 9	•	•	•	•	•	•	•	Х	•	•	•	•
9	•	•	•	•	•	•	•	•	Х	•	•	•
10	•	•	•	•	•	•	•	•	•	Х	•	•
11	•	•	•	•	•	•	•	•	•	•	Х	•
12	•	•	•	•	•	•	•	•	•	•	•	Х
13	•	•	•	•	•	•	•	•	•	•	•	•
14	•	•	•	•	•	•	•	•	•	•	•	•
15	•	•	•	•	•	•	•	•	•	•	•	•
16	•	•	•	•	•	•	•	•	•	•	•	•
17	•	•			•	•	•	•	•	•	•	•
18	•	•			•	•	•	•	•	•	•	•
19	•	•			•	•	•	•	•	•	•	•
20	•	•	•	•	•	•	•	•	•	•	•	•
21	•	•	•	•	•	•	•	•	•	•	•	•
22	•	•	•	•	•	•	•	•	•	•	•	•
23	•	•	•	•	•	•	•	•	•	•	•	•
24	•	•	•	•	•	•	•	•	•	•	•	•
25	•	•	•	•	•	•	•	•	•	•	•	•
26	•	•	•	•	•	•	•	•	•	•	•	•
27	•	•	•	•	•	•	•	•	•	•	•	•
28	•	•	•	•	•	•	•	•	•	•	•	•
29	•	•	•	•	•	•	•	•	•	•	•	•
30	•	•	•	•	•	•	•	•	•	•	•	•
31	•	•	•	•	•	•	•	•	•	•	•	•
32	•	•	•	•	•	•	•	•	•	•	•	•

Detector Cross Switching

						Pha	se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
1	•	•	•	•	•	•	•	•	•	•	•	•
1 2 3 4 5 6	•	•	•	•	•	•	•	•	•	•	•	
3	•	•	•	•	•	•	•	•	•	•	•	
4	•	•	•	•	•	•	•	•	•	•	•	•
5	•	•	•	•	•	•	•	•	•	•	•	•
6	•	•	•	•	•	•	•	•	•	•	•	•
7	•	•	•	•	•	•	•	•	•	•	•	•
8 9	•	•	•	•	•	•	•	•	•	•	•	•
9	•	•	•	•	•	•	•	•	•	•	•	•
10	•	•	•	•	•	•	•	•	•	•	•	
11	•	•	•	•	•	•	•	•	•	•	•	•
12	•	•	•	•	•	•	•	•	•	•	•	•
13	•	•	•	•	•	•	•	•	•	•	•	•
14	•	•	•	•	•	•	•	•	•	•	•	•
15	•	•	•	•	•	•	•	•	•	•	•	•
16	•	•	•	•	•	•	•	•	•	•	•	•
17	•	•	•	•	•	•	•	•	•	•	•	•
18	•	•	•	•	•	•	•	•	•	•	•	•
19	•	•	•	•	•	•	•	•	•	•	•	•
20	•	•	•	•	•	•	•	•	•	•	•	•
21	•	•	•	•	•	•	•	•	•	•	•	•
22	•	•	•	•	•	•	•	•	•	•	•	•
23	•	•	•	•	•	•	•	•	•	•	•	•
24	•	•	•	•	•	•	•	•	•	•	•	•
25	•	•	•	•	•	•	•	•	•	•	•	•
26	•	•	•	•	•	•	•	•	•	•	•	•
27	•	•	•	•	•	•	•	•	•	•	•	•
28	•	•	•	•	•	•	•	•	•	•	•	•
29	•	•	•	•	•	•	•	•	•	•	•	•
30	•	•	•	•	•	•	•	•	•	•	•	•
31	•	•	•	•	•	•	•	•	•	•	•	•
32	•	•	•	•	•	•	•	•	•	•	•	•

Detector Cross Switching

						Pha	se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
33	•	•	•	•	•	•	•	•		•	•	
34						•	•	•				
35						•	•	•				
36	•	•	•	•	•	•	•	•		•	•	•
37	•	•	•	•	•	•	•	•		•	•	•
38	•	•	•	•	•	•	•	•		•	•	•
39	•	•	•	•	•	•	•	•		•	•	•
40	•	•	•	•	•	•	•	•	•	•	•	•
41	•	•	•	•	•	•	•	•	•	•	•	•
42	•	•	•	•	•	•	•	•	•	•	•	•
43		•	•		•	•	•	•		•	•	•
44	•	•	•	•	•	•	•	•	•	•	•	•
45		•	•		•	•	•	•		•	•	•
46	•	•	•	•	•	•	•	•	•	•	•	•
47	•	•	•	•	•	•	•	•	•	•	•	•
48	•	•	•	•	•	•	•	•	•	•	•	•
49	•	•	•	•	•	•	•	•	•	•	•	•
50	•	•	•	•	•	•	•	•	•	•	•	•
51	•	•	•	•	•	•	•	•	•	•	•	•
52	•	•	•	•	•	•	•	•	•	•	•	•
53	•	•	•	•	•	•	•	•	•	•	•	•
54	•	•	•	•	•	•	•	•	•	•	•	•
55	•	•	•	•	•	•	•	•	•	•	•	•
56	•	•	•	•	•	•	•	•	•	•	•	•
57	•	•	•	•	•	•	•	•	•	•	•	•
58	•	•	•	•	•	•	•	•	•	•	•	•
59	•	•	•	•	•	•	•	•	•	•	•	•
60	•	•	•	•	•	•	•	•	•	•	•	•
61	•	•	•	•	•	•	•	•	•	•	•	•
62	•	•	•	•	•	•	•	•	•	•	•	•
63	•	•	•	•	•	•	•	•	•	•	•	•
64	•	•	•	•	•	•	•	•	•	•	•	•

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13 Ped/SD Local Assign,Log Interval _____ Phase Ped Detector 1 2 3 4 5 6 7 8 9 10 11 12 Is Ped Detector No. . . 1 2 3 4 5 6 7 8 9 10 11 12 *Local System Detector No. Detector Log Interval . . 0 *NOTE: System master designations cross referenced to local system detector numbers are: SDA1 = 1 & 9SDA2 = 2 & 10SDB1 = 3 & 11 SDB2 = 4 & 12SDC1 = 5 & 13SDC2 = 6 & 14SDD1 = 7 & 15SDD2 = 8 & 16

Diagnostic Plans/Fail Action

									Dete	ctor							
Pl	an	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
*F	ail Action	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
									Dete	ctor							
Pl	an	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
-	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
-	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1

*NOTE: 0 = No Action, 1 = Min Recall, 2 = Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

Diagnostic Plans/Fail Action

									Dete	ctor							
Pl	an	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
*F	ail Action	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
									Dete	ctor							
Pl	an	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
-	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
	ail Action	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

*NOTE: 0 = No Action, 1 = Min Recall, 2 = Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

Ped Diagnostic Plans

												_	
Plan		1	2	3	4	5	6	7	8	9	10	11	12
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1

Detector Diagnostic Intervals

Diagnostic Number	*No-Activity Diagnostic Interval	*Max Presence Diagnostic Interval	Erratic Counts
1	0	0	0
2	0	0	0
3	0	0	0
4	0	0	0
5	0	0	0
6	0	0	0
7	0	0	0
8	0	0	0
9	0	0	0
10	0	0	0
11	0	0	0
12	0	0	0
13	0	0	0
14	0	0	0
15	0	0	0
16	0	0	0
17	0	0	0
18	0	0	0
19	0	0	0
20	0	0	0
21	0	0	0
22	0	0	0
23	0	0	0
24	0	0	0
25	0	0	0
26	0	0	0
27	0	0	0
28	0	0	0
29	0	0	0
30	0	0	0
31	0	0	0
32	0	0	0

*NOTE: Scaling is specified in each detector diagnostic plan.

Speed Detectors

			Local	Snee	d Det	ector	~	
One Detector Speed:	1	2	3	-			- 7	8
Local Detector Number	0	0	0	0	0	0	0	0
Vehicle Length	0	0	0	0	0	0	0	0
Loop Length	0	0	0	0	0	0	0	0
Two Detector Speed:								
Local Detector Number	0	0	0	0	0	0	0	0
Speed Trap Length	0	0	0	0	0	0	0	0
			Local	Spee	d Det	ector	2	
One Detector Speed:	9	10	Local 11	Spee 12	d Det 13		- 15	16
One Detector Speed: Local Detector Number	9 0	10 0		12 0	13 0		15	16 0
-	9 0 0		11	12	13 0	14 0	15 0	
Local Detector Number	9 0 0	0	11 0	12 0	13 0	14 0	15 0	
Local Detector Number Vehicle Length	9 0 0	0	11 0	12 0 0	13 0	14 0 0	15 0 0	
Local Detector Number Vehicle Length Loop Length	9 0 0 0	0	11 0	12 0 0	13 0	14 0 0	15 0 0	
Local Detector Number Vehicle Length Loop Length Two Detector Speed:	9 0 0 0 0	0 0 0	11 0 0 0	12 0 0 0	13 0	14 0 0	15 0 0 0	

Units. Inches

NOTE: Speed Detector 1 = STA, Speed Detector 2 = STB

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13 Coordinator Manual Command and Options _____ Manual Enable Pattern 0 Split Units PercentOffsetUnits PercentInterconnect Format . STDInterconnect Source . NICTransition. SMOOTHDwell Period. . . . 0 Resync Count. . . 0 Actuated Coord Phase . . . Actuated Walk Rest . . . Inhibit Max Timing Max 2 Select Floating Force Off Multisync. Phase Split Demand: Call Time Cyc Count 1 2 3 4 5 6 7 8 9 10 11 12 Phase

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 Auto Permissive Min Green
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 < A B C D E F Free Alternate Sequence

Coordination Patterns

Preemptors _____ Preemptor 1 Active Det Lock. Ped Dark Priority Preemption. Yel-Red To Grn. . . Ped Active Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 0 No CVM in Flash. Inhibit Time . . . Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0 Enable Max Time. Max Time 0 Exit Max 0 0 Min Hold Time. . . Hold Delay Time. . 0 Green Yellow Red 0.0 Minimum 0 0.0 0.0 0 Track Clear 0.0 Hold. 0.0 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • • Exit Phases • . . Out of Flash Color for Exit Phases . . . Green _____ Preemptor 2 Active Det Lock. Ped Dark Priority Preemption. Yel-Red To Grn. . . Ped Active . . . Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 No CVM in Flash. Inhibit Time . . 0 Fast Flash Grn on Hold Phase. . Min Ped Clear. . 0 Enable Max Time.... Max Time 0 Exit Max 0 Min Hold Time. . . 0 Hold Delay Time. . 0 Red Green Yellow Minimum 0 0.0 0.0 0.0 Track Clear 0 0.0 0.0 Hold. 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • • • • Hold Phases • . . • • • • • Exit Phases • • • • • • • . . • . Out of Flash Color for Exit Phases . . . Green Linked Preemptor . . . 0 _____

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13

Preemptors _____ Preemptor 3 Active Det Lock. Ped Dark Priority Preemption. Yel-Red To Grn. . . Ped Active Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 No CVM in Flash. Inhibit Time . . . 0 Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0 Enable Max Time. Max Time 0 Exit Max 0 Min Hold Time. . . 0 Hold Delay Time. . 0 Green Yellow Red 0.0 Minimum 0 0.0 0.0 0.0 Track Clear 0 Hold. 0.0 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • Exit Phases • . . Exit Calls on Phase Out of Flash Color for Exit Phases . . . Green Linked Preemptor . . . 0 _____ Preemptor 4 Active Det Lock. Ped Dark Priority Preemption. . . . Yel-Red To Grn. . . Ped Active Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 No CVM in Flash. Inhibit Time . . . 0 Fast Flash Grn on Hold Phase. . Min Ped Clear. . 0 Enable Max Time. Max Time 0 0 Exit Max Min Hold Time. . . 0 Hold Delay Time. . 0 Red 0.0 Green Yellow 0.0 Minimum 0 Track Clear 0 0.0 0.0 Hold. 0.0 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • . . . Hold Phases • • • . • • . . • Exit Phases • • • • . Exit Calls on Phase Out of Flash Color for Exit Phases . . . Green Linked Preemptor . . . 0 _____

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13

Preemptors _____ Preemptor 5 Active Det Lock. Ped Dark Priority Preemption. Yel-Red To Grn. . . Ped Active Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 No CVM in Flash. Inhibit Time . . . 0 Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0 Enable Max Time. Max Time 0 Exit Max 0 Min Hold Time. . . 0 Hold Delay Time. . 0 Green Yellow Red 0.0 Minimum 0 0.0 0.0 0.0 Track Clear 0 Hold. 0.0 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • Exit Phases • . . Exit Calls on Phase Out of Flash Color for Exit Phases . . . Green Linked Preemptor . . . 0 _____ Preemptor 6 Active Det Lock. Ped Dark Priority Preemption. . . . Yel-Red To Grn. . . Ped Active Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time. . Terminate Overlap ASAP Terminate Phases. . Ped Clr Thru Yel . . Don't Override Flash Duration Time. . . 0 Flash During Hold. Delay Time . . . 0 No CVM in Flash. Inhibit Time . . . 0 Fast Flash Grn on Hold Phase. . Min Ped Clear. . 0 Enable Max Time. Max Time 0 0 Exit Max Min Hold Time. . . 0 Hold Delay Time. . 0 Red 0.0 Green Yellow 0.0 Minimum 0 Track Clear 0 0.0 0.0 Hold. 0.0 0.0 Phase/Overlap 1 2 3 4 5 6 7 8 9 10 11 12/ A B C D Track Clearance Phase • . . . Hold Phases • • • . • • . . • Exit Phases • • • • . Exit Calls on Phase Out of Flash Color for Exit Phases . . . Green Linked Preemptor . . . 0 _____

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Bus Preemptors

				Bus Preemptor					
			1	2	3	4			
Preemptor Active									
Detector Lock				•	•	•			
Maximum Time				•	•	•			
Reservice Time				0	0	0			
				0	0	0			
Delay Time				0	0	0			
Inhibit Time				0	0	0			
Entrance Green				0	0	0			
Entrance Ped Cleara	ance		0	0	0	0			
Entrance Yellow			0.0	0.0	0.0	0.0			
Entrance Red			0.0	0.0	0.0	0.0			
Minimum Hold Time .				0	0	0			
				Hold Phases					
	1	2	3 4	56	7 8	9 10	11 12		
Preemptor 1		•							
Preemptor 2		•							
Preemptor 3		•	• •				<u>.</u>		
Preemptor 4					•••	• •	• •		
TTEEmplor 4	••••	•	• •	• •	• •	• •	• •		

TOD Weekly/Yearly

|--|

			1	2	3		kly 4	Prog 5	ram 6	Numb 7		8	9	10				
Sunda	у.		1	1	1		1	1	1	1		1	1	1	F	rogr	am N	ю.
Monda	- 		1	1	1		1	1	1	1		1	1	1			am N	
Tuesd			1	1	1		1	1	1	1		1	1	1	P	rogr	am N	lo.
Wedne			1	1	1		1	1	1	1		1	1	1			am N	
Thurs	day		1	1	1		1	1	1	1		1	1	1	P	rogr'	am N	lo.
Frida			1	1	1		1	1	1	1		1	1	1	P	rogr'	am N	lo.
Satur			1	1	1		1	1	1	1		1	1	1	P	rogr	am N	lo.
								Wee	k of	Yea	r							
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Prog	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
Prog	19 1	20 1	21 1	22 1	23 1	24 1	25 1	26 1	27 1	28 1	29 1	30 1	31 1	32 1	33 1	34 1	35 1	36 1
Prog	37 1	38 1	39 1	40 1	41 1	42 1	43 1	44 1	45 1	46 1	47 1	48 1	49 1	50 1	51 1	52 1	53 1	

Holiday Programs

Holiday	Туре	Month	Day of Week/ Day of Month	Week of Year/ Year	Program
1	Fixed	0	0	0	0
2	Fixed	0	0	0	0
3	Fixed	0	0	0	0
4	Fixed	0	0	0	0
5	Fixed	0	0	0	0
6	Fixed	0	0	0	0
7	Fixed	0	0	0	0
8	Fixed	0	0	0	0
9	Fixed	0	0	0	0
10	Fixed	0	0	0	0
11	Fixed	0	0	0	0
12	Fixed	0	0	0	0
13	Fixed	0	0	0	0
14	Fixed	0	0	0	0
15	Fixed	0	0	0	0
16	Fixed	0	0	0	0
17	Fixed	0	0	0	0
18	Fixed	0	0	0	0
19	Fixed	0	0	0	0
20	Fixed	0	0	0	0
21	Fixed	0	0	0	0
22	Fixed	0	0	0	0
23	Fixed	0	0	0	0
24	Fixed	0	0	0	0
25	Fixed	0	0	0	0
26	Fixed	0	0	0	0
27	Fixed	0	0	0	0
28	Fixed	0	0	0	0
29	Fixed	0	0	0	0
30	Fixed	0	0	0	0
31	Fixed	0	0	0	0
32	Fixed	0	0	0	0
33	Fixed	0	0	0	0
34	Fixed	0	0	0	0
35	Fixed	0	0	0	0
36	Fixed	0	0	0	0

Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13 NIC Program Steps

Step	Program	Step Begins	Pattern	Override
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TOD Program Steps

Step 1	Program	1	St	сер В	egin	IS	230	00						
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla	•••••• ••••• Enable	• • •	Alt Det Spa	: Veh : Log are 4	Ext Ena	ensi ble.	on . 							
						Phas	e Nu	umber						
			1	2	3	4	5	6	7	8	9	10	11	12
Max 2 Enable	e			Х	•	•	•	Х		•	•	•		•
Max 3 Enable				•	•	•	•	•			•			•
Veh Recall					•	•	•	•	•		•			•
Veh Max Reca				•	•	•	•	•	•	•	•	•	•	•
Ped Recall			•	•	•	•	•	•	•	•	•	•	•	•
Cond Service			•	•	•	•	•	•	•	•	•	•	•	•
Phase Omit			•	•	•	•	•	•	•	•	•	•	•	•
Special Fund	ction	•••	•	•	•	•	•	•	•	•				
			А	В		D	Е	F						
Alt Sequence	e	•••	•	•	•	•		•						
Step 2 Flash Red Rest . Spare 5 Spare 3 Type 0 Dly M Det Diag Pla	 Enable		Dim Alt Det Spa	: Veh : Log	Ena Ext Ena	ble. ensi ble.	 on . 	· · ·	•					
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly 1			Dim Alt Det Spa	nming Veh Log are 4	Ena Ext Ena 	able. ensi able. 	on . 	· · ·	•					
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla	 Enable an	• • • 0	Dim Alt Det Spa	nming Veh Log are 4	Ena Ext Ena 	able. ensi able. 	on . 	· · · ·	•	8	9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable	• • • • • • • • • • • • • • • • • • •	0	Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 		8	9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable	• • • • • • • • • • • • • • • • • • •	0	Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall	• • • • • • • • • • • • • • • • • • •	0	Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca	• • • • • • • • • • • • • • • • • • •		Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall	• • • • • • • • • • • • • • • • • • •	· · · · · ·	Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9 • • •	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall Cond Service	• • • • • • • • • • • • • • • • • • •	· · · · · ·	Dim Alt Det Spa Spa 1	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall Cond Service Phase Omit	• • • • • • • • • • • • • • • • • • •	· · · · · ·	Din Alt Det Spa Spa	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall Cond Service	• • • • • • • • • • • • • • • • • • •	· · · · · ·	Din Alt Det Spa Spa 1	nming Veh Log are 4 are 2	Ena Ext Ena · ·	ble. ensi ble. Phas	on . 	 			9	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall Cond Service Phase Omit	• • • • • • • • • • • • • • • • • • •	· · · · · ·	Din Alt Det Spa Spa 1	nming Veh Log are 4 are 2	Ena Ext Ena 	able. ensi able. Phas 4	on . 	 			9 • • • •	10	11	12
Flash Red Rest . Spare 5 Spare 3 Type 0 Dly H Det Diag Pla Max 2 Enable Max 3 Enable Veh Recall Veh Max Reca Ped Recall Cond Service Phase Omit	• • • • • • • • • • • Enable. • an. • • • • • • • • • • • • • all • • • • • • • • • Inhibit. • • • • • ction • •	· · · · · · ·	Din Alt Det Spa Spa 1	uming Veh Log are 4 are 2 2	Ena Ext Ena 	ble. ensi ble. Phas 4	 on 5	umber 6			9	10	11	12

APPENDIX D

LEVEL OF SERVICE DEFINITIONS

LEVEL OF SERVICE DEFINITIONS

Level of Service Criteria for Signalized Intersections

Level of Service	Control Delay per Vehicle (seconds)	Interpretation
Α	≤ 10	Excellent. Progression is extremely favourable and most of the vehicles arrive during the green phase. Most vehicles do not stop at all
В	>10 & ≤ 20	Very Good. Good progressing, short cycle lengths or both. More vehicles stop than with LOS "A", causing higher levels of average delay.
с	>20 & ≤ 35	Good. Fair progressing, longer cycle lengths or both. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level, though many still pass through the intersection without stopping.
D	>35 & ≤ 55	Fair. At level D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavourable progression, long cycle lengths, or high V/C ratio. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	>55 & ≤ 80	Poor. This level is considered by many agencies to be the limit of acceptable delay. These high delay values generally indicate poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences.
F	>80	Unsatisfactory. This level, considered to be unacceptable to most drivers, often occurs with oversaturation, that is, when arrival flow rates exceed the capacity of the intersection. It may also occurs at high V/C ratios below 1.0 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors to such delays. These high delay values generally indicate poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences.

Source: From Highway Capacity Manual Special Report 209-Table 9-1, Page 9-7

LEVEL OF SERVICE DEFINITIONS

Level of Service Criteria for Two Way Stop Control (TWSC) Intersections

Level of Service	Control Delay per Vehicle (seconds)	Interpretation
A	≤ 10	Excellent. Large & frequent gaps in traffic on the main roadway. Queuing on the minor street is rare
В	>10 & ≤ 15	Very Good. Fewer gaps exist in the traffic on the main roadway. Queuing on the minor street is minimal.
С	>15 & ≤ 25	Good. Fewer gaps exist in traffic on the main roadway. Delay on the minor approach becomes more noticeable.
D	>25 & ≤ 35	Fair. Infrequent & shorter gaps in traffic on the main roadway. Queuing lengths develop on the minor street.
E	>35 & ≤ 50	Poor. Very infrequent gaps in traffic on the main roadway. Queuing lengths become noticeable.
F	>50	Unsatisfactory. Very few gaps in traffic on the main roadway. Excessive delays with significant queue lengths on the minor street

Source: From Highway Capacity Manual Special Report 209-Table 10-7, Page No.10-25

APPENDIX E

SIGNALIZED AND UN-SIGNALIZED INTERSECTION CAPACITY ANALYSIS FOR EXISTING (2021), FUTURE (2026 & 2031) TOTAL BACKGROUND AND FUTURE (2026 & 2031) TOTAL TRAFFIC SENARIOS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		ľ	el 🕴		<u> </u>	eî.	
Traffic Volume (vph)	26	67	32	65	47	36	11	119	55	80	214	26
Future Volume (vph)	26	67	32	65	47	36	11	119	55	80	214	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	0	1746	0	0	1559	0	1805	1563	0	1492	1660	0
Flt Permitted		0.896			0.812		0.590			0.632		
Satd. Flow (perm)	0	1580	0	0	1295	0	1121	1563	0	992	1660	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		25			23			39			10	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0%	19%	9%	21%	12%	18%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	144	0	0	170	0	13	200	0	92	276	0
Turn Type	Perm	NA	-	Perm	NA	-	Perm	NA	-	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8	•		2	-		6	Ū	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase				•	•		_	-		Ū	Ū	
Minimum Initial (s)	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		37.5	37.5		37.5	37.5	
Total Split (s)	34.5	34.5		34.5	34.5		42.5	42.5		42.5	42.5	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%	55.2%		55.2%	55.2%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		5.5	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		7.5			7.5		7.5	7.5		7.5	7.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		16.3			16.3		37.5	37.5		37.5	37.5	
Actuated g/C Ratio		0.24			0.24		0.54	0.54		0.54	0.54	
v/c Ratio		0.37			0.52		0.02	0.23		0.17	0.30	
Control Delay		20.0			25.2		8.5	7.9		9.7	9.9	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		20.0			25.2		8.5	7.9		9.7	9.9	
LOS		B			C		A	A		A	A	
Approach Delay		20.0			25.2			7.9			9.8	
Approach LOS		B			C			A			A	
Queue Length 50th (m)		12.6			16.2		0.7	9.6		5.4	16.9	
Queue Length 95th (m)		25.5			31.9		3.3	22.2		14.1	34.5	
Internal Link Dist (m)		280.3			317.1		0.0	223.9			1432.2	
Turn Bay Length (m)		200.0			• • • •		40.0	220.0		45.0	1102.2	
Base Capacity (vph)		637			523		609	867		539	907	
		501			525		000	507		000	501	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.23			0.33		0.02	0.23		0.17	0.30	
Intersection Summary												
Area Type:	Other											
Cycle Length: 77												
Actuated Cycle Length: 68	3.9											
Natural Cycle: 60												
Control Type: Semi Act-Ur	ncoord											
Maximum v/c Ratio: 0.52												
Intersection Signal Delay:	In	tersectior	LOS: B									
Intersection Capacity Utiliz	IC	U Level o	of Service	E								
Analysis Period (min) 15												

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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42.5 s	34.5 s
Ø6	₩ Ø8
42.5 s	34.5 s

HCM Un-signalized Intersection Capacity Analysis 8: Trafalgar Road North & George Street/Mill Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			- ↔			4	
Traffic Volume (veh/h)	1	1	7	34	0	13	7	160	12	2	293	1
Future Volume (Veh/h)	1	1	7	34	0	13	7	160	12	2	293	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	1	8	37	0	14	8	174	13	2	318	1
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	533	526	320	528	520	180	319			187		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	533	526	320	528	520	180	319			187		
tC, single (s)	7.1	6.5	6.4	7.2	6.5	6.3	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.6	4.0	3.4	2.2			2.2		
p0 queue free %	100	100	99	92	100	98	99			100		
cM capacity (veh/h)	450	456	687	444	460	844	1252			1399		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	10	51	195	321								
Volume Left	1	37	8	2								
Volume Right	8	14	13	1								
cSH	623	510	1252	1399								
Volume to Capacity	0.02	0.10	0.01	0.00								
Queue Length 95th (m)	0.4	2.6	0.2	0.0								
Control Delay (s)	10.9	12.8	0.4	0.1								
Lane LOS	В	В	А	А								
Approach Delay (s)	10.9	12.8	0.4	0.1								
Approach LOS	В	В										
Intersection Summary												
Average Delay			1.5									
Intersection Capacity Utilizat	ion		32.2%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 5: Trafalgar Road North & Upper Canada Drive/Church Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	0	19	12	1	5	2	152	5	2	235	1
Future Volume (Veh/h)	1	0	19	12	1	5	2	152	5	2	235	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	1	0	23	15	1	6	2	188	6	2	290	1
Pedestrians								4				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	496	492	294	516	490	191	291			194		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	496	492	294	516	490	191	291			194		
tC, single (s)	7.1	6.5	6.2	7.1	7.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.9	3.3	2.2			2.2		
p0 queue free %	100	100	97	97	100	99	100			100		
cM capacity (veh/h)	482	479	747	455	362	856	1282			1391		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	24	22	196	293								
Volume Left	1	15	2	2								
Volume Right	23	6	6	1								
cSH	730	515	1282	1391								
Volume to Capacity	0.03	0.04	0.00	0.00								
Queue Length 95th (m)	0.8	1.1	0.0	0.0								
Control Delay (s)	10.1	12.3	0.1	0.1								
Lane LOS	В	В	Α	A								
Approach Delay (s)	10.1	12.3	0.1	0.1								
Approach LOS	В	В										
Intersection Summary												
Average Delay			1.0									
Intersection Capacity Utilizatio	n		26.4%	IC	U Level o	of Service			А			
Analysis Period (min)			15									
· · · · · · · · · · · · · · · · · · ·												

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰Y		4			र्भ
Traffic Volume (veh/h)	1	4	150	1	4	199
Future Volume (Veh/h)	1	4	150	1	4	199
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1	4	155	1	4	205
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	368	156			156	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	368	156			156	
tC, single (s)	7.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	4.4	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	475	896			1436	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	5	156	209			
Volume Left	1	0	4			
Volume Right	4	1	4			
cSH	761	1700	1436			
Volume to Capacity	0.01	0.09	0.00			
	0.01	0.09	0.00			
Queue Length 95th (m)	0.2 9.8		0.1			
Control Delay (s)		0.0				
Lane LOS	A	0.0	A			
Approach Delay (s)	9.8	0.0	0.2			
Approach LOS	А					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization	on		23.7%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		ľ	el 🕴		7	eî.	
Traffic Volume (vph)	48	60	8	56	70	103	30	364	82	56	167	29
Future Volume (vph)	48	60	8	56	70	103	30	364	82	56	167	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	0	1809	0	0	1720	0	1504	1803	0	1703	1812	0
Flt Permitted		0.747			0.876		0.623			0.426		
Satd. Flow (perm)	0	1379	0	0	1525	0	986	1803	0	764	1812	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5			59			19			15	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	2%	14%	4%	2%	2%	20%	3%	0%	6%	3%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	128	0	0	252	0	33	490	0	62	216	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4	-		8	-		2			6	-	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase		-		-	-		_			-	-	
Minimum Initial (s)	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		37.5	37.5		37.5	37.5	
Total Split (s)	34.5	34.5		34.5	34.5		42.5	42.5		42.5	42.5	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%	55.2%		55.2%	55.2%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		5.5	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		7.5			7.5		7.5	7.5		7.5	7.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		16.7			16.7		35.9	35.9		35.9	35.9	
Actuated g/C Ratio		0.25			0.25		0.53	0.53		0.53	0.53	
v/c Ratio		0.37			0.60		0.06	0.51		0.15	0.22	
Control Delay		23.2			23.2		9.1	12.6		10.2	9.1	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		23.2			23.2		9.1	12.6		10.2	9.1	
LOS		C			C		A	B		B	A	
Approach Delay		23.2			23.2			12.4		_	9.4	
Approach LOS		C			C			В			A	
Queue Length 50th (m)		13.2			21.6		1.8	34.2		3.6	12.0	
Queue Length 95th (m)		26.9			42.9		6.8	70.6		11.6	27.8	
Internal Link Dist (m)		280.3			317.1		0.0	223.9		11.0	1432.2	
Turn Bay Length (m)		200.0			V 1111		40.0	220.0		45.0	1102.2	
Base Capacity (vph)		554			644		523	966		405	969	
		007			777		525	500		-100	505	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.23			0.39		0.06	0.51		0.15	0.22	
Intersection Summary												
Area Type:	Other											
Cycle Length: 77												
Actuated Cycle Length: 67	7.6											
Natural Cycle: 60												
Control Type: Semi Act-U	ncoord											
Maximum v/c Ratio: 0.60												
Intersection Signal Delay:	15.2			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 73.7%			IC	U Level o	of Service	D					
Analysis Period (min) 15												

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	23	16	43	32	13	22	67	341	54	16	215	7
Future Volume (Veh/h)	23	16	43	32	13	22	67	341	54	16	215	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	24	17	45	34	14	23	71	359	57	17	226	7
Pedestrians		5			3							
Lane Width (m)		3.6			3.6							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	828	830	234	850	804	390	238			419		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	828	830	234	850	804	390	238			419		
tC, single (s)	7.3	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.6	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	90	94	94	85	95	97	95			99		
cM capacity (veh/h)	240	285	806	226	295	661	1312			1148		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	86	71	487	250								
Volume Left	24	34	71	17								
Volume Right	45	23	57	7								
cSH	399	305	1312	1148								
Volume to Capacity	0.22	0.23	0.05	0.01								
Queue Length 95th (m)	6.5	7.1	1.4	0.4								
Control Delay (s)	16.5	20.3	1.6	0.7								
Lane LOS	С	С	A	А								
Approach Delay (s)	16.5	20.3	1.6	0.7								
Approach LOS	С	С										
Intersection Summary												
Average Delay			4.3									
Intersection Capacity Utilization	n		54.2%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 5: Trafalgar Road North & Upper Canada Drive/Church Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			÷			\$	
Traffic Volume (veh/h)	0	0	2	14	0	5	17	378	20	6	240	1
Future Volume (Veh/h)	0	0	2	14	0	5	17	378	20	6	240	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	0	0	2	14	0	5	18	390	21	6	247	1
Pedestrians					1			5				
Lane Width (m)					3.6			3.6				
Walking Speed (m/s)					1.2			1.2				
Percent Blockage					0			0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	701	708	252	704	698	402	248			412		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	701	708	252	704	698	402	248			412		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	96	100	99	99			99		
cM capacity (veh/h)	348	355	788	336	360	653	1330			1157		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	2	19	429	254								
Volume Left	0	14	18	6								
Volume Right	2	5	21	1								
cSH	788	385	1330	1157								
Volume to Capacity	0.00	0.05	0.01	0.01								
Queue Length 95th (m)	0.1	1.2	0.3	0.1								
Control Delay (s)	9.6	14.8	0.5	0.2								
Lane LOS	А	В	А	А								
Approach Delay (s)	9.6	14.8	0.5	0.2								
Approach LOS	А	В										
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization	tion		43.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									
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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4			र्भ
Traffic Volume (veh/h)	4	0	366	10	6	250
Future Volume (Veh/h)	4	0	366	10	6	250
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	0	398	11	7	272
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	690	404			409	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	690	404			409	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	100			99	
cM capacity (veh/h)	412	651			1161	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	4	409	279			
Volume Left	4	0	7			
Volume Right	0	11	0			
cSH	412	1700	1161			
Volume to Capacity	0.01	0.24	0.01			
Queue Length 95th (m)	0.2	0.0	0.1			
Control Delay (s)	13.8	0.0	0.3			
Lane LOS	В		А			
Approach Delay (s)	13.8	0.0	0.3			
Approach LOS	В					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliza	ation		29.9%	IC	U Level o	of Service
Analysis Period (min)			15			
			10			

Lane Configurations Image: Configuration in the image: Configuration in th	ST SBR 51 30 51 30
Lane Configurations Image: Configuration of the second secon	51 30
Traffic Volume (vph)30170120169864039172949333Future Volume (vph)30170120169864039172949333	51 30
Future Volume (vph) 30 170 120 169 86 40 39 172 94 93 3	
	JI JU
	00 1900
Storage Length (m) 0.0 0.0 0.0 0.0 40.0 0.0 45.0	0.0
Storage Lanes 0 0 0 0 1 0 1	0.0
Taper Length (m) 7.5 7.5 7.5	·
	69 0
Fit Permitted 0.940 0.577 0.422 0.556	
	69 0
Right Turn on Red Yes Yes Yes	Yes
Satd. Flow (RTOR) 43 11 47	7
Link Speed (k/h) 70 70 40	40
Link Distance (m) 304.3 341.1 247.9 145	
Travel Time (s) 15.6 17.5 22.3 13	
	.% 18%
Shared Lane Traffic (%)	1070
	37 0
	IA
Protected Phases 4 8 2	6
Permitted Phases 4 8 2 6	Ū
Detector Phase 4 4 8 8 2 2 6	6
Switch Phase	•
	0.0
	.5
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Total Split (%) 44.8% 44.8% 44.8% 44.8% 55.2% 55.2% 55.2% 55.2% 55.2%	
	5.5
	2.0
	.0
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Lead/Lag	
Lead-Lag Optimize?	
	ах
	5.0
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LOS C F B B B	В
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Approach LOS C F B	В
	5.5
o (<i>i</i>)	.0
Internal Link Dist (m) 280.3 317.1 223.9 143	
Turn Bay Length (m) 40.0 45.0	
	62

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.61			1.02		0.12	0.42		0.27	0.57	
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 75												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 1.02												
Intersection Signal Delay: 31	.9			In	itersectior	LOS: C						
Intersection Capacity Utilizati	on 109.2%	1		IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
 Volume exceeds capacity 	/, queue is	theoretica	ally infinit	e.								
Queue shown is maximun	n after two	cycles.										
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximun	n after two	cycles.										
Splits and Phases: 11: Tra	falgar Roa	d North &	Wellingt	on Road	22							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		- 4 >			- ↔			- ↔			- ↔	
Traffic Volume (veh/h)	1	1	8	34	0	13	12	212	14	2	335	1
Future Volume (Veh/h)	1	1	8	34	0	13	12	212	14	2	335	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	1	9	37	0	14	13	230	15	2	364	1
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	646	640	366	642	632	238	365			245		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	646	640	366	642	632	238	365			245		
tC, single (s)	7.1	6.5	6.4	7.2	6.5	6.3	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.6	4.0	3.4	2.2			2.2		
p0 queue free %	100	100	99	90	100	98	99			100		
cM capacity (veh/h)	377	391	647	370	395	784	1205			1333		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	11	51	258	367								
Volume Left	1	37	13	2								
Volume Right	9	14	15	1								
cSH	575	433	1205	1333								
Volume to Capacity	0.02	0.12	0.01	0.00								
Queue Length 95th (m)	0.5	3.2	0.3	0.0								
Control Delay (s)	11.4	14.4	0.5	0.1								
Lane LOS	В	В	A	A								
Approach Delay (s)	11.4	14.4	0.5	0.1								
Approach LOS	В	В										
Intersection Summary												
Average Delay			1.5									
Intersection Capacity Utilization	on		36.5%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	0	19	12	1	5	2	168	5	2	259	1
Future Volume (Veh/h)	1	0	19	12	1	5	2	168	5	2	259	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	1	0	23	15	1	6	2	207	6	2	320	1
Pedestrians								4				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	545	542	324	566	539	210	321			213		
vC1, stage 1 conf vol	0.10	0.12	021	000		210	021			210		
vC2, stage 2 conf vol												
vCu, unblocked vol	545	542	324	566	539	210	321			213		
tC, single (s)	7.1	6.5	6.2	7.1	7.5	6.2	4.1			4.1		
tC, 2 stage (s)	7.1	0.0	0.2	7.1	1.0	0.2	1.1					
tF (s)	3.5	4.0	3.3	3.5	4.9	3.3	2.2			2.2		
p0 queue free %	100	100	97	96	100	99	100			100		
cM capacity (veh/h)	447	449	719	422	336	835	1250			1369		
					550	000	1200			1000		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	24	22	215	323								
Volume Left	1	15	2	2								
Volume Right	23	6	6	1								
cSH	701	481	1250	1369								
Volume to Capacity	0.03	0.05	0.00	0.00								
Queue Length 95th (m)	0.8	1.1	0.0	0.0								
Control Delay (s)	10.3	12.8	0.1	0.1								
Lane LOS	В	В	А	Α								
Approach Delay (s)	10.3	12.8	0.1	0.1								
Approach LOS	В	В										
Intersection Summary												
Average Delay			1.0									
Intersection Capacity Utilization	on		27.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4			र्भ
Traffic Volume (veh/h)	1	4	166	1	4	220
Future Volume (Veh/h)	1	4	166	1	4	220
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1	4	171	1	4	227
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	406	172			172	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	406	172			172	
tC, single (s)	7.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	4.4	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	449	877			1417	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	5	172	231			
Volume Left	1	0	4			
Volume Right	4	1	0			
cSH	737	1700	1417			
Volume to Capacity	0.01	0.10	0.00			
Queue Length 95th (m)	0.2	0.0	0.1			
Control Delay (s)	9.9	0.0	0.2			
Lane LOS	A		A			
Approach Delay (s)	9.9	0.0	0.2			
Approach LOS	A					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliza	ation		24.8%	IC	U Level o	of Service
Analysis Period (min)			15	.0	2 _ 5 . 6 . 6	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	4Î		ሻ	ef 👘	
Traffic Volume (vph)	54	130	68	122	192	114	123	536	198	65	261	34
Future Volume (vph)	54	130	68	122	192	114	123	536	198	65	261	34
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5		, e	7.5		•	7.5		•	7.5		·
Satd. Flow (prot)	0	1726	0	0	1761	0	1504	1783	0	1703	1819	0
Flt Permitted	Ŭ	0.792	Ŭ	Ū	0.802	•	0.540		Ŭ	0.114	1010	Ū
Satd. Flow (perm)	0	1382	0	0	1432	0	855	1783	0	204	1819	0
Right Turn on Red	Ŭ	1002	Yes	Ū	1102	Yes	000		Yes	201	1010	Yes
Satd. Flow (RTOR)		27	100		26	100		32	100		11	100
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	2%	14%	4%	2%	2%	20%	3%	0%	6%	3%	0%
Shared Lane Traffic (%)	0,0	270	1170	170	270	270	2070	0,0	0,0	0,0	0,0	0,0
Lane Group Flow (vph)	0	277	0	0	470	0	135	807	0	71	324	0
Turn Type	Perm	NA	Ŭ	Perm	NA	Ŭ	Perm	NA	Ŭ	Perm	NA	Ű
Protected Phases		4			8			2			6	
Permitted Phases	4	•		8	Ű		2	-		6	Ŭ	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	·	·		•	Ū		_	_		Ţ	Ŭ	
Minimum Initial (s)	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		37.5	37.5		37.5	37.5	
Total Split (s)	34.5	34.5		34.5	34.5		42.5	42.5		42.5	42.5	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%	55.2%		55.2%	55.2%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		5.5	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		7.5			7.5		7.5	7.5		7.5	7.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		26.0			26.0		35.0	35.0		35.0	35.0	
Actuated g/C Ratio		0.34			0.34		0.46	0.46		0.46	0.46	
v/c Ratio		0.57			0.93		0.34	0.96		0.76	0.38	
Control Delay		23.5			50.6		16.7	45.0		69.8	14.9	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		23.5			50.6		16.7	45.0		69.8	14.9	
LOS		С			D		В	D		E	В	
Approach Delay		23.5			50.6			41.0			24.8	
Approach LOS		С			D			D			С	
Queue Length 50th (m)		30.2			64.1		12.8	112.6		8.7	30.3	
Queue Length 95th (m)		54.3			#122.7		26.5	#193.5		#33.3	49.8	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)							40.0			45.0		
Base Capacity (vph)		508			526		393	838		93	843	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.55			0.89		0.34	0.96		0.76	0.38	
Intersection Summary												
Area Type:	Other											
Cycle Length: 77												
Actuated Cycle Length: 76												
Natural Cycle: 90												
Control Type: Semi Act-Un	icoord											
Maximum v/c Ratio: 0.96												
Intersection Signal Delay:	37.7			In	tersectior	LOS: D						
Intersection Capacity Utiliz	ation 119.6%)		IC	U Level o	of Service	Н					
Analysis Period (min) 15												
# 95th percentile volume	exceeds cap	acity, que	eue may l	be longer								
Queue shown is maxim	um after two	cycles.										

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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42.5 s	34.5 s	
Ø6	₩ Ø8	
42.5 s	34.5 s	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		- 4 >			- 4 >			- 4 >			4	
Traffic Volume (veh/h)	23	16	48	34	13	22	71	397	54	16	275	7
Future Volume (Veh/h)	23	16	48	34	13	22	71	397	54	16	275	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	24	17	51	36	14	23	75	418	57	17	289	7
Pedestrians		5			3							
Lane Width (m)		3.6			3.6							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	958	960	298	986	934	450	301			478		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	958	960	298	986	934	450	301			478		
tC, single (s)	7.3	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.6	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	88	93	93	80	94	96	94			98		
cM capacity (veh/h)	192	238	744	177	246	612	1244			1092		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	73	550	313								
Volume Left	24	36	75	17								
Volume Right	51	23	57	7								
cSH	347	245	1244	1092								
Volume to Capacity	0.26	0.30	0.06	0.02								
Queue Length 95th (m)	8.4	9.6	1.5	0.4								
Control Delay (s)	19.1	25.8	1.7	0.6								
Lane LOS	C	D	A	A								
Approach Delay (s)	19.1	25.8	1.7	0.6								
Approach LOS	C	20.0 D	1.7	0.0								
Intersection Summary												
Average Delay			4.6									
Intersection Capacity Utilizati	on		60.9%	IC	Ulevelo	of Service			В			
Analysis Period (min)	~		15						D			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		- 4			- ↔			4			4	
Traffic Volume (veh/h)	0	0	2	14	0	5	17	417	20	6	265	1
Future Volume (Veh/h)	0	0	2	14	0	5	17	417	20	6	265	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	0	0	2	14	0	5	18	430	21	6	273	1
Pedestrians					1			5				
Lane Width (m)					3.6			3.6				
Walking Speed (m/s)					1.2			1.2				
Percent Blockage					0			0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	767	774	278	770	764	442	274			452		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	767	774	278	770	764	442	274			452		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	95	100	99	99			99		
cM capacity (veh/h)	314	325	762	303	330	620	1301			1118		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	2	19	469	280								
Volume Left	0	14	18	6								
Volume Right	2	5	21	1								
cSH	762	350	1301	1118								
Volume to Capacity	0.00	0.05	0.01	0.01								
Queue Length 95th (m)	0.1	1.4	0.3	0.1								
Control Delay (s)	9.7	15.9	0.4	0.2								
Lane LOS	А	С	А	А								
Approach Delay (s)	9.7	15.9	0.4	0.2								
Approach LOS	А	С										
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization	n		46.1%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		eî.			र्स
Traffic Volume (veh/h)	4	0	404	10	6	276
Future Volume (Veh/h)	4	0	404	10	6	276
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	0	439	11	7	300
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	758	444			450	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	758	444			450	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	100			99	
cM capacity (veh/h)	375	618			1121	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	4	450	307			
Volume Left	4	0	7			
Volume Right	0	11	0			
cSH	375	1700	1121			
Volume to Capacity	0.01	0.26	0.01			
Queue Length 95th (m)	0.3	0.20	0.2			
Control Delay (s)	14.7	0.0	0.2			
Lane LOS	В	0.0	A A			
Approach Delay (s)	14.7	0.0	0.2			
Approach LOS	14.7 B	0.0	0.2			
Approach LOS	D					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliz	ation		31.9%	IC	U Level o	of Service
Analysis Period (min)			15			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		٦	¢Î,		ሻ	eî 🗧	
Traffic Volume (vph)	33	178	124	176	91	44	40	186	100	103	376	33
Future Volume (vph)	33	178	124	176	91	44	40	186	100	103	376	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5			7.5		·	7.5		•	7.5		
Satd. Flow (prot)	0	1736	0	0	1569	0	1805	1559	0	1492	1669	0
Flt Permitted	•	0.935			0.564	·	0.390		•	0.532		
Satd. Flow (perm)	0	1632	0	0	909	0	741	1559	0	835	1669	0
Right Turn on Red	Ŭ	1002	Yes	Ū	000	Yes	•••	1000	Yes	000	1000	Yes
Satd. Flow (RTOR)		42	100		12	100		46	100		8	100
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0.07	19%	9%	21%	12%	18%
Shared Lane Traffic (%)	570	2 /0	7/0	2070	570	2070	0 /0	1370	570	2170	12/0	10 /0
Lane Group Flow (vph)	0	386	0	0	358	0	46	329	0	118	470	0
Turn Type	Perm	NA	0	Perm	NA	0	Perm	NA	0	Perm	NA	0
Protected Phases	Feilli	4		Feilli	8		Feilii	2		Feilli	6	
Permitted Phases	4	4		8	0		2	2		6	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	4	4		0	0		2	2		0	0	
	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Initial (s)	22.5	22.5		22.5	22.5		30.0	30.0		30.0	30.0	
Minimum Split (s)	22.5 34.5	22.5 34.5		22.5 34.5	22.5 34.5		42.5	37.5 42.5		37.5 42.5	37.5 42.5	
Total Split (s)								42.5 55.2%			42.5 55.2%	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%			55.2%		
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5 2.0	5.5		5.5 2.0	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0			2.0			2.0	
Lost Time Adjust (s)		0.0			0.0 7.5		0.0	0.0		0.0 7.5	0.0	
Total Lost Time (s)		7.5			<i>I</i> .5		7.5	7.5		C.1	7.5	
Lead/Lag												
Lead-Lag Optimize?	Mana	Nana		Neze	Nama		Max	Max		Max	Max	
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		27.0			27.0		35.0	35.0		35.0	35.0	
Actuated g/C Ratio		0.35			0.35		0.45	0.45		0.45	0.45	_
v/c Ratio		0.64			1.10		0.14	0.45		0.31	0.62	
Control Delay		24.5			106.0		13.6	14.6		16.2	20.0	_
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		24.5			106.0		13.6	14.6		16.2	20.0	_
LOS Approach Dalay		C			F		В	B		В	B	
Approach Delay		24.5			106.0			14.5			19.2	_
Approach LOS		C			F		0.0	B		44.0	B	
Queue Length 50th (m)		43.4			~62.2		3.9	27.9		11.0	51.3	_
Queue Length 95th (m)		69.5			#108.5		9.9	46.8		22.3	78.0	
Internal Link Dist (m)		280.3			317.1		40.0	223.9		4= 4	1432.2	_
Turn Bay Length (m)							40.0			45.0		
Base Capacity (vph)		599			326		336	733		379	763	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.64			1.10		0.14	0.45		0.31	0.62	
Intersection Summary												
Area Type: O	ther											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 80												
Control Type: Semi Act-Uncod	ord											
Maximum v/c Ratio: 1.10												
Intersection Signal Delay: 37.6					ntersectior							
Intersection Capacity Utilization	on 111.0%	1		IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
 Volume exceeds capacity, 	, queue is	theoretica	ally infinit	e.								
Queue shown is maximum	after two	cycles.										
# 95th percentile volume exercise	ceeds cap	acity, que	eue may l	be longer	٢.							
Queue shown is maximum	after two	cycles.										
Splits and Phases: 11: Traf	algar Roa	d North &	Wellingt	on Road	22							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		- 4 >			- ↔			- ↔			4	
Traffic Volume (veh/h)	1	1	8	34	0	13	12	230	14	2	369	1
Future Volume (Veh/h)	1	1	8	34	0	13	12	230	14	2	369	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	1	9	37	0	14	13	250	15	2	401	1
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	703	696	402	700	690	258	402			265		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	703	696	402	700	690	258	402			265		
tC, single (s)	7.1	6.5	6.4	7.2	6.5	6.3	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.6	4.0	3.4	2.2			2.2		
p0 queue free %	100	100	99	89	100	98	99			100		
cM capacity (veh/h)	345	363	616	338	366	764	1168			1311		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	11		278	404								
		51										_
Volume Left	1	37	13	2								
Volume Right	9	14	15	1								_
cSH	543	399	1168	1311								
Volume to Capacity	0.02	0.13	0.01	0.00								
Queue Length 95th (m)	0.5	3.5	0.3	0.0								
Control Delay (s)	11.8	15.3	0.5	0.1								_
Lane LOS	В	C	A	A								
Approach Delay (s)	11.8	15.3	0.5	0.1								_
Approach LOS	В	С										
Intersection Summary												
Average Delay			1.4									
Intersection Capacity Utilizatio	n		37.5%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	0	19	12	1	5	2	185	5	2	286	1
Future Volume (Veh/h)	1	0	19	12	1	5	2	185	5	2	286	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	1	0	23	15	1	6	2	228	6	2	353	1
Pedestrians								4				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	599	596	358	620	593	231	354			234		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	599	596	358	620	593	231	354			234		
tC, single (s)	7.1	6.5	6.2	7.1	7.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.9	3.3	2.2			2.2		
p0 queue free %	100	100	97	96	100	99	100			100		
cM capacity (veh/h)	411	419	689	388	310	813	1216			1345		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	24	22	236	356								
Volume Left	1	15	2	2								
Volume Right	23	6	6	1								
cSH	670	446	1216	1345								
Volume to Capacity	0.04	0.05	0.00	0.00								
Queue Length 95th (m)	0.9	1.2	0.0	0.0								
Control Delay (s)	10.6	13.5	0.0	0.0								
Lane LOS	B	B	A	A								
Approach Delay (s)	10.6	13.5	0.1	0.1								
Approach LOS	B	B	0.1	0.1								
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utilization	n		29.1%	IC	U Level o	of Service			А			
Analysis Period (min)			15		2 20.014							

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4			4	
Traffic Volume (veh/h)	1	4	183	1	4	243	
Future Volume (Veh/h)	1	4	183	1	4	243	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	1	4	189	1	4	251	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	448	190			190		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	448	190			190		
tC, single (s)	7.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	4.4	3.3			2.2		
p0 queue free %	100	100			100		
cM capacity (veh/h)	422	857			1396		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	5	190	255				
Volume Left	1	0	4				
Volume Right	4	1	0				
cSH	711	1700	1396				
Volume to Capacity	0.01	0.11	0.00				
Queue Length 95th (m)	0.2	0.0	0.1				
Control Delay (s)	10.1	0.0	0.1				
Lane LOS	В		А				
Approach Delay (s)	10.1	0.0	0.1				
Approach LOS	В						
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utiliza	ation		26.0%	IC	U Level o	of Service	
Analysis Period (min)			15				
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		٦	4Î		ሻ	4Î	
Traffic Volume (vph)	60	137	69	128	200	126	127	578	207	71	281	37
Future Volume (vph)	60	137	69	128	200	126	127	578	207	71	281	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	1000	0.0	0.0	1000	0.0	40.0	1000	0.0	45.0	1000	0.0
Storage Lanes	0		0	0		0	1		0.0	10.0		0
Taper Length (m)	7.5		Ū	7.5		Ŭ	7.5		Ŭ	7.5		Ū
Satd. Flow (prot)	0	1733	0	0	1759	0	1504	1785	0	1703	1818	0
Flt Permitted	U	0.765	Ū	v	0.794	Ū	0.510	1100	Ū	0.114	1010	v
Satd. Flow (perm)	0	1340	0	0	1416	0	808	1785	0	204	1818	0
Right Turn on Red	0	1040	Yes	Ū	1410	Yes	000	1700	Yes	204	1010	Yes
Satd. Flow (RTOR)		25	103		28	103		31	103		11	103
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0.91	2%	14%	4%	2%	2%	20%	3%	0.91	6%	3%	0.91
Shared Lane Traffic (%)	0 /0	∠ /0	14 /0	4 /0	۷ ک	∠ /0	20 /0	J /0	0 /0	0 /0	J /0	0 /0
Lane Group Flow (vph)	0	293	0	0	499	0	140	862	0	78	350	0
	Perm	Z95 NA	0	Perm	499 NA	0	Perm	NA	0	Perm	NA	0
Turn Type Protected Phases	Feilii	4		Feim	NA 8		Feilii	NA 2		Feim	6	
Protected Phases	4	4		8	0		2	Z		6	0	
Detector Phase	4	4		o 8	8		2	2		6	6	
Switch Phase	4	4		0	0		Z	Z		0	0	
	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Minimum Initial (s)	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		37.5	37.5		37.5	37.5 42.5	
Total Split (s)	34.5	34.5		34.5	34.5		42.5	42.5		42.5		
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%	55.2%		55.2%	55.2%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		5.5	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	_
Total Lost Time (s)		7.5			7.5		7.5	7.5		7.5	7.5	
Lead/Lag												_
Lead-Lag Optimize?	Nama	Nana		Nana	Mana		Max	Max		Max	Max	
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		27.0			27.0		35.0	35.0		35.0	35.0	
Actuated g/C Ratio		0.35			0.35		0.45	0.45		0.45	0.45	_
v/c Ratio		0.60			0.97		0.38	1.04		0.85	0.42	
Control Delay		25.0			59.1		17.8	65.2		86.5	15.6	_
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		25.0			59.1		17.8	65.2		86.5	15.6	_
LOS Annarach Dalau		C			E		В	E		F	B	
Approach Delay		25.0			59.1			58.6			28.5	
Approach LOS		C			E		40.0	E		40.4	C	
Queue Length 50th (m)		33.1			70.6		13.6	~142.8		10.1	33.4	_
Queue Length 95th (m)		59.3			#133.9		28.3	#213.2		#36.6	54.4	
Internal Link Dist (m)		280.3			317.1		10.0	223.9		1 = 4	1432.2	
Turn Bay Length (m)							40.0			45.0		
Base Capacity (vph)		486			514		367	828		92	832	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.60			0.97		0.38	1.04		0.85	0.42	
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 90												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 1.04												
Intersection Signal Delay: 48.5			In	tersection	LOS: D							
Intersection Capacity Utilization 123.6%			CU Level o	of Service	Н							
Analysis Period (min) 15												
 Volume exceeds capacity, queue is theoretically infinite. 												
Queue shown is maximum after two cycles.												
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximum after two cycles.												
Splits and Phases: 11: Trafalgar Road North & Wellington Road 22												

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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42.5 s	34.5 s
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42.5 s	34.5 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	23	16	48	34	13	22	71	437	54	16	300	7
Future Volume (Veh/h)	23	16	48	34	13	22	71	437	54	16	300	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	24	17	51	36	14	23	75	460	57	17	316	7
Pedestrians		5			3							
Lane Width (m)		3.6			3.6							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1027	1028	324	1054	1004	492	328			520		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1027	1028	324	1054	1004	492	328			520		
tC, single (s)	7.3	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.6	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	86	92	93	77	94	96	94			98		
cM capacity (veh/h)	171	216	718	157	224	580	1215			1054		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	73	592	340								
Volume Left	24	36	75	17								
Volume Right	51	23	57	7								
cSH	317	220	1215	1054								
Volume to Capacity	0.29	0.33	0.06	0.02								
Queue Length 95th (m)	9.4	11.1	1.6	0.4								
Control Delay (s)	20.9	29.2	1.0	0.4								
Lane LOS	20.0 C	D	Α	A								
Approach Delay (s)	20.9	29.2	1.7	0.6								
Approach LOS	20.5 C	20.2 D	1.7	0.0								
Intersection Summary												
Average Delay			4.8									
Intersection Capacity Utilizat	ion		64.4%	IC	Ulevelo	of Service			С			
Analysis Period (min)			15						Ŭ			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Volume (veh/h)	0	0	2	14	0	5	17	461	20	6	293	1
Future Volume (Veh/h)	0	0	2	14	0	5	17	461	20	6	293	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	0	0	2	14	0	5	18	475	21	6	302	1
Pedestrians					1			5				
Lane Width (m)					3.6			3.6				
Walking Speed (m/s)					1.2			1.2				
Percent Blockage					0			0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	841	848	308	844	838	486	303			497		
vC1, stage 1 conf vol	•	0.0		• • •								
vC2, stage 2 conf vol												
vCu, unblocked vol	841	848	308	844	838	486	303			497		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	•		0.0	•.=						
tF (s)	3.5	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	95	100	99	99			99		
cM capacity (veh/h)	280	295	734	270	299	585	1269			1076		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1	200		1200			1010		
Volume Total	2	19	514	309								_
Volume Left	0	14	18	6								
Volume Right	2	5	21	1								
cSH	734	315	1269	1076								
Volume to Capacity	0.00	0.06	0.01	0.01								_
Queue Length 95th (m)	0.1	1.5	0.3	0.1								
Control Delay (s)	9.9	17.2	0.4	0.2								
Lane LOS	А	С	А	А								
Approach Delay (s)	9.9	17.2	0.4	0.2								_
Approach LOS	А	С										
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization	on		48.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Υ		4			र्भ
Traffic Volume (veh/h)	4	0	446	10	6	305
Future Volume (Veh/h)	4	0	446	10	6	305
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	0	485	11	7	332
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	836	490			496	
vC1, stage 1 conf vol	000	100			100	
vC2, stage 2 conf vol						
vCu, unblocked vol	836	490			496	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	100			99	
cM capacity (veh/h)	337	582			1078	
,					1010	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	4	496	339			
Volume Left	4	0	7			
Volume Right	0	11	0			
cSH	337	1700	1078			
Volume to Capacity	0.01	0.29	0.01			
Queue Length 95th (m)	0.3	0.0	0.2			
Control Delay (s)	15.8	0.0	0.2			
Lane LOS	С		А			
Approach Delay (s)	15.8	0.0	0.2			
Approach LOS	С					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliza	ation		34.1%	IC	U Level o	of Service
Analysis Period (min)			15		0 2010. (
			10			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		٦	ef 👘		5	eî 🗧	
Traffic Volume (vph)	30	170	120	169	86	66	39	195	94	237	482	30
Future Volume (vph)	30	170	120	169	86	66	39	195	94	237	482	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5			7.5		·	7.5		, e	7.5		
Satd. Flow (prot)	0	1735	0	0	1551	0	1805	1561	0	1492	1676	0
Flt Permitted	•	0.939		•	0.596	·	0.280		, e	0.528		
Satd. Flow (perm)	0	1637	0	0	949	0	532	1561	0	829	1676	0
Right Turn on Red	Ū	1001	Yes	U	010	Yes	002	1001	Yes	020	1010	Yes
Satd. Flow (RTOR)		43	100		19	100		41	100		5	100
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0.07	19%	9%	21%	12%	18%
Shared Lane Traffic (%)	570	2 /0	7/0	2070	570	2070	0 /0	1370	570	2170	12/0	10 /0
Lane Group Flow (vph)	0	367	0	0	369	0	45	332	0	272	588	0
Turn Type	Perm	NA	0	Perm	NA	0	Perm	NA	0	Perm	NA	0
Protected Phases	Feilli	4		Feilli	NA 8		Feilii	2		Feilii	6	
Permitted Phases	4	4		8	0		2	2		6	0	
	4	4		o 8	8		2	2		6 6	6	
Detector Phase Switch Phase	4	4		0	0		Z	Z		0	0	
	15.0	15.0		15.0	15.0		20.0	20.0		20.0	30.0	
Minimum Initial (s)	15.0	15.0		15.0	15.0		30.0	30.0		30.0		_
Minimum Split (s)	22.5	22.5		22.5	22.5		37.5	37.5		37.5	37.5	
Total Split (s)	34.5	34.5		34.5	34.5		42.5	42.5		42.5	42.5	_
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%	55.2%		55.2%	55.2%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		5.5	5.5	_
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	_
Total Lost Time (s)		7.5			7.5		7.5	7.5		7.5	7.5	
Lead/Lag												_
Lead-Lag Optimize?	NI.	NL		NL	NL		Ν.			M.	Ν4.	
Recall Mode	None	None		None	None		Max	Max		Max	Max	_
Act Effct Green (s)		27.0			27.0		35.0	35.0		35.0	35.0	
Actuated g/C Ratio		0.35			0.35		0.45	0.45		0.45	0.45	_
v/c Ratio		0.61			1.07		0.19	0.45		0.72	0.77	
Control Delay		23.3			95.1		15.1	15.0		30.8	26.0	_
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		23.3			95.1		15.1	15.0		30.8	26.0	
LOS		С			F		В	В		С	С	
Approach Delay		23.3			95.1			15.0			27.5	
Approach LOS		С			F			В			С	
Queue Length 50th (m)		40.1			~61.9		3.9	28.9		32.4	71.9	
Queue Length 95th (m)		64.7			#108.6		10.4	47.9		#68.1	107.6	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)							40.0			45.0		
Base Capacity (vph)		601			345		241	731		376	764	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.61			1.07		0.19	0.45		0.72	0.77	
Intersection Summary												
Area Type:	Other											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 75												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 1.07												
Intersection Signal Delay: 3	7.0			In	tersectior	LOS: D						
Intersection Capacity Utiliza	tion 113.0%			IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
~ Volume exceeds capaci	ty, queue is	theoretic	ally infinit	e.								
Queue shown is maximu	m after two	cycles.										
# 95th percentile volume e	exceeds cap	acity, que	eue may l	be longer								
Queue shown is maximu	m after two	cycles.										
Splits and Phases: 11: Tr	afalgar Road	d North &	Wellingt	on Road	22							

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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42.5 s	34.5 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	5	1	8	34	0	36	12	262	14	2	617	1
Future Volume (Veh/h)	5	1	8	34	0	36	12	262	14	2	617	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	1	9	37	0	39	13	285	15	2	671	1
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1033	1002	672	1004	994	292	672			300		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1033	1002	672	1004	994	292	672			300		
tC, single (s)	7.1	6.5	6.4	7.2	6.5	6.3	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.6	4.0	3.4	2.2			2.2		
p0 queue free %	97	100	98	82	100	95	99			100		
cM capacity (veh/h)	199	241	430	208	243	730	928			1273		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1	-					-		
Volume Total	15	76	313	674								
Volume Left	5	37	13	2								
Volume Right	9	39	15	1								
cSH	299	328	928	1273								
	0.05	0.23	920 0.01	0.00								
Volume to Capacity	1.3	7.0	0.01	0.00								
Queue Length 95th (m)												
Control Delay (s)	17.7	19.2	0.5	0.0								
Lane LOS	C	C	A	A								
Approach Delay (s)	17.7	19.2	0.5	0.0								
Approach LOS	С	С										
Intersection Summary												
Average Delay			1.8									
Intersection Capacity Utilization	on		46.0%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	10	0	19	12	1	27	2	245	5	2	543	1
Future Volume (Veh/h)	10	0	19	12	1	27	2	245	5	2	543	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	12	0	23	15	1	33	2	302	6	2	670	1
Pedestrians								4				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
	1017	986	674	1010	984	305	671			308		
vC1, stage 1 conf vol	1011	000	011	1010	001	000	011			000		
vC2, stage 2 conf vol												
	1017	986	674	1010	984	305	671			308		
tC, single (s)	7.1	6.5	6.2	7.1	7.5	6.2	4.1			4.1		
tC, 2 stage (s)	7.1	0.0	0.2	7.1	7.0	0.2	7.1					
tF (s)	3.5	4.0	3.3	3.5	4.9	3.3	2.2			2.2		
p0 queue free %	94	100	95	93	99	96	100			100		
cM capacity (veh/h)	207	249	456	208	171	740	929			1264		
					171	740	929			1204		
	EB 1	WB 1	NB 1	SB 1								
Volume Total	35	49	310	673								
Volume Left	12	15	2	2								
Volume Right	23	33	6	1								
cSH	323	399	929	1264								
Volume to Capacity	0.11	0.12	0.00	0.00								
Queue Length 95th (m)	2.9	3.3	0.1	0.0								
Control Delay (s)	17.5	15.3	0.1	0.0								
Lane LOS	С	С	А	А								
Approach Delay (s)	17.5	15.3	0.1	0.0								
Approach LOS	С	С										
Intersection Summary												
Average Delay			1.3									
Intersection Capacity Utilization			41.2%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	14	0	195	1	27	4	79	177	1	4	224	2
Future Volume (Veh/h)	14	0	195	1	27	4	79	177	1	4	224	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	14	0	201	1	28	4	81	182	1	4	231	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	602	585	232	786	586	182	233			183		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	602	585	232	786	586	182	233			183		
tC, single (s)	7.1	6.5	6.2	8.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	4.4	4.0	3.3	2.2			2.2		
p0 queue free %	96	100	75	99	93	100	94			100		
cM capacity (veh/h)	369	396	807	155	396	865	1335			1404		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	215	33	264	237								
Volume Left	14	1	81	4								
Volume Right	201	4	1	2								
cSH	749	403	1335	1404								
Volume to Capacity	0.29	0.08	0.06	0.00								
Queue Length 95th (m)	9.5	2.1	1.5	0.1								
Control Delay (s)	11.7	14.7	2.8	0.2								
Lane LOS	В	В	A	A								
Approach Delay (s)	11.7	14.7	2.8	0.2								
Approach LOS	В	В		•								
Intersection Summary												
Average Delay			5.0									
Intersection Capacity Utilizat	ion		55.0%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	Y			र्भ	4		_	
Traffic Volume (veh/h)	11	89	29	253	416	4		
Future Volume (Veh/h)	11	89	29	253	416	4		
Sign Control	Stop			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	12	97	32	275	452	4		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type				None	None			
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	793	454	456					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	793	454	456					
tC, single (s)	6.4	6.2	4.1					
tC, 2 stage (s)								
tF (s)	3.5	3.3	2.2					
p0 queue free %	97	84	97					
cM capacity (veh/h)	347	606	1105					
Direction, Lane #	EB 1	NB 1	SB 1					
Volume Total	109	307	456					
Volume Left	12	32	0					
Volume Right	97	0	4					
cSH	560	1105	1700					
Volume to Capacity	0.19	0.03	0.27					
Queue Length 95th (m)	5.7	0.7	0.0					
Control Delay (s)	13.0	1.1	0.0					
Lane LOS	В	А						
Approach Delay (s)	13.0	1.1	0.0					
Approach LOS	В							
Intersection Summary								
Average Delay			2.0					
Intersection Capacity Utiliza	tion		50.3%	IC	CU Level c	f Service		
Analysis Period (min)			15					
			10					

Intersection Intersection Delay, s/veh	4.8			
Intersection LOS	4.0 A			
	A			
Approach	EB	WB	NB	
Entry Lanes	1	1	1	
Conflicting Circle Lanes	1	1	1	
Adj Approach Flow, veh/h	223	116	28	
Demand Flow Rate, veh/h	227	118	29	
Vehicles Circulating, veh/h	1	25	227	
Vehicles Exiting, veh/h	142	230	1	
Follow-Up Headway, s	3.186	3.186	3.186	
Ped Vol Crossing Leg, #/h	0	0	0	
Ped Cap Adj	1.000	1.000	1.000	
Approach Delay, s/veh	5.1	4.3	4.4	
Approach LOS	А	А	А	
Lane	Left	Left	Left	
Designated Moves	TR	LT	LR	
Assumed Moves	TR	LT	LR	
	TR	LT	LR	
RT Channelized	TR 1.000	LT 1.000	LR 1.000	
RT Channelized Lane Util				
RT Channelized Lane Util Critical Headway, s	1.000	1.000	1.000	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h	1.000 5.193	1.000 5.193	1.000 5.193	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h	1.000 5.193 227	1.000 5.193 118	1.000 5.193 29	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	1.000 5.193 227 1129 0.980 223	1.000 5.193 118 1102 0.981 116	1.000 5.193 29 900 0.966 28	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h	1.000 5.193 227 1129 0.980 223 1107	1.000 5.193 118 1102 0.981 116 1081	1.000 5.193 29 900 0.966 28 869	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	1.000 5.193 227 1129 0.980 223 1107 0.201	1.000 5.193 118 1102 0.981 116 1081 0.107	1.000 5.193 29 900 0.966 28 869 0.032	
RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh	1.000 5.193 227 1129 0.980 223 1107 0.201 5.1	1.000 5.193 118 1102 0.981 116 1081 0.107 4.3	1.000 5.193 29 900 0.966 28 869	
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh LOS 95th %tile Queue, veh	1.000 5.193 227 1129 0.980 223 1107 0.201	1.000 5.193 118 1102 0.981 116 1081 0.107	1.000 5.193 29 900 0.966 28 869 0.032	

Lane Configurations 4 7 62 198 116 306 12 Traffic Volume (vph) 54 130 68 122 192 217 123 629 198 116 306 13 Ideal Flow (vph) 1900 170 171 0 171 172 172 9 1456.2 175 22.3 131.1 1822 1140 1160 160.1 <t< th=""><th></th><th>≯</th><th>-</th><th>7</th><th>4</th><th>+</th><th>•</th><th>1</th><th>1</th><th>1</th><th>1</th><th>ţ</th><th>~</th></t<>		≯	-	7	4	+	•	1	1	1	1	ţ	~
Lane Configurations 4 5 5 7 5 7 5 7 7 Traffic Volume (vph) 54 130 68 122 192 217 123 629 198 116 306 12 192 217 123 629 198 116 306 120 1900 1301 1901 101 0 1733 1504 1791 0 1733 1822 FI FI Firmited 0.714 0 1733 1824 Fireit Kirkir Kirkirkirkir Kirkirkir Kirkir Kirkir Kirkirkirkir Kirkir K	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 54 130 68 122 192 217 123 629 198 116 306 306 Future Volume (vph) 54 130 68 122 192 217 123 629 198 116 301 301 306 301													
Future Volume (vph) 54 130 68 122 192 217 123 629 198 116 306 130 ideal Flow (vphpl) 1900 190 110 173 30 1504 1791 0 170 1791 0 170 1791 0 1822 Y Sdd. Flow (RTOR) 27 50 27 9 111 150 150 150 150 150 150 150 150 150		54		68	122		217			198			34
Ideal Flow (vphp) 1900 1000 100	(,,,,												34
Storage Length (m) 0.0 0.0 0.0 0.0 40.0 0.0 45.0 0.0 Storage Lanes 0 0 0 0 1 0 1 1 1 Taper Length (m) 7.5 7.5 7.5 7.5 7.5 7.5 Stdt, Flow (prot) 0 1726 0 0 1733 0 1504 1791 0 1703 1822 Fit Permitted 0.741 0.834 0.486 0.114 0 1703 1822 Right Turn on Red Yes													1900
Storage Lanes 0 0 0 0 1 0 1 Taper Length (m) 7.5			1000			1000			1000			1000	0.0
Taper Length (m) 7.5 7.5 7.5 7.5 7.5 Satd. Flow (prot) 0 1726 0 0 1733 0 1504 1791 0 1703 1822 Filt Permitted 0.741 0.834 0.486 0.114 1822 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (ROR) 27 50 27 9 1456.2 Link Speed (kh) 70 70 40 40 156.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91<													0.0
Satid. Flow (prot) 0 1726 0 0 1733 0 1504 1791 0 1703 1822 Fit Permitted 0.741 0.834 0.486 0.114 0.101 0.101 0.114 0.114 0.101 0.101 0.101 0.101 0.101 0.114 0.101 0.101 0.114 0.101 0.114 0.101 0.114 0.111 <td></td> <td></td> <td></td> <td>Ū</td> <td></td> <td></td> <td>v</td> <td></td> <td></td> <td>Ū</td> <td></td> <td></td> <td>v</td>				Ū			v			Ū			v
Fit Permitted 0.741 0.834 0.486 0.114 Satd. Flow (perm) 0 1293 0 0 1461 0 770 1791 0 204 1822 Right Turn on Red Yes			1726	0		1733	0		1791	0		1822	0
Satd. Flow (perm) 0 1293 0 0 1461 0 770 1791 0 204 1822 Right Turn on Red Yes		v		Ū	v		v		1701	Ū		IULL	v
Right Tum on Red Yes		0		0	0		0		1701	0		1822	0
Satd. Flow (RTOR) 27 50 27 9 Link Speed (k/h) 70 70 40 40 Link Distance (m) 304.3 341.1 247.9 1456.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91<		U	1200		0	1401		110	1751		204	1022	Yes
Link Speed (k/h) 70 70 40 40 Link Distance (m) 304.3 341.1 247.9 1456.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91 </td <td></td> <td></td> <td>27</td> <td>163</td> <td></td> <td>50</td> <td>163</td> <td></td> <td>27</td> <td>163</td> <td></td> <td>Q</td> <td>163</td>			27	163		50	163		27	163		Q	163
Link Distance (m) 304.3 341.1 247.9 1456.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91 <td></td>													
Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91 <tde< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tde<>													
Peak Hour Factor 0.91													
Heavy Vehicles (%) 0% 2% 14% 4% 2% 2% 20% 3% 0% 6% 3% 0 Shared Lane Traffic (%) U 0 277 0 0 583 0 135 909 0 127 373 Turn Type Perm NA Size Gize Gi		0.01		0.01	0.01		0.01	0.01		0.01	0.01		0.01
Shared Lane Traffic (%) Uane Group Flow (vph) 0 277 0 0 583 0 135 909 0 127 373 Turn Type Perm NA Sign 15.0 15.0 15.0 15.0 15.0 15.0 31.5 31.5 31.5 31.													0.91
Lane Group Flow (vph) 0 277 0 0 583 0 135 909 0 127 373 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 4 8 2 6 6 Detector Phase 4 4 8 2 6 6 Switch Phase 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Initial (s) 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Initial (s) 14.5 34.5 34.5 34.5 42.5 42.5 42.5 42.5 Total Split (s) 34.5 34.5 34.5 55.5 5.5		0%	Ζ70	14%	4 %	Ζ70	Ζ %	20%	3%	0%	0%	3%	0%
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 4 8 2 6 6 Permitted Phases 4 8 2 6 6 Detector Phase 4 4 8 2 2 6 Switch Phase	()	٥	077	0	٥	500	0	105	000	٥	107	272	0
Protected Phases 4 8 2 6 Permitted Phases 4 4 8 2 2 6 Detector Phase 4 4 8 8 2 2 6 6 Switch Phase				0			0			0			0
Permitted Phases 4 8 2 6 Detector Phase 4 4 8 8 2 2 6 6 Switch Phase 15.0 15.0 15.0 30.0 <		Perm			Perm			Perm			Perm		
Detector Phase 4 4 8 8 2 2 6 6 Switch Phase			4		•	8		•	2		•	6	
Switch Phase Minimum Initial (s) 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 57.5 55.5 5.5						•			<u> </u>			<u>,</u>	
Minimum Initial (s) 15.0 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 55.5 5.5		4	4		8	8		2	2		6	6	
Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 37.5 37.5 37.5 Total Split (s) 34.5 34.5 34.5 34.5 34.5 42.5 55.2% 55		45.0	45.0		4 = 0	45.0							
Total Split (s)34.534.534.534.542.542.542.542.5Total Split (%)44.8%44.8%44.8%44.8%55.2%55.2%55.2%55.2%Yellow Time (s)5.55.55.55.55.55.55.55.55.5All-Red Time (s)2.02.02.02.02.02.02.02.0Lost Time Adjust (s)0.00.00.00.00.00.00.0Total Lost Time (s)7.57.57.57.57.57.5Lead-Lag Optimize?42.542.542.5Recall ModeNoneNoneNoneMaxMaxMaxMaxAct Effct Green (s)27.027.035.035.035.035.0Actuated g/C Ratio0.350.350.450.450.450.45V/c Ratio0.591.070.391.101.380.45Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2													
Total Split (%)44.8%44.8%44.8%55.2%55.2%55.2%55.2%Yellow Time (s)5.55.55.55.55.55.55.55.5All-Red Time (s)2.02.02.02.02.02.02.0Lost Time Adjust (s)0.00.00.00.00.00.0Total Lost Time (s)7.57.57.57.57.57.5Lead-Lag </td <td></td>													
Yellow Time (s) 5.5 5.5 5.5 5.5 5.5 5.5 5.5 All-Red Time (s) 2.0 </td <td></td>													
All-Red Time (s) 2.0													
Lost Time Adjust (s) 0.0													
Total Lost Time (s) 7.5<		2.0			2.0								
Lead/Lag None None None Max													
Lead-Lag Optimize? Recall Mode None None None Max Max Max Max Act Effct Green (s) 27.0 27.0 35.0			7.5			7.5		7.5	7.5		7.5	7.5	
Recall ModeNoneNoneMaxMaxMaxMaxAct Effct Green (s)27.027.035.035.035.0Actuated g/C Ratio0.350.350.450.450.45v/c Ratio0.591.070.391.101.380.45Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2													
Act Effct Green (s)27.027.035.035.035.0Actuated g/C Ratio0.350.350.450.450.45v/c Ratio0.591.070.391.101.380.45Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2													
Actuated g/C Ratio0.350.350.450.450.45v/c Ratio0.591.070.391.101.380.45Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2		None			None								
v/c Ratio0.591.070.391.101.380.45Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2	()												
Control Delay24.584.618.084.3250.816.2Queue Delay0.00.00.00.00.00.0Total Delay24.584.618.084.3250.816.2	Actuated g/C Ratio												
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 24.5 84.6 18.0 84.3 250.8 16.2													
Total Delay 24.5 84.6 18.0 84.3 250.8 16.2	Control Delay		24.5			84.6		18.0			250.8	16.2	
	Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
	Total Delay		24.5			84.6		18.0			250.8	16.2	
	LOS		С			F		В	F		F	В	
Approach Delay 24.5 84.6 75.8 75.8	Approach Delay		24.5									75.8	
Approach LOS C F E E	Approach LOS		С			F			Е			Е	
Queue Length 50th (m) 30.7 ~95.6 13.1 ~158.8 ~26.2 36.5	•••		30.7			~95.6		13.1	~158.8		~26.2		
Queue Length 95th (m) 56.0 #158.0 27.8 #230.1 #46.4 58.8													
Internal Link Dist (m) 280.3 317.1 223.9 1432.2													
Turn Bay Length (m) 40.0 45.0								40.0			45.0		
Base Capacity (vph) 470 544 350 828 92 833			470			544			828			833	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.59			1.07		0.39	1.10		1.38	0.45	
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 150												
Control Type: Semi Act-Unco	oord											
Maximum v/c Ratio: 1.38												
Intersection Signal Delay: 72	.0			In	Itersectior	LOS: E						
Intersection Capacity Utilizat	ion 130.2%	1		IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
~ Volume exceeds capacity	y, queue is	theoretica	ally infinit	e.								
Queue shown is maximur	n after two	cycles.										
# 95th percentile volume e	xceeds cap	acity, que	eue may	be longer								
Queue shown is maximur	n after two	cycles.										
Splits and Phases: 11: Tra	afalgar Roa	d North &	Wellinat	on Road	22							

Splits and Phases: 11: Tratalgar Road North & Wellington Road 22

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42.5 s	34.5 s	
↓ Ø6	↓ Ø8	
42.5 s	34.5 s	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	23	16	48	34	13	22	71	596	54	22	372	8
Future Volume (Veh/h)	23	16	48	34	13	22	71	596	54	22	372	8
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	24	17	51	36	14	23	75	627	57	23	392	8
Pedestrians		5			3							
Lane Width (m)		3.6			3.6							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1282	1284	401	1310	1260	658	405			687		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1282	1284	401	1310	1260	658	405			687		
tC, single (s)	7.3	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.6	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	78	89	92	64	91	95	93			97		
cM capacity (veh/h)	109	150	651	100	156	466	1138			914		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	73	759	423								
Volume Left	24	36	75	23								
Volume Right	51	23	57	8								
cSH	223	146	1138	914								
Volume to Capacity	0.41	0.50	0.07	0.03								
Queue Length 95th (m)	15.1	19.1	1.7	0.6								
Control Delay (s)	32.0	52.3	1.7	0.8								
Lane LOS	D	F	А	А								
Approach Delay (s)	32.0	52.3	1.7	0.8								
Approach LOS	D	F										
Intersection Summary												
Average Delay			6.2									
Intersection Capacity Utilization	n		72.4%	IC	U Level o	of Service			С			
Analysis Period (min)			15									
· · · /												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	0	0	2	14	0	5	17	616	20	11	369	3
Future Volume (Veh/h)	0	0	2	14	0	5	17	616	20	11	369	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	0	0	2	14	0	5	18	635	21	11	380	3
Pedestrians					1			5				
Lane Width (m)					3.6			3.6				
Walking Speed (m/s)					1.2			1.2				
Percent Blockage					0			0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1090	1096	386	1093	1088	646	383			657		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1090	1096	386	1093	1088	646	383			657		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	92	100	99	98			99		
cM capacity (veh/h)	188	209	663	181	212	474	1187			939		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	2	19	674	394								
Volume Left	0	14	18	11								
Volume Right	2	5	21	3								
cSH	663	216	1187	939								
Volume to Capacity	0.00	0.09	0.02	0.01								
Queue Length 95th (m)	0.0	2.3	0.4	0.3								
Control Delay (s)	10.4	23.2	0.4	0.4								
Lane LOS	B	20.2 C	A	A.								
Approach Delay (s)	10.4	23.2	0.4	0.4								
Approach LOS	B	20.2 C	0.4	U. T								
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utiliza	ation		55.5%	IC		of Service			В			
Analysis Period (min)			15 15	ic.					D			
			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	4	6	53	4	0	0	101	412	10	6	288	9
Future Volume (Veh/h)	4	6	53	4	0	0	101	412	10	6	288	9
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	7	58	4	0	0	110	448	11	7	313	10
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1006	1011	318	1067	1010	454	323			459		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1006	1011	318	1067	1010	454	323			459		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	97	92	98	100	100	91			99		
cM capacity (veh/h)	204	217	723	168	217	611	1237			1113		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	69	4	569	330								
Volume Left	4	4	110	7								
Volume Right	58	0	11	10								
cSH	522	168	1237	1113								
Volume to Capacity	0.13	0.02	0.09	0.01								
Queue Length 95th (m)	3.6	0.6	2.3	0.2								
Control Delay (s)	12.9	27.0	2.4	0.2								
Lane LOS	В	D	А	А								
Approach Delay (s)	12.9	27.0	2.4	0.2								
Approach LOS	В	D										
Intersection Summary												
Average Delay			2.5									
Intersection Capacity Utilizati	ion		57.6%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	¢Î	
Traffic Volume (veh/h)	8	58	98	523	333	12
Future Volume (Veh/h)	8	58	98	523	333	12
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	63	107	568	362	13
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1150	368	375			
vC1, stage 1 conf vol	1100		0.0			
vC2, stage 2 conf vol						
vCu, unblocked vol	1150	368	375			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	91	91			
cM capacity (veh/h)	199	677	1183			
,						
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	72	675	375			
Volume Left	9	107	0			
Volume Right	63	0	13			
cSH	521	1183	1700			
Volume to Capacity	0.14	0.09	0.22			
Queue Length 95th (m)	3.8	2.4	0.0			
Control Delay (s)	13.0	2.3	0.0			
Lane LOS	В	А				
Approach Delay (s)	13.0	2.3	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization	ation		65.2%	IC	CU Level o	f Service
Analysis Period (min)			15			

Intersection Delay, s/veh 4.0 Intersection LOS A Approach EB WB NB Entry Lanes 1 1 1 Conflicting Circle Lanes 1 1 1 Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Cap Adj 1.000 1.000 1.000 Approach LOS A A A Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR 120 2 Cap Entry Lane, veh/h 67 120 2 Cap Entry Lane, veh/h 67	Intersection				
Intersection LOS A Approach EB WB NB Entry Lanes 1 1 1 Conflicting Circle Lanes 1 1 1 Adj Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach LOS A A A Approach LOS R LT LR Assumed Moves TR LT LR Assumed Moves TR 120 2 Cap Entry Lane, veh/h 67 120 2 Cap Entry Lane, veh/h 1103 1057 2 <tr< td=""><td></td><td>4.0</td><td></td><td></td><td></td></tr<>		4.0			
Approach EB WB NB Entry Lanes 1 1 1 Conflicting Circle Lanes 1 1 1 Adj Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Vol Crossing Leg, #/h 0 0 1.000 Approach LOS A A A Lane Left Left Left Lane Loft					
Liny Lanes 1 1 1 Conflicting Circle Lanes 1 1 1 Adj Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach LOS A A A Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Christel Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980		A			
Conflicting Circle Lanes 1 1 1 Adj Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach LOS A A A Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR 120 2 Cap Entry Lane, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1103	Approach	EB	WB	NB	
Adj Approach Flow, veh/h 66 118 2 Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Lane Moves TR LT LR Assumed Moves TR LT LR RT Channelized 2 2 Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry WAdj Factor 0.980 </td <td>Entry Lanes</td> <td>1</td> <td>1</td> <td>1</td> <td></td>	Entry Lanes	1	1	1	
Demand Flow Rate, veh/h 67 120 2 Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR RT Channelized Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry Flow, veh/h 66 118 2 Cap Entry, veh/h 103 1109 1056 V/C Ratio 0.060 0.0166 0.002 3.4 A	Conflicting Circle Lanes	1	1	1	
Vehicles Circulating, veh/h 4 0 67 Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1056 V/C Ratio 0.060 0.106 0.002 <t< td=""><td>Adj Approach Flow, veh/h</td><td>66</td><td>118</td><td></td><td></td></t<>	Adj Approach Flow, veh/h	66	118		
Vehicles Exiting, veh/h 116 69 4 Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR 100 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry Vane, veh/h 66 118 2 Cap Entry, veh/h 103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8	Demand Flow Rate, veh/h	67	120		
Follow-Up Headway, s 3.186 3.186 3.186 Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A <td>Vehicles Circulating, veh/h</td> <td>4</td> <td>•</td> <td>67</td> <td></td>	Vehicles Circulating, veh/h	4	•	67	
Ped Vol Crossing Leg, #/h 0 0 3 Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Archannelized	Vehicles Exiting, veh/h		69	•	
Ped Cap Adj 1.000 1.000 1.000 Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR Annelized U U 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Follow-Up Headway, s	3.186	3.186	3.186	
Approach Delay, s/veh 3.8 4.2 3.4 Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR RT Channelized Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 1056 V/C Ratio 0.060 0.106 0.002 Coap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Ped Vol Crossing Leg, #/h	-	-	-	
Approach LOS A A A Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR RT Channelized L L LR Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Ped Cap Adj	1.000	1.000		
Lane Left Left Left Designated Moves TR LT LR Assumed Moves TR LT LR Assumed Moves TR LT LR RT Channelized	Approach Delay, s/veh	3.8	4.2	3.4	
Designated Moves TR LT LR Assumed Moves TR LT LR RT Channelized	Approach LOS	А	А	А	
Assumed Moves TR LT LR RT Channelized 1.000 1.000 1.000 Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Lane	Left	Left	Left	
RT Channelized Lane Util 1.000 1.000 Critical Headway, s 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A A	Designated Moves	TR	LT	LR	
Lane Util 1.000 1.000 1.000 Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Assumed Moves	TR	LT	LR	
Critical Headway, s 5.193 5.193 5.193 Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	RT Channelized				
Entry Flow, veh/h 67 120 2 Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Lane Util	1.000	1.000	1.000	
Cap Entry Lane, veh/h 1125 1130 1057 Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Critical Headway, s	5.193	5.193	5.193	
Entry HV Adj Factor 0.980 0.981 1.000 Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Entry Flow, veh/h	67	120	2	
Flow Entry, veh/h 66 118 2 Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Cap Entry Lane, veh/h	1125	1130	1057	
Cap Entry, veh/h 1103 1109 1056 V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Entry HV Adj Factor	0.980	0.981	1.000	
V/C Ratio 0.060 0.106 0.002 Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	El a con El cadar o con la lla		110	2	
Control Delay, s/veh 3.8 4.2 3.4 LOS A A A	Flow Entry, ven/n				
LOS A A A	Cap Entry, veh/h	1103	1109	1056	
	Cap Entry, veh/h V/C Ratio	1103 0.060	1109	1056 0.002	
95th %tile Queue, veh 0 0 0	Cap Entry, veh/h V/C Ratio Control Delay, s/veh	1103 0.060	1109 0.106	1056 0.002	
	Cap Entry, veh/h V/C Ratio	1103 0.060 3.8	1109 0.106 4.2	1056 0.002 3.4 A	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		۲	ef 👘		ሻ	eî.	
Traffic Volume (vph)	33	178	124	176	91	76	40	214	100	264	522	33
Future Volume (vph)	33	178	124	176	91	76	40	214	100	264	522	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	40.0		0.0	45.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.5			7.5		·	7.5		•	7.5		
Satd. Flow (prot)	0	1736	0	0	1549	0	1805	1562	0	1492	1676	0
Flt Permitted	•	0.931			0.586	·	0.235		•	0.498		
Satd. Flow (perm)	0	1625	0	0	931	0	446	1562	0	782	1676	0
Right Turn on Red	Ŭ	1020	Yes	Ū	001	Yes		1002	Yes	102	1010	Yes
Satd. Flow (RTOR)		42	100		20	100		40	100		5	100
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0.07	19%	9%	21%	12%	18%
Shared Lane Traffic (%)	570	2 /0	7/0	2070	570	2070	070	1370	570	2170	12/0	1070
Lane Group Flow (vph)	0	386	0	0	394	0	46	361	0	303	638	0
Turn Type	Perm	NA	0	Perm	NA	0	Perm	NA	0	Perm	NA	0
Protected Phases	Feilli	4		Feilli	8		Feilli	2		Feilii	6	
Permitted Phases	4	4		8	0		2	2		6	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	4	4		0	0		2	2		0	0	
	15.0	15.0		15.0	15.0		30.0	30.0		30.0	30.0	
Minimum Initial (s)	22.5	22.5		22.5	22.5		30.0	30.0 37.5		30.0	30.0 37.5	
Minimum Split (s)	22.5 34.5	22.5 34.5		22.5 34.5	22.5 34.5		37.5 42.5	37.5 42.5		37.5 42.5	37.5 42.5	
Total Split (s)								42.5 55.2%			42.5 55.2%	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		55.2%			55.2%		
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5 2.0	5.5		5.5 2.0	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0			2.0			2.0	
Lost Time Adjust (s)		0.0			0.0 7.5		0.0	0.0		0.0 7.5	0.0	
Total Lost Time (s)		7.5			<i>I</i> .5		7.5	7.5		1.5	7.5	
Lead/Lag												
Lead-Lag Optimize?	Nono	None		None	Nono		Мах	Max		Max	Мох	
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)		27.0			27.0		35.0	35.0		35.0	35.0	
Actuated g/C Ratio		0.35			0.35		0.45	0.45		0.45	0.45	_
v/c Ratio		0.65			1.16		0.23	0.49		0.85	0.84	
Control Delay		24.6			126.8		16.5	15.8		44.1	30.3	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		24.6			126.8		16.5	15.8		44.1	30.3	_
LOS Approach Delay		C			F		В	B		D	C	
Approach Delay		24.6			126.8			15.9			34.8	_
Approach LOS		C			F			В		00.0	C	
Queue Length 50th (m)		43.4			~71.1		4.1	32.7		39.6	81.8	
Queue Length 95th (m)		69.6			#118.8		11.1	53.3		#83.2	#136.2	
Internal Link Dist (m)		280.3			317.1			223.9		4- 4	1432.2	
Turn Bay Length (m)							40.0			45.0		
Base Capacity (vph)		597			339		202	731		355	764	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.65			1.16		0.23	0.49		0.85	0.84	
Intersection Summary												
Area Type:	Other											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 75												
Control Type: Semi Act-Une	coord											
Maximum v/c Ratio: 1.16												
Intersection Signal Delay: 4	6.4			In	Itersectior	LOS: D						
Intersection Capacity Utiliza	ation 117.4%			IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
 Volume exceeds capac 	ity, queue is	theoretica	ally infinit	e.								
Queue shown is maximu	um after two	cycles.										
# 95th percentile volume	exceeds cap	acity, que	eue may l	be longer								
Queue shown is maximu	um after two	cycles.										
Splits and Phases: 11: T	rafalgar Roa	d North &	Wellinat	on Road	22							

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42.5 s	34.5 s
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42.5 s	34.5 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	4	1	8	34	0	34	12	292	14	2	683	1
Future Volume (Veh/h)	4	1	8	34	0	34	12	292	14	2	683	1
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	1	9	37	0	37	13	317	15	2	742	1
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1134	1104	744	1108	1098	324	743			332		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1134	1104	744	1108	1098	324	743			332		
tC, single (s)	7.1	6.5	6.4	7.2	6.5	6.3	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.6	4.0	3.4	2.2			2.2		
p0 queue free %	98	100	98	79	100	95	99			100		
cM capacity (veh/h)	170	209	391	176	211	701	873			1239		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	14	74	345	745								
Volume Left	4	37	13	2								
Volume Right	9	37	15	1								
cSH	272	281	873	1239								
Volume to Capacity	0.05	0.26	0.01	0.00								
Queue Length 95th (m)	1.3	8.2	0.4	0.0								
Control Delay (s)	18.9	22.3	0.5	0.0								
Lane LOS	С	С	A	A								
Approach Delay (s)	18.9	22.3	0.5	0.0								
Approach LOS	С	С										
Intersection Summary												
Average Delay			1.8									
Intersection Capacity Utiliza	ation		50.0%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	15	0	19	12	1	29	2	271	5	5	602	4
Future Volume (Veh/h)	15	0	19	12	1	29	2	271	5	5	602	4
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	19	0	23	15	1	36	2	335	6	6	743	5
Pedestrians								4				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1136	1102	750	1126	1102	338	748			341		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1136	1102	750	1126	1102	338	748			341		
tC, single (s)	7.1	6.5	6.2	7.1	7.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.9	3.3	2.2			2.2		
p0 queue free %	89	100	94	91	99	95	100			100		
cM capacity (veh/h)	170	212	413	172	142	709	870			1229		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	42	52	343	754								
Volume Left	19	15	2	6								
Volume Right	23	36	6	5								
cSH	251	358	870	1229								
Volume to Capacity	0.17	0.15	0.00	0.00								
Queue Length 95th (m)	4.7	4.0	0.1	0.1								
Control Delay (s)	22.2	16.7	0.1	0.1								
Lane LOS	С	С	A	A								
Approach Delay (s)	22.2	16.7	0.1	0.1								
Approach LOS	С	С										
Intersection Summary												
Average Delay			1.6									
Intersection Capacity Utiliza	ation		46.6%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			÷			\$			\$	
Traffic Volume (veh/h)	18	0	227	1	39	4	95	216	1	12	257	3
Future Volume (Veh/h)	18	0	227	1	39	4	95	216	1	12	257	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	19	0	234	1	40	4	98	223	1	12	265	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	734	710	266	944	712	224	268			224		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	734	710	266	944	712	224	268			224		
tC, single (s)	7.1	6.5	6.2	8.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	4.4	4.0	3.3	2.2			2.2		
p0 queue free %	93	100	70	99	88	100	92			99		
cM capacity (veh/h)	284	328	772	108	328	821	1296			1357		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	253	45	322	280								
Volume Left	19	1	98	12								
Volume Right	234	4	1	3								
cSH	684	330	1296	1357								
Volume to Capacity	0.37	0.14	0.08	0.01								
Queue Length 95th (m)	13.7	3.7	2.0	0.2								
Control Delay (s)	13.3	17.6	2.9	0.4								
Lane LOS	В	С	А	А								
Approach Delay (s)	13.3	17.6	2.9	0.4								
Approach LOS	В	С										
Intersection Summary												
Average Delay			5.8									
Intersection Capacity Utilization	on		62.8%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			र्स	eî.	
Traffic Volume (veh/h)	11	89	29	286	481	4
Future Volume (Veh/h)	11	89	29	286	481	4
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	97	32	311	523	4
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	900	525	527			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	900	525	527			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	82	97			
cM capacity (veh/h)	300	552	1040			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	109	343	527			
Volume Left	12	32	0			
Volume Right	97	0	4			
cSH	506	1040	1700			
Volume to Capacity	0.22	0.03	0.31			
Queue Length 95th (m)	6.5	0.8	0.0			
Control Delay (s)	14.1	1.1	0.0			
Lane LOS	В	А				
Approach Delay (s)	14.1	1.1	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilizati	on		52.0%	IC	CU Level o	f Service
Analysis Period (min)	•		15			
			10			

Intersection				
Intersection Delay, s/veh	5.5			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	299	143	60	57
Demand Flow Rate, veh/h	305	146	62	58
Vehicles Circulating, veh/h	40	110	327	162
Vehicles Exiting, veh/h	180	277	18	93
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	6.1	4.9	5.3	4.4
Approach LOS	А	А	А	А
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Designated Moves Assumed Moves	LTR LTR	LTR LTR	LTR LTR	LTR LTR
v				
Assumed Moves				
Assumed Moves RT Channelized	LTR	LTR	LTR	LTR
Assumed Moves RT Channelized Lane Util	LTR 1.000	LTR 1.000	LTR 1.000	LTR 1.000
Assumed Moves RT Channelized Lane Util Critical Headway, s	LTR 1.000 5.193	LTR 1.000 5.193	LTR 1.000 5.193	LTR 1.000 5.193
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor	LTR 1.000 5.193 305	LTR 1.000 5.193 146	LTR 1.000 5.193 62	LTR 1.000 5.193 58
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h	LTR 1.000 5.193 305 1086 0.981 299	LTR 1.000 5.193 146 1012 0.983 143	LTR 1.000 5.193 62 815 0.976 60	LTR 1.000 5.193 58 961 0.980 57
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h	LTR 1.000 5.193 305 1086 0.981 299 1065	LTR 1.000 5.193 146 1012 0.983	LTR 1.000 5.193 62 815 0.976	LTR 1.000 5.193 58 961 0.980
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	LTR 1.000 5.193 305 1086 0.981 299 1065 0.281	LTR 1.000 5.193 146 1012 0.983 143	LTR 1.000 5.193 62 815 0.976 60	LTR 1.000 5.193 58 961 0.980 57
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh	LTR 1.000 5.193 305 1086 0.981 299 1065 0.281 6.1	LTR 1.000 5.193 146 1012 0.983 143 995 0.144 4.9	LTR 1.000 5.193 62 815 0.976 60 795	LTR 1.000 5.193 58 961 0.980 57 942
Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	LTR 1.000 5.193 305 1086 0.981 299 1065 0.281	LTR 1.000 5.193 146 1012 0.983 143 995 0.144	LTR 1.000 5.193 62 815 0.976 60 795 0.076	LTR 1.000 5.193 58 961 0.980 57 942 0.060

Lane Configurations Image: Configuration in the image: Configuratine in the image: Configuration in the image: Configuration in th		٦	-	7	4	+	×	1	1	1	1	ţ	~
Lane Configurations Image of the second	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 60 137 69 128 200 247 127 688 207 133 335 33 Future Volume (vph) 600 137 69 128 200 247 127 688 207 133 335 33 Storage Length (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1900 <td></td>													
Future Volume (vph) 60 137 69 128 200 247 127 688 207 133 33 35 13 ideal Flow (vphpl) 1900 1200 0 0 0 1 0 1 131 132 1 132 131 1 1450 0 713 1792 0 103 103 103 131 1 1 131 1 1 136 1 1 1 1 1 1 1 1 1 1 <td></td> <td>60</td> <td></td> <td>69</td> <td>128</td> <td></td> <td>247</td> <td></td> <td></td> <td>207</td> <td></td> <td></td> <td>37</td>		60		69	128		247			207			37
Ideal Flow (vphp) 1900 100 1 0 1 <th< td=""><td>· · · /</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>37</td></th<>	· · · /												37
Storage Length (m) 0.0 0.0 0.0 0.0 0.0 40.0 0.0 45.0 0.0 Storage Lanes 0 0 0 1 0 1 0 1 Taper Length (m) 7.5 7.6 7.6 2.01 1822 11.6 12.6 11.6 12.6 11.6 12.7 1435.2 11.6 12.7 1435.2 13.1 1 12.7 14.65.2 13.1 1.6 1.6 1.7 1.5 2.2.3 13.1.1 1 14.6 2.9 14.6 1.9 1.9 1.9 1.9 1.9 1.9 1.9 1.9													1900
Storage Lanes 0 0 0 1 0 1 Taper Length (m) 7.5 <t< td=""><td></td><td></td><td>1000</td><td></td><td></td><td>1000</td><td></td><td></td><td>1000</td><td></td><td></td><td>1000</td><td>0.0</td></t<>			1000			1000			1000			1000	0.0
Taper Length (m) 7.5 7.5 7.5 7.5 Satd. Flow (prot) 0 1733 0 0 1728 0 1504 1792 0 1703 1822 Fit Permitted 0.697 0.330 0.450 0.114 0 1792 0 204 1822 Right Turn on Red Yes Ye													0.0
Said. Flow (prot) 0 1733 0 0 1728 0 1504 1792 0 1703 1822 Fit Permitted 0.697 0.830 0.450 0.114 0 1713 1792 0 204 1822 Right Turn on Red Yes				Ū			v			v			v
Fit Permitted 0.697 0.830 0.450 0.114 Satd. Flow (perm) 0 1221 0 0 1450 0 713 1792 0 204 1822 Right Tum on Red Yes			1733	0		1728	0		1792	0		1822	0
Satd. Flow (perm) 0 1221 0 0 1450 0 713 1792 0 204 1822 Right Turn on Red Yes		v		Ū	v		Ū		1102	v		TOLL	v
Right Tum on Red Yes		0		0	0		0		1792	0		1822	0
Satid. Flow (RTOR) 25 54 26 10 Link Speed (k/h) 70 70 40 40 40 Link Distance (m) 304.3 341.1 247.9 1456.2 131.1 Peak Hour Factor 0.91		U	1221		0	1400		710	1152		204	1022	Yes
Link Speed (k/h) 70 70 40 40 Link Distance (m) 304.3 341.1 247.9 1456.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91 </td <td></td> <td></td> <td>25</td> <td>163</td> <td></td> <td>5/</td> <td>163</td> <td></td> <td>26</td> <td>163</td> <td></td> <td>10</td> <td>163</td>			25	163		5/	163		26	163		10	163
Link Distance (m) 304.3 341.1 247.9 1456.2 Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91 <td></td>													
Travel Time (s) 15.6 17.5 22.3 131.1 Peak Hour Factor 0.91													
Peak Hour Factor 0.91													
Heavy Vehicles (%) 0% 2% 14% 4% 2% 2% 20% 3% 0% 6% 3% 0% Shared Lane Traffic (%) 0 0 632 0 140 983 0 146 409 Lane Group Flow (vph) 0 293 0 0 632 0 140 983 0 146 409 Turn Type Perm NA Sizes Sizes Sizes Sizes Sizes Sizes Sizes Sizes Sizes Sizes<		0.01		0.01	0.01		0.01	0.01		0.01	0.01		0.01
Shared Lane Traffic (%) Lane Group Flow (vph) 0 293 0 0 632 0 140 983 0 146 409 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 4 8 2 6 Detector Phase 4 4 8 2 6 Detector Phase 4 4 8 8 2 6 Detector Phase 4 4 8 8 2 6 6 Switch Phase													
Lane Group Flow (vph) 0 293 0 0 632 0 140 983 0 146 409 Tum Type Perm NA Su 30.0 30.0 30.0 30.0		0%	Ζ70	14%	4 %	Ζ70	Ζ %	20%	3%	0%	070	3%	0%
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 4 8 2 6 Detector Phase 4 4 8 2 6 Detector Phase 4 4 8 2 2 6 Switch Phase Minimun Initial (s) 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimun Initial (s) 15.0 15.0 15.0 30.0 30.0 30.0 30.0 30.0 Minimun Initial (s) 22.5 22.5 22.5 37.5 37.5 37.5 37.5 Total Split (s) 34.5 34.5 34.5 42.5 42.5 42.5 42.5 Yellow Time (s) 5.5 <td></td> <td>0</td> <td>202</td> <td>0</td> <td>٥</td> <td>620</td> <td>0</td> <td>140</td> <td>002</td> <td>0</td> <td>140</td> <td>400</td> <td>0</td>		0	202	0	٥	620	0	140	002	0	140	400	0
Protected Phases 4 8 2 6 Permitted Phases 4 4 8 2 6 Detector Phase 4 4 8 8 2 2 6 6 Switch Phase				0			0			0			0
Permitted Phases 4 8 2 6 Detector Phase 4 4 8 8 2 2 6 6 Switch Phase		Perm			Perm			Perm			Perm		
Detector Phase 4 4 8 8 2 2 6 6 Switch Phase			4		0	8		•	2		•	6	
Switch Phase Minimum Initial (s) 15.0 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 37.5 37.5 37.5 Total Split (s) 34.5 34.5 34.5 34.5 34.5 42.5 42.5 42.5 42.5 42.5 Total Split (%) 44.8% 44.8% 44.8% 55.2% 55.2% 55.2% 55.2% Yellow Time (s) 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 All-Red Time (s) 2.0			4			•			•			0	
Minimum Initial (s) 15.0 15.0 15.0 15.0 30.0 30.0 30.0 30.0 Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 37.5 37.5 37.5 Total Split (s) 34.5 34.5 34.5 34.5 42.5 42.5 42.5 42.5 Total Split (%) 44.8% 44.8% 44.8% 55.2% 55.2% 55.2% 55.2% Yellow Time (s) 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 All-Red Time (s) 2.0 </td <td></td> <td>4</td> <td>4</td> <td></td> <td>8</td> <td>8</td> <td></td> <td>2</td> <td>2</td> <td></td> <td>6</td> <td>6</td> <td></td>		4	4		8	8		2	2		6	6	
Minimum Split (s) 22.5 22.5 22.5 22.5 37.5 37.5 37.5 37.5 Total Split (s) 34.5 34.5 34.5 34.5 34.5 42.5		15.0	45.0		1 = 0	15.0							
Total Split (s) 34.5 34.5 34.5 34.5 42.5 42.5 42.5 42.5 Total Split (%) 44.8% 44.8% 44.8% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.2% 55.5% 5.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5													
Total Split (%) 44.8% 44.8% 44.8% 55.2% 55.2% 55.2% Yellow Time (s) 5.5 5.5 5.5 5.5 5.5 5.5 5.5 All-Red Time (s) 2.0<													
Yellow Time (s)5.55.55.55.55.55.55.5All-Red Time (s)2.02.02.02.02.02.02.02.0Lost Time Adjust (s)0.00.00.00.00.00.00.0Total Lost Time (s)7.57.57.57.57.57.5Lead/Lag </td <td></td>													
All-Red Time (s) 2.0													
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 7.5 <													
Total Lost Time (s) 7.5		2.0			2.0								
Lead/Lag None None None Max Max <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>													
Lead-Lag Optimize? Recall Mode None None None None Max			7.5			7.5		7.5	7.5		7.5	7.5	
Recall ModeNoneNoneNoneMaxMaxMaxMaxAct Effct Green (s)27.027.035.035.035.035.0Actuated g/C Ratio0.350.350.450.450.450.45v/c Ratio0.661.160.431.191.590.49Control Delay27.7117.319.5119.2333.116.9Queue Delay0.00.00.00.00.00.0Total Delay27.7117.319.5119.2333.116.9LOSCFBFFBApproach Delay27.7117.3106.8100.1													
Act Effct Green (s)27.027.035.035.035.0Actuated g/C Ratio0.350.350.450.450.45v/c Ratio0.661.160.431.191.590.49Control Delay27.7117.319.5119.2333.116.9Queue Delay0.00.00.00.00.00.0Total Delay27.7117.319.5119.2333.116.9LOSCFBFFBApproach Delay27.7117.3106.8100.1													
Actuated g/C Ratio0.350.350.450.450.450.45v/c Ratio0.661.160.431.191.590.49Control Delay27.7117.319.5119.2333.116.9Queue Delay0.00.00.00.00.00.0Total Delay27.7117.319.5119.2333.116.9LOSCFBFFBApproach Delay27.7117.3106.8100.1		None			None								
v/c Ratio 0.66 1.16 0.43 1.19 1.59 0.49 Control Delay 27.7 117.3 19.5 119.2 333.1 16.9 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.7 117.3 19.5 119.2 333.1 16.9 LOS C F B F F B Approach Delay 27.7 117.3 106.8 100.1	()												
Control Delay 27.7 117.3 19.5 119.2 333.1 16.9 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.7 117.3 19.5 119.2 333.1 16.9 LOS C F B F F B Approach Delay 27.7 117.3 106.8 100.1	Actuated g/C Ratio												
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.7 117.3 19.5 119.2 333.1 16.9 LOS C F B F F B Approach Delay 27.7 117.3 106.8 100.1													
Total Delay 27.7 117.3 19.5 119.2 333.1 16.9 LOS C F B F F B Approach Delay 27.7 117.3 106.8 100.1	Control Delay												
LOS C F B F B Approach Delay 27.7 117.3 106.8 100.1	Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Approach Delay 27.7 117.3 106.8 100.1	Total Delay		27.7			117.3		19.5	119.2		333.1	16.9	
	LOS		С			F		В	F		F	В	
Approach LOS C F F F	Approach Delay												
	Approach LOS		С			F			F			F	
Queue Length 50th (m) 34.1 ~111.8 14.0 ~183.6 ~32.4 41.0	••		34.1			~111.8		14.0	~183.6		~32.4	41.0	
Queue Length 95th (m) 62.2 #175.8 30.0 #256.6 #54.8 65.4								30.0					
Internal Link Dist (m) 280.3 317.1 223.9 1432.2													
Turn Bay Length (m) 40.0 45.0								40.0			45.0		
Base Capacity (vph) 444 543 324 828 92 833			444			543			828			833	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.66			1.16		0.43	1.19		1.59	0.49	
Intersection Summary												
Area Type: O	ther											
Cycle Length: 77												
Actuated Cycle Length: 77												
Natural Cycle: 150												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 1.59												
Intersection Signal Delay: 99.					itersectior							
Intersection Capacity Utilization	on 136.1%	i i		IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
 Volume exceeds capacity 			ally infinit	e.								
Queue shown is maximum												
# 95th percentile volume ex	ceeds cap	acity, que	eue may l	be longer	•							
Queue shown is maximum	after two	cycles.										
Splits and Phases: 11: Trat	algar Roa	d North &	Wellingt	on Road	22							

↑ ø2		A ₀₄	
42.5 s		34.5 s	
Ø6		↓ Ø8	
42.5 s		34.5 s	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	23	16	48	34	13	22	71	671	54	21	417	8
Future Volume (Veh/h)	23	16	48	34	13	22	71	671	54	21	417	8
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	24	17	51	36	14	23	75	706	57	22	439	8
Pedestrians		5			3							
Lane Width (m)		3.6			3.6							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1406	1408	448	1434	1384	738	452			766		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1406	1408	448	1434	1384	738	452			766		
tC, single (s)	7.3	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.6	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	72	87	92	55	89	95	93			97		
cM capacity (veh/h)	87	126	612	79	131	420	1093			854		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	73	838	469								
Volume Left	24	36	75	22								
Volume Right	51	23	57	8								
cSH	186	119	1093	854								
Volume to Capacity	0.49	0.62	0.07	0.03								
Queue Length 95th (m)	19.4	24.9	1.8	0.6								
Control Delay (s)	41.9	75.0	1.7	0.7								
Lane LOS	E	F	А	А								
Approach Delay (s)	41.9	75.0	1.7	0.7								
Approach LOS	E	F										
Intersection Summary												
Average Delay			7.6									
Intersection Capacity Utilization	on		78.3%	IC	U Level o	of Service			D			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	6	0	2	14	0	9	17	695	20	14	416	10
Future Volume (Veh/h)	6	0	2	14	0	9	17	695	20	14	416	10
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	6	0	2	14	0	9	18	716	21	14	429	10
Pedestrians					1			5				
Lane Width (m)					3.6			3.6				
Walking Speed (m/s)					1.2			1.2				
Percent Blockage					0			0				
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1234	1236	439	1232	1230	728	439			738		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1234	1236	439	1232	1230	728	439			738		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.6	4.0	3.3	2.2			2.2		
p0 queue free %	96	100	100	90	100	98	98			98		
cM capacity (veh/h)	148	172	620	145	173	427	1132			877		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	8	23	755	453								
Volume Left	6	14	18	14								
Volume Right	2	9	21	10								
cSH	183	195	1132	877								
Volume to Capacity	0.04	0.12	0.02	0.02								
Queue Length 95th (m)	1.1	3.2	0.4	0.4								
Control Delay (s)	25.6	25.9	0.4	0.5								
Lane LOS	D	D	А	А								
Approach Delay (s)	25.6	25.9	0.4	0.5								
Approach LOS	D	D										
Intersection Summary												
Average Delay			1.1									
Intersection Capacity Utilizati	on		56.4%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			- ↔			- ↔			- ↔	
Traffic Volume (veh/h)	7	6	72	4	14	0	146	476	10	22	331	13
Future Volume (Veh/h)	7	6	72	4	14	0	146	476	10	22	331	13
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	7	78	4	15	0	159	517	11	24	360	14
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1263	1261	367	1337	1262	522	374			528		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1263	1261	367	1337	1262	522	374			528		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	93	95	89	96	90	100	87			98		
cM capacity (veh/h)	119	144	678	99	144	558	1184			1049		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	93	19	687	398								
Volume Left	8	4	159	24								
Volume Right	78	4	11	14								
cSH	403	131	1184	1049								
	403 0.23	0.14	0.13	0.02								
Volume to Capacity	7.0	3.9	3.7	0.02								
Queue Length 95th (m)			3.2									
Control Delay (s)	16.6	37.0		0.7								
Lane LOS	C	E	A	A								
Approach Delay (s)	16.6	37.0	3.2	0.7								
Approach LOS	С	E										
Intersection Summary												
Average Delay			4.0									
Intersection Capacity Utilizat	ion		68.8%	IC	CU Level o	of Service			С			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	Ą	
Traffic Volume (veh/h)	8	58	98	612	395	12
Future Volume (Veh/h)	8	58	98	612	395	12
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	63	107	665	429	13
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1314	436	442			
vC1, stage 1 conf vol		400				
vC2, stage 2 conf vol						
vCu, unblocked vol	1314	436	442			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	U.7	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	94	90	90			
cM capacity (veh/h)	158	621	1118			
,						
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	72	772	442			
Volume Left	9	107	0			
Volume Right	63	0	13			
cSH	454	1118	1700			
Volume to Capacity	0.16	0.10	0.26			
Queue Length 95th (m)	4.5	2.5	0.0			
Control Delay (s)	14.4	2.3	0.0			
Lane LOS	В	А				
Approach Delay (s)	14.4	2.3	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization	ation		73.2%	IC	CU Level o	f Service
Analysis Period (min)			15	ic.		
			15			

Intersection				
Intersection Delay, s/veh	4.9			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	152	197	31	71
Demand Flow Rate, veh/h	155	200	32	72
Vehicles Circulating, veh/h	43	90	156	177
Vehicles Exiting, veh/h	206	98	42	113
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	4.7	5.4	4.1	4.6
Approach LOS	А	А	А	А
Lane	Left	Left	Left	Left
Lane Designated Moves	Left LTR	Left LTR	Left LTR	Left LTR
Designated Moves	LTR	LTR	LTR	LTR
Designated Moves Assumed Moves	LTR	LTR	LTR	LTR
Designated Moves Assumed Moves RT Channelized	LTR LTR	LTR LTR	LTR LTR	LTR LTR
Designated Moves Assumed Moves RT Channelized Lane Util	LTR LTR 1.000	LTR LTR 1.000	LTR LTR 1.000	LTR LTR 1.000
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s	LTR LTR 1.000 5.193	LTR LTR 1.000 5.193	LTR LTR 1.000 5.193	LTR LTR 1.000 5.193
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h	LTR LTR 1.000 5.193 155	LTR LTR 1.000 5.193 200	LTR LTR 1.000 5.193 32	LTR LTR 1.000 5.193 72
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	LTR LTR 1.000 5.193 155 1082 0.983 152	LTR LTR 1.000 5.193 200 1033 0.985 197	LTR LTR 1.000 5.193 32 967 0.983 31	LTR LTR 1.000 5.193 72 947 0.982 71
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h	LTR LTR 1.000 5.193 155 1082 0.983 152 1064	LTR LTR 1.000 5.193 200 1033 0.985 197 1017	LTR LTR 1.000 5.193 32 967 0.983 31 951	LTR LTR 1.000 5.193 72 947 0.982 71 930
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	LTR LTR 1.000 5.193 155 1082 0.983 152 1064 0.143	LTR LTR 1.000 5.193 200 1033 0.985 197 1017 0.194	LTR LTR 1.000 5.193 32 967 0.983 31 951 0.033	LTR LTR 1.000 5.193 72 947 0.982 71 930 0.076
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh	LTR LTR 1.000 5.193 155 1082 0.983 152 1064 0.143 4.7	LTR LTR 1.000 5.193 200 1033 0.985 197 1017 0.194 5.4	LTR LTR 1.000 5.193 32 967 0.983 31 951 0.033 4.1	LTR LTR 1.000 5.193 72 947 0.982 71 930 0.076 4.6
Designated Moves Assumed Moves RT Channelized Lane Util Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	LTR LTR 1.000 5.193 155 1082 0.983 152 1064 0.143	LTR LTR 1.000 5.193 200 1033 0.985 197 1017 0.194	LTR LTR 1.000 5.193 32 967 0.983 31 951 0.033	LTR LTR 1.000 5.193 72 947 0.982 71 930 0.076

HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North & Wellington Road 22

2026 Future Total Traffic - AM - with Improvements

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	\$		7	\$		۲	el el		<u> </u>	ef 🗧	
Traffic Volume (vph)	30	170	120	169	86	66	39	195	94	237	482	30
Future Volume (vph)	30	170	120	169	86	66	39	195	94	237	482	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	35.0		0.0	45.0		0.0	40.0		0.0	65.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1573	1646	0	1429	1504	0	1805	1561	0	1492	1676	0
Flt Permitted	0.630	0.996		0.278	0.846		0.286			0.550		
Satd. Flow (perm)	1043	1639	0	418	1279	0	543	1561	0	864	1676	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		49			53			47			6	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0%	19%	9%	21%	12%	18%
Shared Lane Traffic (%)	10%			10%								
Lane Group Flow (vph)	31	336	0	175	194	0	45	332	0	272	588	0
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		3	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		5.0	12.0		8.0	8.0		8.0	8.0	
Minimum Split (s)	19.5	19.5		8.0	22.5		22.7	22.7		22.7	22.7	
Total Split (s)	21.0	21.0		8.0	29.0		36.0	36.0		36.0	36.0	
Total Split (%)	32.3%	32.3%		12.3%	44.6%		55.4%	55.4%		55.4%	55.4%	
Yellow Time (s)	5.5	5.5		3.0	5.5		5.5	5.5		5.5	5.5	
All-Red Time (s)	2.0	2.0		0.0	2.0		2.3	2.3		2.3	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.5	7.5		3.0	7.5		7.8	7.8		7.8	7.8	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None		Max	Max		Max	Max	
Act Effct Green (s)	13.4	13.4		25.9	21.4		28.2	28.2		28.2	28.2	
Actuated g/C Ratio	0.21	0.21		0.40	0.33		0.43	0.43		0.43	0.43	
v/c Ratio	0.14	0.89		0.72	0.42		0.19	0.47		0.73	0.80	
Control Delay	23.1	50.1		34.1	15.5		14.0	13.8		29.5	26.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	23.1	50.1		34.1	15.5		14.0	13.8		29.5	26.7	
LOS	С	D		С	В		В	В		С	С	
Approach Delay		47.8			24.3			13.8			27.6	
Approach LOS		D			С			В			С	
Queue Length 50th (m)	3.3	37.5		15.3	13.6		3.4	24.0		27.2	61.3	
Queue Length 95th (m)	10.0	#80.0		#36.2	28.5		9.5	42.4		#60.9	#109.7	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)	35.0			45.0			40.0			65.0		
Base Capacity (vph)	216	379		244	460		235	704		375	731	

CANDEVCON LIMITED

HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North & Wellington Road 22

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.14	0.89		0.72	0.42		0.19	0.47		0.73	0.80	
Intersection Summary												
Area Type:	Other											
Cycle Length: 65												
Actuated Cycle Length: 64	.9											
Natural Cycle: 65												
Control Type: Semi Act-Ur	ncoord											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay:	28.1			In	tersectior	LOS: C						
Intersection Capacity Utiliz	IC	U Level o	of Service	D								
Analysis Period (min) 15												
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximum after two cycles.												

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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36 s	8 s	21 s
Ø6	Ø8	
36 s	29 s	

HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - AM - with Improvements 3: Trafalgar Road North & Street 'A'/Howe Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	4î		ሻ	4	
Traffic Volume (veh/h)	14	0	195	1	27	4	79	177	1	4	224	2
Future Volume (Veh/h)	14	0	195	1	27	4	79	177	1	4	224	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	14	0	201	1	28	4	81	182	1	4	231	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	602	585	232	784	586	182	233			183		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	602	585	232	784	586	182	233			183		
tC, single (s)	7.1	6.5	6.2	8.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	0.2	0.1	0.0	0.2						
tF (s)	3.5	4.0	3.3	4.4	4.0	3.3	2.2			2.2		
p0 queue free %	96	100	75	99	93	100	94			100		
cM capacity (veh/h)	369	396	807	156	396	865	1335			1404		
,							1000					
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	215	33	81	183	4	233						_
Volume Left	14	1	81	0	4	0						
Volume Right	201	4	0	1	0	2						
cSH	749	404	1335	1700	1404	1700						
Volume to Capacity	0.29	0.08	0.06	0.11	0.00	0.14						_
Queue Length 95th (m)	9.5	2.1	1.5	0.0	0.1	0.0						
Control Delay (s)	11.7	14.7	7.9	0.0	7.6	0.0						_
Lane LOS	В	В	A		A							
Approach Delay (s)	11.7	14.7	2.4		0.1							
Approach LOS	В	В										
Intersection Summary												
Average Delay			4.9									
Intersection Capacity Utilizat	tion		45.4%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - AM - with Improvements 14: Trafalgar Road North & Street 'E'

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y		1	•	el el		
Traffic Volume (veh/h)	11	89	29	253	416	4	
Future Volume (Veh/h)	11	89	29	253	416	4	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	12	97	32	275	452	4	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	793	454	456				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	793	454	456				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	97	84	97				
cM capacity (veh/h)	347	606	1105				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	109	32	275	456			
Volume Left	103	32	0	430			
Volume Right	97	0	0	4			
cSH	560	1105	1700	1700			
Volume to Capacity	0.19	0.03	0.16	0.27			
Queue Length 95th (m)	5.7	0.03	0.10	0.27			
	5.7 13.0	8.4	0.0	0.0			
Control Delay (s)			0.0	0.0			
Lane LOS	B 13.0	A 0.9		0.0			
Approach Delay (s)	13.0 B	0.9		0.0			
Approach LOS	В						
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Utilizat	tion		36.9%	IC	CU Level o	f Service	
Analysis Period (min)			15				

HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North & Wellington Road 22

2026 Future Total Traffic - PM - with Improvements

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	el el		1	et		ľ	ę		ľ	el el	
Traffic Volume (vph)	54	130	68	122	192	217	123	629	198	116	306	34
Future Volume (vph)	54	130	68	122	192	217	123	629	198	116	306	34
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	35.0		0.0	45.0		0.0	40.0		0.0	65.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1805	1697	0	1736	1714	0	1504	1791	0	1703	1822	0
Flt Permitted	0.186			0.566			0.540			0.083		
Satd. Flow (perm)	353	1697	0	1034	1714	0	855	1791	0	149	1822	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		28			59			25			11	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	2%	14%	4%	2%	2%	20%	3%	0%	6%	3%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	59	218	0	134	449	0	135	909	0	127	373	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		12.0	12.0		8.0	8.0		5.0	8.0	
Minimum Split (s)	19.5	19.5		19.5	19.5		22.7	22.7		8.0	22.7	
Total Split (s)	29.0	29.0		29.0	29.0		53.0	53.0		8.0	61.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		58.9%	58.9%		8.9%	67.8%	
Yellow Time (s)	5.5	5.5		5.5	5.5		5.5	5.5		3.0	5.5	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.3	2.3		0.0	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.5	7.5		7.5	7.5		7.8	7.8		3.0	7.8	
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		Max	Max		None	Max	
Act Effct Green (s)	21.5	21.5		21.5	21.5		45.2	45.2		58.0	53.2	
Actuated g/C Ratio	0.24	0.24		0.24	0.24		0.50	0.50		0.64	0.59	
v/c Ratio	0.70	0.51		0.54	0.99		0.31	1.00		0.70	0.35	
Control Delay	75.4	30.7		39.4	71.3		15.8	52.8		32.0	10.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	75.4	30.7		39.4	71.3		15.8	52.8		32.0	10.2	
LOS	E	С		D	E		В	D		С	В	
Approach Delay		40.2			64.0			48.0			15.8	
Approach LOS		D			E			D			В	
Queue Length 50th (m)	9.8	29.8		21.4	72.4		13.8	154.4		7.9	31.0	
Queue Length 95th (m)	#31.7	52.7		40.8	#135.7		27.1	#245.9		#21.7	48.1	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)	35.0			45.0			40.0			65.0		
Base Capacity (vph)	84	426		247	454		429	911		182	1081	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.70	0.51		0.54	0.99		0.31	1.00		0.70	0.35	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Natural Cycle: 90												
Control Type: Semi Act-Un	ncoord											
Maximum v/c Ratio: 1.00												
Intersection Signal Delay:	44.3			In	tersectior	LOS: D						
Intersection Capacity Utiliz	ation 107.3%)		IC	U Level o	of Service	G					
Analysis Period (min) 15												
# 95th percentile volume	# 95th percentile volume exceeds capacity, queue may be longer.											
Queue shown is maxim												

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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8 s	53 s	29 s
Ø6		₩ Ø8
61s		29 s

HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - PM - with Improvements 3: Trafalgar Road North & Street 'A'/Howe Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	4		ሻ	eî 👘	
Traffic Volume (veh/h)	4	6	53	4	0	0	101	412	10	6	288	9
Future Volume (Veh/h)	4	6	53	4	0	0	101	412	10	6	288	9
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	7	58	4	0	0	110	448	11	7	313	10
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1000	1011	318	1062	1010	454	323			459		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1000	1011	318	1062	1010	454	323			459		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	97	92	98	100	100	91			99		
cM capacity (veh/h)	206	217	723	169	217	611	1237			1113		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	69	4	110	459	7	323						
Volume Left	4	4	110	0	7	0						
Volume Right	58	0	0	11	0	10						
cSH	523	169	1237	1700	1113	1700						
Volume to Capacity	0.13	0.02	0.09	0.27	0.01	0.19						
Queue Length 95th (m)	3.6	0.6	2.3	0.0	0.2	0.0						
Control Delay (s)	12.9	26.8	8.2	0.0	8.3	0.0						
Lane LOS	В	D	Α		A							
Approach Delay (s)	12.9	26.8	1.6		0.2							
Approach LOS	В	D										
Intersection Summary												
Average Delay			2.0									
Intersection Capacity Utilizat	ion		39.3%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - PM - with Improvements 14: Trafalgar Road North & Street 'E'

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y.			↑	ef 👘	
Traffic Volume (veh/h)	8	58	98	523	333	12
Future Volume (Veh/h)	8	58	98	523	333	12
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	63	107	568	362	13
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1150	368	375			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1150	368	375			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	91	91			
cM capacity (veh/h)	199	677	1183			
,						
Direction, Lane #	EB 1	NB 1	NB 2	SB 1		
Volume Total	72	107	568	375		
Volume Left	9	107	0	0		
Volume Right	63	0	0	13		
cSH	521	1183	1700	1700		
Volume to Capacity	0.14	0.09	0.33	0.22		
Queue Length 95th (m)	3.8	2.4	0.0	0.0		
Control Delay (s)	13.0	8.3	0.0	0.0		
Lane LOS	В	А				
Approach Delay (s)	13.0	1.3		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utiliza	ation		38.2%	IC	U Level o	f Service
Analysis Period (min)			15			

2031 Future Total Traffic - AM - with Improvements

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲ ۲	ę.		ľ	•	1	1	†	1	5	ę	
Traffic Volume (vph)	33	178	124	176	91	76	40	214	100	264	522	33
Future Volume (vph)	33	178	124	176	91	76	40	214	100	264	522	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	35.0		0.0	45.0		35.0	40.0		20.0	70.0		0.0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1656	1733	0	1504	1810	1346	1805	1597	1482	1492	1676	0
Flt Permitted	0.689			0.291			0.244			0.606		
Satd. Flow (perm)	1201	1733	0	461	1810	1346	464	1597	1482	952	1676	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		46				87			120		6	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	9%	2%	4%	20%	5%	20%	0%	19%	9%	21%	12%	18%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	38	348	0	202	105	87	46	246	115	303	638	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		5.0	12.0	12.0	8.0	8.0	8.0	8.0	8.0	
Minimum Split (s)	22.6	22.6		8.0	22.6	22.6	25.9	25.9	25.9	25.9	25.9	
Total Split (s)	23.0	23.0		8.0	31.0	31.0	39.0	39.0	39.0	39.0	39.0	
Total Split (%)	32.9%	32.9%		11.4%	44.3%	44.3%	55.7%	55.7%	55.7%	55.7%	55.7%	
Yellow Time (s)	5.5	5.5		3.0	5.5	5.5	5.5	5.5	5.5	5.5	5.5	
All-Red Time (s)	2.2	2.2		0.0	2.2	2.2	2.6	2.6	2.6	2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.7	7.7		3.0	7.7	7.7	8.1	8.1	8.1	8.1	8.1	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None	None	Max	Max	Max	Max	Max	
Act Effct Green (s)	14.8	14.8		27.5	22.8	22.8	30.9	30.9	30.9	30.9	30.9	
Actuated g/C Ratio	0.21	0.21		0.40	0.33	0.33	0.44	0.44	0.44	0.44	0.44	
v/c Ratio	0.15	0.86		0.79	0.18	0.17	0.22	0.35	0.16	0.72	0.85	
Control Delay	23.9	45.6		40.6	17.6	5.4	15.7	14.6	3.1	28.0	31.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	23.9	45.6		40.6	17.6	5.4	15.7	14.6	3.1	28.0	31.0	
LOS	С	D		D	В	А	В	В	А	С	С	
Approach Delay		43.4			26.7			11.5			30.0	
Approach LOS		D			С			В			С	
Queue Length 50th (m)	4.2	40.2		18.7	10.1	0.0	3.8	21.4	0.0	32.7	75.1	
Queue Length 95th (m)	11.4	#79.2		#43.5	20.0	8.1	10.6	36.1	7.1	#68.1	#130.0	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)	35.0			45.0		35.0	40.0		20.0	70.0		
Base Capacity (vph)	264	417		257	606	509	206	709	725	423	748	

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Synchro 9 Report

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.83		0.79	0.17	0.17	0.22	0.35	0.16	0.72	0.85	
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 69	.5											
Natural Cycle: 70												
Control Type: Semi Act-Un	ncoord											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay:	28.3			In	tersectior	LOS: C						
Intersection Capacity Utilization 86.1% ICU Level of Service E												
Analysis Period (min) 15												
# 95th percentile volume	exceeds cap	acity, que	eue may l	be longer								
Queue shown is maxim	um after two	cycles.										

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

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39 s	8 s	23 s
Ø6	₹ <mark>ø</mark> 8	
39 s	31 s	

HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - AM - with Improvements 3: Trafalgar Road North & Street 'A'/Howe Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		<u>٦</u>	1 2		<u>۲</u>	1 2	
Traffic Volume (veh/h)	18	0	227	1	39	4	95	216	1	12	257	3
Future Volume (Veh/h)	18	0	227	1	39	4	95	216	1	12	257	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	19	0	234	1	40	4	98	223	1	12	265	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	734	710	266	942	712	224	268			224		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	734	710	266	942	712	224	268			224		
tC, single (s)	7.1	6.5	6.2	8.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	•	•	0.0	•.=						
tF (s)	3.5	4.0	3.3	4.4	4.0	3.3	2.2			2.2		
p0 queue free %	93	100	70	99	88	100	92			99		
cM capacity (veh/h)	284	328	772	108	328	821	1296			1357		
,						SB 2	1200			1001		
Direction, Lane # Volume Total	EB 1 253	WB 1	NB 1	NB 2	SB 1 12	268						
	253 19	45	98 98	224	12							
Volume Left		1		0		0						
Volume Right	234	4	0	1	0	3						
cSH	684	331	1296	1700	1357	1700						
Volume to Capacity	0.37	0.14	0.08	0.13	0.01	0.16						
Queue Length 95th (m)	13.7	3.7	2.0	0.0	0.2	0.0						
Control Delay (s)	13.3	17.6	8.0	0.0	7.7	0.0						
Lane LOS	В	C	A		A							
Approach Delay (s)	13.3	17.6	2.4		0.3							
Approach LOS	В	С										
Intersection Summary												
Average Delay			5.6									
Intersection Capacity Utilizat	ion		50.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - AM - with Improvements 14: Trafalgar Road North & Street 'E'

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		٦	↑	f,	•=
Traffic Volume (veh/h)	11	89	29	286	481	4
Future Volume (Veh/h)	11	89	29	286	481	4
Sign Control	Stop	00	20	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	97	32	311	523	4
Pedestrians	12	51	02	011	020	т
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NULLE	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	900	525	527			
vC1, stage 1 conf vol	900	J25	521			
vC2, stage 2 conf vol						
vCu, unblocked vol	900	525	527			
tC, single (s)	900 6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	4.1			
	3.5	3.3	2.2			
tF (s)	3.5 96	3.3 82	2.2 97			
p0 queue free %	300	552	1040			
cM capacity (veh/h)						
Direction, Lane #	EB 1	NB 1	NB 2	SB 1		
Volume Total	109	32	311	527		
Volume Left	12	32	0	0		
Volume Right	97	0	0	4		
cSH	506	1040	1700	1700		
Volume to Capacity	0.22	0.03	0.18	0.31		
Queue Length 95th (m)	6.5	0.8	0.0	0.0		
Control Delay (s)	14.1	8.6	0.0	0.0		
Lane LOS	В	А				
Approach Delay (s)	14.1	0.8		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			1.8			
Intersection Capacity Utilizat	tion		38.3%	IC	U Level o	f Service
Analysis Period (min)			15			
			10			

2031 Future Total Traffic - PM - with Improvements

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲.	eî 🗧		5	†	1	1	†	*	ሻ	eî 👘	
Traffic Volume (vph)	60	137	69	128	200	247	127	688	207	133	335	37
Future Volume (vph)	60	137	69	128	200	247	127	688	207	133	335	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	35.0		0.0	45.0		35.0	40.0		20.0	70.0		0.0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1805	1703	0	1736	1863	1583	1504	1845	1615	1703	1822	0
Flt Permitted	0.621			0.617			0.512			0.208		
Satd. Flow (perm)	1180	1703	0	1127	1863	1583	811	1845	1615	373	1822	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		40				142			123		13	
Link Speed (k/h)		70			70			40			40	
Link Distance (m)		304.3			341.1			247.9			1456.2	
Travel Time (s)		15.6			17.5			22.3			131.1	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	2%	14%	4%	2%	2%	20%	3%	0%	6%	3%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	66	227	0	141	220	271	140	756	227	146	409	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		12.0	12.0	12.0	8.0	8.0	8.0	8.0	8.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	25.9	25.9	25.9	25.9	25.9	
Total Split (s)	22.6	22.6		22.6	22.6	22.6	37.4	37.4	37.4	37.4	37.4	
Total Split (%)	37.7%	37.7%		37.7%	37.7%	37.7%	62.3%	62.3%	62.3%	62.3%	62.3%	
Yellow Time (s)	5.5	5.5		5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	
All-Red Time (s)	2.2	2.2		2.2	2.2	2.2	2.6	2.6	2.6	2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.7	7.7		7.7	7.7	7.7	8.1	8.1	8.1	8.1	8.1	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None	None	Max	Max	Max	Max	Max	
Act Effct Green (s)	13.6	13.6		13.6	13.6	13.6	29.3	29.3	29.3	29.3	29.3	
Actuated g/C Ratio	0.23	0.23		0.23	0.23	0.23	0.50	0.50	0.50	0.50	0.50	
v/c Ratio	0.24	0.54		0.54	0.51	0.57	0.35	0.82	0.26	0.78	0.45	
Control Delay	20.9	21.3		28.5	24.3	15.0	12.4	22.8	5.1	47.3	11.4	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	20.9	21.3		28.5	24.3	15.0	12.4	22.8	5.1	47.3	11.4	
LOS	С	С		С	С	В	В	С	А	D	В	
Approach Delay		21.2			21.2			17.9			20.8	
Approach LOS		С			С			В			С	
Queue Length 50th (m)	6.1	18.3		14.0	21.7	12.2	9.1	68.6	6.0	12.9	27.2	
Queue Length 95th (m)	15.3	36.9		29.9	39.9	32.3	21.3	#135.6	16.5	#44.6	47.8	
Internal Link Dist (m)		280.3			317.1			223.9			1432.2	
Turn Bay Length (m)	35.0			45.0		35.0	40.0		20.0	70.0		
Base Capacity (vph)	299	461		286	473	507	405	920	867	186	916	

CANDEVCON LIMITED

Synchro 9 Report

2031 Future Total Traffic - PM - with Improvements

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.49		0.49	0.47	0.53	0.35	0.82	0.26	0.78	0.45	
Intersection Summary												
Area Type:	Other											
Cycle Length: 60												
Actuated Cycle Length: 58	8.7											
Natural Cycle: 60												
Control Type: Semi Act-Ur	ncoord											
Maximum v/c Ratio: 0.82												
Intersection Signal Delay:	19.7			In	tersectior	LOS: B						
Intersection Capacity Utiliz	zation 91.3%			IC	U Level o	of Service	F					
Analysis Period (min) 15												
# 95th percentile volume	exceeds cap	acity, que	eue may l	be longer								
Queue shown is maxim	num after two	cycles.										

Splits and Phases: 11: Trafalgar Road North & Wellington Road 22

√1 ø2	<u></u> Ø4	
37.4 s	22.6 s	
↓ Ø6	4 ▼ Ø8	
37.4s	22.6 s	

HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - PM - with Improvements 3: Trafalgar Road North & Street 'A'/Howe Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	4		ሻ	ef 👘	
Traffic Volume (veh/h)	7	6	72	4	14	0	146	476	10	22	331	13
Future Volume (Veh/h)	7	6	72	4	14	0	146	476	10	22	331	13
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	7	78	4	15	0	159	517	11	24	360	14
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1258	1261	367	1330	1262	522	374			528		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1258	1261	367	1330	1262	522	374			528		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	93	95	89	96	90	100	87			98		
cM capacity (veh/h)	120	144	678	100	144	558	1184			1049		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	93	19	159	528	24	374						
Volume Left	8	4	159	0	24	0						
Volume Right	78	0	0	11	0	14						
cSH	404	132	1184	1700	1049	1700						
Volume to Capacity	0.23	0.14	0.13	0.31	0.02	0.22						
Queue Length 95th (m)	7.0	3.9	3.7	0.0	0.6	0.0						
Control Delay (s)	16.6	36.9	8.5	0.0	8.5	0.0						
Lane LOS	С	E	A		A							
Approach Delay (s)	16.6	36.9	2.0		0.5							
Approach LOS	С	Е										
Intersection Summary												
Average Delay			3.2									
Intersection Capacity Utilization	tion		44.6%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - PM - with Improvements 14: Trafalgar Road North & Street 'E'

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			ND			000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	. Y		<u></u>	†	4	10
Traffic Volume (veh/h)	8	58	98	612	395	12
Future Volume (Veh/h)	8	58	98	612	395	12
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	63	107	665	429	13
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1314	436	442			
vC1, stage 1 conf vol		100	112			
vC2, stage 2 conf vol						
vCu, unblocked vol	1314	436	442			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	94	3.3 90	2.2			
	94 158	90 621	90 1118			
cM capacity (veh/h)						
Direction, Lane #	EB 1	NB 1	NB 2	SB 1		
Volume Total	72	107	665	442		
Volume Left	9	107	0	0		
Volume Right	63	0	0	13		
cSH	454	1118	1700	1700		
Volume to Capacity	0.16	0.10	0.39	0.26		
Queue Length 95th (m)	4.5	2.5	0.0	0.0		
Control Delay (s)	14.4	8.6	0.0	0.0		
Lane LOS	В	А				
Approach Delay (s)	14.4	1.2		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utiliza	ation		42.9%	10	CU Level c	fSoniac
				IC		I SELVICE
Analysis Period (min)			15			

APPENDIX F

TRANSPORTATION TOMORROW SURVEY DATABASE QUERY

Tue Nov 09 2021 13:34:54 GMT-0500 (Eastern Standard Time) - Run Time: 2485ms

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig Column: 2006 GTA zone of destination - gta06_dest

Filters: (2006 GTA zone of origin - gta06_orig In 8370 and Start time of trip - start_time In 700-859 and Primary travel mode of trip - mode_prime In D and Trip purpose of origin - purp_orig In H

Trip 2016 Table:								
Location	Toronto	Brampton	Brampton	Brampton	Halton	Internal	Erin Village	
	299	3332	3375	3462	4183	8370	8380 Total	
8370) 11	22	8	82	57	41	28	249
Percentage	4%	9%	3%	33%	23%	16%	11%	

Wed Nov 10 2021 13:45:46 GMT-0500 (Eastern Standard Time) - Run Time: 2788ms

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig Column: 2006 GTA zone of destination - gta06_dest

Filters: (2006 GTA zone of origin - gta06_orig In 8370, 8371, 8373 and Start time of trip - start_time In 700-859 and Primary travel mode of trip - mode_prime In D and Trip purpose of origin - purp_orig In H)

Trip 2016

Table:

	Toronto	Toronto	Markham	Brampton	Brampton	Brampton	Brampton	Brampton	Halton	Halton	Guelph	Wellington	nternal	Erin Village D	Dufferin	External	
	299	309	2393	3332	3343	3375	3462	3721	4160	4183	8024	8365	8370	8380	8417	9057	
8370) 11	0	0	22	0	8	82	0	0	57	0	0	41	28	0	0	
8371	. 0	12	. 0	0	0	0	0	12	0	0	0	0	0	0	0	0	
8373	8 0	0	8	0	46	0	0	0	8	0	30	17	0	0	17	17	
	11	12	. 8	22	46	8	82	12	8	57	30	17	41	28	17	17	399
	3%	3%	2%	6%	12%	2%	21%	3%	2%	14%	8%	4%	10%	7%	4%	4%	
Trip Assignment	1	1	. 1	1	1	1	. 1	. 1	1	1	1	. 1	2	1	3		

1 50% south via Trafalgar 50% east via Wellington Road 22
 2 30% east via Wellington Road 22 and 70% south and within the Hillsburgh BA
 3 north via Trafalgar

APPENDIX G

SIGNAL WARRANT ANALYSIS

Intersection	Trafalgar Road North at George Street/Mill Street - Future (2026) Total Traffic
Number of Lanes on Main Road (1 = 2 lane 2= more than 2 lanes)	1
Rural (enter 1) or Urban (enter 2)	2
Existing (enter 1) or New (enter 2) intersection	1
T Intersection (yes =1 no = 2)	2

Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
AM	12	262	14	2	617	1	5	1	8	34	0	36
PM	71	596	54	22	372	8	23	16	48	34	13	22
Average Hourly Volume	21	215	17	6	247	2	7	4	14	17	3	15

Justification 7

		Mini Requirem			mum ent 2 lane			Existing /New	Scenario	Scenario Volume			с	ompliance	%	
Justification	Description	Rural	Urban	Rural	Urban	Initial Requirment	T intersection Factor	Intersection Factor	Requirement	AM	PM	AHV	AM	PM	AHV	Justification
1A Minimum Veh. Volume 1B Minimum Veh. Volume	All Approaches Minor Street	480					1	1.2	864 204	992	1279 156	568	100%	100%	66%	No
2A Crossing Traffic	Major Street Volume	480	720	600	900	720	1	1.2	864	908	1123	508	100%	100%	59%	
2B Crossing Traffic	Crossing volume of Minor Street	50	75	120	170	75	1	1.2	90	40	73	28.25	44%	81%	31%	No

Intersection	Trafalgar Road North at George Street/Mill Street - Future (2031) Total Traffic
Number of Lanes on Main Road (1 = 2 lane 2= more than 2 lanes)	1
Rural (enter 1) or Urban (enter 2)	2
Existing (enter 1) or New (enter 2) intersection	1
T Intersection (yes =1 no = 2)	2

Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
AM	12	292	14	2	683	1	4	1	8	34	0	34
PM	71	596	54	22	372	8	23	16	48	34	13	22
Average Hourly Volume	21	222	17	6	264	2	7	4	14	17	3	14

Justification 7

		Mini Requirem			mum ent 2 lane			Existing /New	Scenario	Scenario Volume			с	ompliance	%	
Justification	Description	Rural	Urban	Rural	Urban	Initial Requirment	T intersection Factor	Intersection Factor	Requirement	AM	PM	AHV	AM	PM	AHV	Justification
1A Minimum Veh. Volume 1B Minimum Veh. Volume	All Approaches Minor Street	480					1	1.2	864	1085	1279		100%	100%	68%	No
2A Crossing Traffic	Major Street Volume	480	720	600	900	720	1	1.2	864	1004	1123	532	100%	100%	62%	
2B Crossing Traffic	Crossing volume of Minor Street	50	75	120	170	75	1	1.2	90	39	73	28	43%	81%	31%	No

APPENDIX H

SITE VISIT PICTURES FOR SIGHT DISTANCE MEASUREMENTS



PROPOSED STREET 'A'/HOWE STREET AT TRAFALGAR ROAD NORTH – LOOKING NORTH



PROPOSED STREET 'A'/HOWE STREET AT TRAFALGAR ROAD NORTH – LOOKING SOUTH



PROPOSED STREET 'E' AT TRAFALGAR ROAD NORTH – LOOKING NORTH



PROPOSED STREET 'E' AT TRAFALGAR ROAD NORTH – LOOKING SOUTH