TRAFFIC IMPACT STUDY

# PROPOSED RESIDENTIAL SUBDIVISION 

 HILLSBURGH HEIGHTS INC.5916 TRAFALGAR ROAD NORTH
HILLSBURGH URBAN AREA
TOWN OF ERIN
TOWN FILE NOS. OP21-01 \& Z21-09

UPDATED JULY $\mathbf{2 8}^{\text {TH }} 2022$

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## 1. INTRODUCTION

CANDEVCON LIMITED was retained by Hillsburgh Heights Inc. to undertake a Traffic Impact Study in support of the Draft Plan of Subdivision and Zoning By-law Amendment Application for the proposed Residential Subdivision that is located immediately west of Trafalgar Road North and approximately two (2) kilometres north of Wellington Road 22, in the Town of Erin. Figure 1 illustrates the location of the proposed Residential Subdivision.

As a requirement of the approval process, the Town of Erin and the County of Wellington require the preparation of a Traffic Impact Study to support the proposed Residential Subdivision and to examine the implications of the proposed Residential Subdivision on the adjacent transportation infrastructure.

It is anticipated that the proposed Residential Subdivision will be fully built-out and occupied by 2026. As a result, a full build-out 2026 horizon along with a five (5) year post build-out 2031 horizon were analyzed.

The Terms of Reference for the Study (copy included in Appendix A) were circulated to the County of Wellington and to the Town of Erin and the comments were received as included in Appendix A. The comments ${ }^{1}$ provided on behalf of the Town requested that the TIS should also account for four other proposed developments in the Community of Hillsburgh. Since no information is available with respect to the development timing, it was assumed that they would also be fully built-out by 2026. The comments provided on behalf of the Town also stipulated that 5 year and 10 year post full build-out be analyzed. Since there are too many variables associated with a 10 year build-out in the context of other potential developments, a 10 year build-out was not analyzed.

The purpose of this Study is to determine the traffic impacts of the proposed Residential Subdivision as well as the other proposed developments on the surrounding road network.

[^0]
## 1. INTRODUCTION (CONT'D)

The Traffic Impact Study addresses the future operations at the following intersections as stipulated by the Town:

- Trafalgar Road North at Wellington Road 22,
- George Street/Mill Street at Trafalgar Road North,
- Upper Canada Drive/Church Street at Trafalgar Road North,
- Proposed Street 'A'/Howe Street at Trafalgar Road North,
- Proposed Street 'E' at Trafalgar Road North,
- Proposed Street 'A' at proposed Street ' $G$ '/future Local Road.

The Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North and Howe Street at Trafalgar Road North intersections were studied under the Existing (2021), the Future (2026 \& 2031) Total Background and the Future ( 2026 \& 2031) Total Traffic scenarios. The proposed Street 'E' at Trafalgar Road North and proposed Street 'A' at proposed Street 'G'/future Local Road intersections were studied under the Future (2026 \& 2031) Total Traffic scenarios.

The Traffic Impact Study addresses the traffic operations during the typical Weekday A.M. and Weekday P.M. Peak Hours.


TRAFFIC IMPACT STUDY
HILLSBURGH HEIGHTS INC. PROPOSED RESIDENTIAL SUBDIVISION

5916 TRAFALGAR ROAD NORTH PART 1 OF PLAN 61R-9590 PART OF LOT 26, CONCESSION 7 HILLSBURGH URBAN AREA TOWN OF ERIN

LOCATION PLAN

| CANDEVCON LIMITED |  |
| :---: | :---: |
| 9358 GOREWAY DRNE TEL. (905) 794-0600 | BRAMPTON, ONTARIO L6P OM7 FAX (905) 794-0611 |
| DRAWN BY: | PROJECT No. |
| K.F. | W21081 |
| $\text { CHECKED BY: } \text { B.W. }$ | FIGURE No. |
| SCALE: N.T.S. |  |
| DATE: <br> JULY 15th 2022 |  |

## 2. SUBJECT DEVELOPMENT - STUDY AREA

The Subject Property is located immediately west of Trafalgar Road North and approximately two (2) kilometres north of Wellington Road 22. The total area of the property is 79.81 acres.

The Subject Development is surrounded by the following land uses:

- To the north, development lands owned by the applicant with future commercial and residential land uses,
- To the east, Trafalgar Road North with existing residential beyond,
- To the south, existing residential with Upper Canada Drive beyond,
- To the west, vacant lands owned by the applicant and other vacant lands with woodlands beyond.

The proposed Residential Subdivision comprises 196 single detached homes (includes the heritage house in Block 8), 174 townhouse units, a school block, a park and twelve (12) public roads. Vehicle access to the proposed Residential Subdivision is provided via Streets 'A' and 'E', which connects with Trafalgar Road North. The proposed Street 'A' access will be a full-moves access that aligns with Howe Street to form a four-legged intersection. The proposed Street ' $E$ ' access will be a full-moves access that is located at the southeast corner of the Subject Property. In addition, west of the Howe Street/proposed Street 'A' at Trafalgar Road North intersection, proposed Street ' B ' and a future Local Road will align at proposed Street ' A '; forming a fourlegged roundabout intersection.

To minimize the proposed Residential Subdivision's impact to Trafalgar Road North, an internal connection to McMurchy Lane was considered so that site-generated trips can use Upper Canada Drive to access Trafalgar Road North rather than providing the Street 'E' at Trafalgar Road North intersection. However, it was determined that an internal connection to McMurchy Lane was not feasible due to grading constraints.

The proposed Draft Plan of Subdivision is provided in Figure 2.


## 3. EXISTING AND FUTURE ROAD NETWORK

### 3.1 Existing Road Network

The road network within the Study Area is described below:

## Trafalgar Road North

Trafalgar Road North is an existing Arterial Road that is under the jurisdiction of the County of Wellington. Within the Study Area, Trafalgar Road North is a two (2) lane roadway with a posted speed limit of $40 \mathrm{~km} / \mathrm{h}$. From Wellington Road 22 to approximately 200 metres south of Howe Street the roadway consists of an urban crosssection. From approximately 200 metres south of Howe Street to the north end of the Study Area (Howe Street), the roadway consists of a rural cross-section. Within the Study Area, where an urban cross-section is provided, a pedestrian sidewalk or a multiuse path is provided on at least one side of the roadway.

## Wellington Road 22

Wellington Road 22 is an existing Arterial Road that is under the jurisdiction of the County of Wellington. Within the vicinity of the Study Area, Wellington Road 22 is a two (2) lane roadway with a rural cross-section. The roadway has a posted speed limit of $70 \mathrm{~km} / \mathrm{h}$ east of Trafalgar Road North and a posted speed limit of $80 \mathrm{~km} / \mathrm{h}$ west of Trafalgar Road North.

## George Street

George Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of $50 \mathrm{~km} / \mathrm{h}$ and a rural cross section. George Street consists of two (2) components: an east-west roadway that connects with Trafalgar Road to the east and that terminates at a driveway of a dwelling unit to the west; and a roadway that travels in the west direction before travelling in the north direction with connections to Mill Street and Trafalgar Road North easterly and to its other component northerly.

## 3. EXISTING AND FUTURE ROAD NETWORK (CONT'D)

### 3.1 Existing Road Network (Cont'd)

## Mill Street

Mill Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of $50 \mathrm{~km} / \mathrm{h}$ and a rural cross section. From its connection with George Street and Trafalgar Road North, the roadway travels in the east direction before travelling in the north direction to connect with Orangeville Street.

## Upper Canada Drive

Upper Canada Drive is an existing east-west local road that is under the jurisdiction of the Town of Erin. The roadway connects with Trafalgar Road North and Church Street easterly and terminates in a cul-de-sac at the western end. The local road comprises two (2) lanes with an assumed speed limit of $50 \mathrm{~km} / \mathrm{h}$, an urban cross section and a pedestrian sidewalk on the south side.

## Church Street

Church Street is an existing local road that is under the jurisdiction of the Town of Erin. The local road comprises two (2) lanes with an assumed speed limit of $50 \mathrm{~km} / \mathrm{h}$ and a rural cross section. From its connection with Upper Canada Drive and Trafalgar Road North, the roadway travels in the east direction before travelling in the south direction to connect with Mill Street. From Trafalgar Road North to Barker Street, a pedestrian sidewalk is provided on at least one side of the roadway.

## Howe Street

Howe Street is an existing local road that is under the jurisdiction of the Town of Erin. The roadway connects with Trafalgar Road North at the west end to form a T-intersection and connects with Wallace Street at the east end to form a road elbow. The local road comprises two (2) lanes with an assumed speed limit of $50 \mathrm{~km} / \mathrm{h}$ and a rural cross section.

## 3. EXISTING AND FUTURE ROAD NETWORK (CONT'D)

### 3.2 Future Road Network

By the 2031 horizon year, it is not anticipated that Trafalgar Road North, Wellington Road 22, George Street, Mill Street, Upper Canada Drive, Church Street, and Howe Street will be widened. However, by the 2026 horizon year, it is anticipated that two (2) collector roads will be constructed within the vicinity of the Study Area ${ }^{2}$. A collector road (proposed West Collector Road) will connect with Wellington Road 22 at approximately 500 metres west of Trafalgar Road North. From Wellington Road 22, the collector road will travel in the north direction before travelling in the north-east direction to connect with Station Street. The east end of Station Street that connects with Trafalgar Road North will be upgraded to a collector road to form part of the proposed roadway. The second collector road (proposed East Collector Road) that is anticipated will connect with Wellington Road 22 at approximately 350 metres east of Trafalgar Road North. From Wellington Road 22, the collector road will travel in the north direction before travelling in the west direction to connect with Trafalgar Road North. The connection with Trafalgar Road North will be immediately south of the existing Station Street at Trafalgar Road North intersection. The proposed collector roads are illustrated in Figure 5.

The proposed Hillsburgh Heights Residential Subdivision comprises twelve (12) public roads with a full-moves access (proposed Street 'A') that connects with Trafalgar Road North and Howe Street and a full-moves access (proposed Street 'E') that connects with Trafalgar Road North. In addition, west of the Howe Street/proposed Street 'A' at Trafalgar Road North intersection, proposed Street ' $G$ ' and a future Local Road will align at proposed Street ' $A$ '; forming a four-legged roundabout intersection.

Given the existing conditions on Trafalgar Road in relation to pedestrian connections, there are currently no sidewalk on Trafalgar Road or Howe Street. There are no future warrants for any sidewalk connections required on the eastside of Trafalgar Road.

[^1]
## 4. EXISTING TRAFFIC CONDITIONS

### 4.1 Existing Traffic

The Existing (2021) traffic volumes for the concerned intersections are based on the turning movement counts taken by Ontario Traffic Inc. (OTI) on Thursday October 28, 2021. (See Appendix B) To capture the Weekday A.M. and P.M. Peak Hours, counts were taken from 7:00 A.M. to 10:00 A.M. and from 3:00 P.M. to 6:00 P.M.

For the intersection of Trafalgar Road North at Wellington Road 22, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:30 A.M. and 8:30 A.M. and between 4:15 P.M. and 5:15 P.M., respectively.

For the intersection of George Street/Mill Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:15 A.M. and 8:15 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

For the intersection of Upper Canada Drive/Church Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 7:15 A.M. and 8:15 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

For the intersection of Howe Street at Trafalgar Road North, the A.M. and P.M. Peak Hour traffic volumes occurred between 8:00 A.M. and 9:00 A.M. and between 3:45 P.M. and 4:45 P.M., respectively.

The turning movement counts were conducted during the Covid-19 pandemic. Although all of the services were open and most of the capacity restrictions were lifted when these counts were taken, some offices were still not operating at full capacity and the traffic volumes are anticipated to be less than typical. Based on this assumption, a pandemic projection factor of $20 \%$ was used conservatively to project the traffic volumes to typical conditions. To determine the Existing (2021) Traffic Volumes, the projection factor was applied to all of the turning movements.

The Existing (2021) Traffic Volumes are illustrated in Figures 3 and 4


| Figure No: | 3 |
| :--- | :--- |
| Date: | November 9 2021 |
| Prepared by: | B.W. | own of Erin



| Figure No: | 4 |  |
| :--- | :--- | :--- |
| Date: | November 9 2021 |  |
| Prepared by: | B.W. | $\mathbf{N}$ |

## 4. EXISTING TRAFFIC CONDITIONS (CONT'D)

### 4.2 Existing Traffic Analysis

The Existing (2021) peak hour traffic volumes are provided in Figures 3 and 4 and the Level of Service (LOS) was analyzed using SYNCHRO 9.0 software ${ }^{3}$.

Trafalgar Road North at Wellington Road 22 was analyzed as a semi-actuated signalized intersection with Trafalgar Road North as the main street. The lane configuration used in the analysis comprises a left and a shared through-right turning lane at the northbound and southbound approaches; and a shared left-through-right turning lane at the eastbound and westbound approaches. The signal timing plans were received from the County of Wellington and are included in Appendix C.

George Street/Mill Street at Trafalgar Road North was analyzed as an un-signalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Upper Canada Drive/Church Street at Trafalgar Road North was analyzed as an unsignalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Howe Street at Trafalgar Road North was analyzed as an un-signalized intersection with a stop-control at the westbound approach. The lane configuration used in the analysis comprises a through and a right turning lane at the northbound approach; a shared leftright turning lane at the westbound approach; and a shared through-left turning lane at the southbound approach.

[^2]
## 4. EXISTING TRAFFIC CONDITIONS (CONT'D)

### 4.2 Existing Traffic Analysis (Cont'd)

The results of the analysis are summarized in Table 1. The related calculations are provided in Appendix E. The LOS definitions for signalized and un-signalized intersections are included in Appendix D for reference.

## 4. EXISTING TRAFFIC CONDITIONS (CONT'D)

### 4.2 Existing Traffic Analysis (Cont'd)

Table 1: Existing (2021) Traffic - Level of Service

| Intersection | Turning Lane /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \end{gathered}$ | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \end{gathered}$ |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 0.52 | B | 13.9 | n/a | 0.60 | B | 15.2 | n/a |
|  | EB Approach | 0.37 | B | 20.0 | 25.5 | 0.37 | C | 23.2 | 26.9 |
|  | WB Approach | 0.52 | C | 25.2 | 31.9 | 0.60 | C | 23.2 | 42.9 |
|  | NBL | 0.02 | A | 8.5 | 3.3 | 0.06 | A | 9.1 | 6.8 |
|  | NB TR | 0.23 | A | 7.9 | 22.2 | 0.51 | B | 12.6 | 70.6 |
|  | SBL | 0.17 | A | 9.7 | 14.1 | 0.15 | B | 10.2 | 11.6 |
|  | SB TR | 0.30 | A | 9.9 | 34.5 | 0.22 | A | 9.1 | 27.8 |
| Trafalgar Road North <br> at <br> Howe Street <br> (Un-signalized) | Overall | 0.09 | A | 0.2 | n/a | 0.24 | A | 0.2 | n/a |
|  | WB Approach | 0.01 | A | 9.8 | 0.2 | 0.01 | B | 13.8 | 0.2 |
|  | NB Approach | 0.09 | A | 0.0 | 0.0 | 0.24 | A | 0.0 | 0.0 |
|  | SB Approach | 0.00 | A | 0.2 | 0.1 | 0.01 | A | 0.3 | 0.1 |
| Trafalgar Road North <br> at <br> George Street/ <br> Mill Street <br> (Un-signalized) | Overall | 0.10 | A | 1.5 | n/a | 0.23 | A | 4.3 | n/a |
|  | EB Approach | 0.02 | B | 10.9 | 0.4 | 0.22 | C | 16.5 | 6.5 |
|  | WB Approach | 0.10 | B | 12.8 | 2.6 | 0.23 | C | 20.3 | 7.1 |
|  | NB Approach | 0.01 | A | 0.4 | 0.2 | 0.05 | A | 1.6 | 1.4 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.7 | 0.4 |
| Trafalgar Road North <br> at <br> Upper Canada Drive <br> /Church Street <br> (Un-signalized) | Overall | 0.04 | A | 1.0 | n/a | 0.05 | A | 0.8 | n/a |
|  | EB Approach | 0.03 | B | 10.1 | 0.8 | 0.00 | A | 9.6 | 0.1 |
|  | WB Approach | 0.04 | B | 12.3 | 1.1 | 0.05 | B | 14.8 | 1.2 |
|  | NB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.5 | 0.3 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.2 | 0.1 |

Note 1: Delays are measured in seconds per vehicle.
Note 2: Signalized intersections are based on existing signal timing plans.

## 4. EXISTING TRAFFIC CONDITIONS (CONT'D)

### 4.2 Existing Traffic Analysis (Cont'd)

## Trafalgar Road North at Wellington Road 22

The analysis of the Existing (2021) Traffic Conditions indicates that the signalized intersection operates at a Level of Service "B" during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements operate at a Level of Service "C" or better.

## Trafalgar Road North at Howe Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service "A" during the A.M. Peak Hour and at a Level of Service "B" or better during the P.M. Peak Hour.

## Trafalgar Road North at George Street/Mill Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service " $B$ " or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

## 4. EXISTING TRAFFIC CONDITIONS (CONT'D)

### 4.2 Existing Traffic Analysis (Cont'd)

Trafalgar Road North at George Street/Mill Street

The analysis of the Existing (2021) Traffic Conditions indicates that the un-signalized intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours.

All of the turning movements operate at a Level of Service "B" or better during the A.M. and P.M. Peak Hours.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS

### 5.1 Other Background Traffic

The Study will consider the site-generated trips from five (5) anticipated developments within the vicinity of Study Area.

An anticipated development owned by Carson Reid Homes Ltd is located immediately south of Station Street and approximately 300 metres west of Trafalgar Road North. The anticipated development will be serviced by the proposed West Collector Road and will comprise of 182 single detached homes.

An anticipated development owned by Thomasfield Homes Ltd is located immediately north of Wellington Road 22 and approximately 450 metres west of Trafalgar Road North. The anticipated development will be serviced by the proposed West Collector Road and will comprise of 210 single detached homes.

An anticipated development owned by Dominion Packers \& Realties (Tavares) that comprises 700 single detached homes is located immediately south of Douglas Crescent and east of Trafalgar Road North. It is assumed that the anticipated development will be serviced by the proposed East Collector Road and the Spruce Street roadway, which is connected to Mill Street.

An anticipated development owned by Chantler that comprises 213 single detached homes is located immediately north of Wellington Road 22 and approximately 350 metres east of Trafalgar Road North. It is assumed that the anticipated development will be serviced by the proposed East Collector Road and Wellington Street, which comprises a westerly connection with Trafalgar Road North.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS

### 5.1 Other Background Traffic

A proposed Residential Subdivision owned by the applicant is immediately north of the Subject Subdivision. At the time this report was prepared, details to the proposed Residential Subdivision are preliminary. However, the proposed Residential Subdivision comprises residential land uses at the north end and commercial land uses at the southeast corner of the property. For this Study, it is assumed that the proposed Residential Subdivision will comprise of 44 single detached homes, 23 townhouse units and 60,000 $\mathrm{ft}^{2}$ of commercial land use. It is also assumed that access to the residential land uses within the proposed Residential Subdivision will be provided via connection with the elbow for Street ' B ', single detached lots fronting the north end of Street ' C ' and a future Local Road that aligns with Street ' $G$ ' at Street 'A' to form a four-legged roundabout intersection. For the commercial land uses at the southeast corner of the property, it is assumed that access is provided via a full-moves access at Trafalgar Road North, a right-in/right-out access at Street 'A' and a full-moves access at the future Local Road that aligns with Street ' $G$ ' at Street 'A' to form a four-legged roundabout intersection. Unlike the four (4) anticipated background developments mentioned above, it is anticipated that the proposed Residential Subdivision will be fully built and occupied between the 2026 and 2031 horizon years. Therefore, site-generated trips from the proposed Residential Subdivision will only be included in the 2031 horizon year. In addition, since access to the anticipated background development is dependent on the construction of the Subject Subdivision, the site-generated trips from the proposed Residential Subdivision will only be included in the Future (2031) Total Traffic scenario.

The locations of the anticipated developments and the future road network are illustrated in Figure 5.


TRAFFIC IMPACT STUDY
HILLSBURGH HEIGHTS INC. PROPOSED RESIDENTIAL SUBDIVISION

5916 TRAFALGAR ROAD NORTH PART 1 OF PLAN 61R-9590
PART OF LOT 26, CONCESSION 7 HILLSBURGH URBAN AREA TOWN OF ERIN

THE FUTURE ROAD NETWORK AND THE LOCATION OF ANTICIPATED DEVELOPMENTS

- CANDEVCON LIMITED CONSULTING ENGINEERS AND PLANNERS $\begin{array}{ll}\text { 9358 GOREWAY DRNE } & \text { BRAMPTON, ONTAR1O L6P } \\ \text { TEL. (905) 794-0600 } & \text { FAX } \\ \text { (905) 794-0611 }\end{array}$

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| $\text { CHECKED BY: } \text { B.W. }$ | FIGURE No. |
| SCALE: N.T.S. |  |
| DATE: <br> JULY 20th 2022 |  |

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.1. Other Background Traffic - Carson Reid Homes Ltd

For the single detached homes (Land Use 210) within the anticipated development, the trip generation formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours ${ }^{4}$.

Table 2 summarizes the trip generation formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

Table 2: Trip Generation Formulae with Inbound and Outbound Percentages - Anticipated Developments

| ITE Land Use | A.M. Peak Hour |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Fitted Curve <br> Equation | \% In | \% Out | Fitted Curve <br> Equation | \% In | \% Out |
| Single-Family <br> Detached Housing | $\mathrm{T}=0.71 \mathrm{X}+4.80$ <br> (Note 1) | $25 \%$ | $75 \%$ | $\operatorname{Ln}(\mathrm{T})=0.96 \operatorname{Ln}(\mathrm{X})+0.20$ <br> (Note 1) | $63 \%$ | $37 \%$ |
| (LU 210) |  |  |  |  |  |  |

Note 1: T represents the total number of trips and X represents the number of dwelling units.
The resulting number of trips generated was determined by the trip generation formulae in Table 2 and the number of dwelling units. The anticipated development comprises 182 single detached homes.

The resulting number of trips generated is provided in Table 3 for the A.M. and P.M. Peak Hours of adjacent street traffic.

[^3]
## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.1. Other Background Traffic - Carson Reid Homes Ltd (Cont'd)

Table 3: Site-Generated Trips - Carson Reid Homes Ltd

| ITE Land Use | No. of <br> dwelling units | A.M. Peak Hour <br> Trips <br> In |  |  | Trips <br> Out | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Trips <br> Out | Total |  |  |  |  |  |
| Single-Family Detached |  | 182 | 34 | 100 | $\mathbf{1 3 4}$ | 114 | 67 |  |
| Housing <br> (LU 210) |  | $\mathbf{1 8 1}$ |  |  |  |  |  |  |

The anticipated development is expected to generate a total of 134 trips during the A.M. Peak Hour (34 inbound trips and 100 outbound trips) and 181 trips during the P.M. Peak Hour (114 inbound trips and 67 outbound trips).

For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in Appendix F.

The assumed trip distribution and assignment will be as follows:

- $11 \%$ ( $11 \%$ ) to/from the north and within the Study Area via Trafalgar Road North,
- $47 \%$ ( $47 \%$ ) to/from the east via Wellington Road 22,
- $42 \%(42 \%)$ to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in Figures 6 and 7.



## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.2. Other Background Traffic - Thomasfield Homes Ltd

The resulting number of trips generated was determined by the trip generation formulae in Table 2 and the number of dwelling units. The anticipated development comprises 210 single detached homes.

The resulting number of trips generated is provided in Table 4 for the A.M. and P.M. Peak Hours of adjacent street traffic.

Table 4: Site-Generated Trips - Thomasfield Homes Ltd

| ITE Land Use | No. of <br> dwelling units | A.M. Peak Hour <br> In |  |  | Trips <br> Out | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

The anticipated development is expected to generate a total of 154 trips during the A.M. Peak Hour ( 39 inbound trips and 115 outbound trips) and 207 trips during the P.M. Peak Hour (130 inbound trips and 77 outbound trips).

For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- $11 \%$ ( $11 \%$ ) to/from the north and within the Study Area via Trafalgar Road North,
- $47 \%$ ( $47 \%$ ) to/from the east via Wellington Road 22,
- $42 \%$ ( $42 \%$ ) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in Figures 8 and 9.



## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.3. Other Background Traffic - Dominion Packers \& Realties (Tavares)

The resulting number of trips generated was determined by the trip generation formulae in Table 2 and the number of dwelling units. The anticipated development comprises 700 single detached homes.

The resulting number of trips generated is provided in Table 5 for the A.M. and P.M. Peak Hours of adjacent street traffic.

Table 5: Site-Generated Trips - Dominion Packers \& Realties (Tavares)

| ITE Land Use | No. of <br> dwelling units | A.M. Peak Hour <br> In |  |  | Trips <br> Out | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Trips <br> Tn | Trips <br> Out | Total |  |  |  |  |
| Single-Family Detached |  |  |  |  |  |  |  |
| Housing <br> (LU 210) | 700 | 126 | 376 | $\mathbf{5 0 2}$ | 415 | 243 | $\mathbf{6 5 8}$ |  |

The anticipated development is expected to generate a total of 502 trips during the A.M. Peak Hour ( 126 inbound trips and 376 outbound trips) and 658 trips during the P.M. Peak Hour (415 inbound trips and 243 outbound trips).

For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- $11 \%(11 \%)$ to/from the north and within the Study Area via Trafalgar Road North,
- $47 \%$ ( $47 \%$ ) to/from the east via Wellington Road 22,
- $42 \%$ ( $42 \%$ ) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in Figures 10 and 11.



## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.4. Other Background Traffic - Chantler

The resulting number of trips generated was determined by the trip generation formulae in Table 2 and the number of dwelling units. The anticipated development comprises 213 single detached homes.

The resulting number of trips generated is provided in Table 6 for the A.M. and P.M. Peak Hours of adjacent street traffic.

Table 6: Site-Generated Trips - Chantler

| ITE Land Use | No. of dwelling units | A.M. Peak Hour |  |  | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Trips In | Trips <br> Out | Total | Trips In | Trips Out | Total |
| Single-Family Detached Housing <br> (LU 210) | 213 | 39 | 117 | 156 | 132 | 78 | 210 |

The anticipated development is expected to generate a total of 156 trips during the A.M. Peak Hour ( 39 inbound trips and 117 outbound trips) and 210 trips during the P.M. Peak Hour (132 inbound trips and 78 outbound trips).

For the site-generated trips from the anticipated development, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- $11 \%$ ( $11 \%$ ) to/from the north and within the Study Area via Trafalgar Road North,
- $47 \%$ ( $47 \%$ ) to/from the east via Wellington Road 22,
- $42 \%$ ( $42 \%$ ) to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in Figures 12 and 13.



## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.5. Other Background Traffic - Anticipated Background Development Owned by the Applicant

For the single detached homes (Land Use 210), the townhouse units (Land Use 220) and the commercial land uses (Land Use 820) within the anticipated development, the trip generation formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours.

Table 7 summarizes the trip generation formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

Table 7: Trip Generation Formulae with Inbound and Outbound Percentages - Anticipated Background Development Owned by the Applicant

| ITE Land Use | A.M. Peak Hour |  |  | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Fitted Curve Equation | \% In | \% Out | Fitted Curve Equation | \% In | \% Out |
| Single-Family Detached Housing <br> (LU 210) | $\begin{aligned} \mathrm{T}= & 0.71 \mathrm{X}+4.80 \\ & \text { (Note 1) } \end{aligned}$ | 25\% | 75\% | $\begin{aligned} \operatorname{Ln}(T)= & 0.96 \operatorname{Ln}(X)+0.20 \\ & (\text { Note } 1) \end{aligned}$ | 63\% | 37\% |
| Multifamily Housing (Low-Rise) (LU 220) | $\begin{aligned} \operatorname{Ln}(T)= & 0.95 \operatorname{Ln}(\mathrm{X})-0.51 \\ & (\text { Note } 1) \end{aligned}$ | 23\% | 77\% | $\begin{aligned} \operatorname{Ln}(T)= & 0.89 \operatorname{Ln}(X)-0.02 \\ & (\text { Note 1) } \end{aligned}$ | 63\% | 37\% |
| Shopping Centre <br> (LU 820) | $\begin{gathered} \mathrm{T}=\underset{ }{0.50 \mathrm{X}+151.78} \mathrm{( } \mathrm{Note} \mathrm{2)} \end{gathered}$ | 62\% | 38\% | $\begin{gathered} \operatorname{Ln}(\mathrm{T})=0.74 \operatorname{Ln}(\mathrm{X})+ \\ 2.89 \\ (\text { Note } 2) \end{gathered}$ | 48\% | 52\% |

Note 1: T represents the total number of trips and $X$ represents the number of dwelling units.
Note 2: T represents the total number of trips and X represents every 1,000 square feet of G.L.A.

The resulting number of trips generated was determined by the trip generation formulae in Table 7 and the land uses that were assumed. It is assumed that the anticipated development comprises 44 single detached homes, 23 townhouse units and $60,000 \mathrm{ft}^{2}$ of commercial land use. For the commercial land uses, this Study applied a pass-by percentage of $34 \%$ for the P.M. Peak Hour. The pass-by trip percentage was based on the data provided in the ITE Trip GenerationHandbook $3{ }^{\text {rd }}$ Edition ${ }^{5}$.

[^4]
## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.5. Other Background Traffic - Anticipated Background Development Owned by the Applicant (Cont'd)

The resulting number of trips generated is provided in Table $\mathbf{8}$ for the A.M. and P.M. Peak Hours of adjacent street traffic.

Table 8: Site-Generated Trips - Anticipated Background Development Owned by the Applicant

| ITE Land Use | Quantity | Trips | A.M. Peak Hour |  |  | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Trips In | Trips Out | Total | Trips <br> In | Trips Out | Total |
| Single-Family Detached Housing <br> (LU 210) | 44 <br> Dwelling units | Gross Trips | 9 | 27 | 36 | 29 | 17 | 46 |
| Multifamily Housing (Low-Rise) <br> (LU 220) | 23 Dwelling Units | Gross Trips | 3 | 9 | 12 | 10 | 6 | 16 |
| Shopping Centre <br> (LU 820) | $\begin{gathered} \text { 60,000 } \mathrm{ft}^{2} \\ \text { G.L.A. } \end{gathered}$ | Gross Trips Passby Net Trips | $\begin{gathered} 113 \\ 0 \\ 113 \\ \hline \end{gathered}$ | $\begin{gathered} 69 \\ 0 \\ 69 \\ \hline \end{gathered}$ | $\begin{gathered} 182 \\ 0 \\ 182 \end{gathered}$ | $\begin{gathered} 179 \\ 61 \\ 118 \\ \hline \end{gathered}$ | $\begin{gathered} 193 \\ 61 \\ 132 \\ \hline \end{gathered}$ | $\begin{array}{r} 372 \\ 122 \\ 250 \\ \hline \end{array}$ |
| TOTAL | - | Gross Trips Passby Net Trips | $\begin{gathered} \hline 125 \\ 0 \\ 125 \\ \hline \end{gathered}$ | $\begin{gathered} 105 \\ 0 \\ 105 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 230 \\ 0 \\ \mathbf{2 3 0} \\ \hline \hline \end{gathered}$ | $\begin{gathered} \hline 218 \\ 61 \\ 157 \\ \hline \hline \end{gathered}$ | $\begin{gathered} 216 \\ 61 \\ 155 \\ \hline \end{gathered}$ | 434 <br> 122 <br> 312 |

The anticipated development is expected to generate a total of 230 trips during the A.M. Peak Hour ( 125 inbound trips and 105 outbound trips) and 434 trips during the P.M. Peak Hour (218 inbound trips and 216 outbound trips).

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.1.5. Other Background Traffic - Anticipated Background Development Owned by the Applicant (Cont'd)

For the single detached homes and the townhouse units, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment.

The assumed trip distribution and assignment will be as follows:

- $47 \%(47 \%)$ to/from the east via Wellington Road 22 ,
- $11 \%$ ( $11 \%$ ) to/from the north via Trafalgar Road North and within the Study Area,
- $42 \%$ ( $42 \%$ ) to/from the south via Trafalgar Road North.

For the commercial land uses, based on the nature of the land uses, the trip distribution and assignment is based on the residential land use within the vicinity of the proposed Residential Subdivision.

The assumed trip distribution and assignment will be as follows:

- $80 \%(80 \%)$ to/from the residential land uses within the Subject Subdivision and the anticipated background development owned by the applicant,
- $12 \%(12 \%)$ to/from the east via Howe Street,
- $5 \%$ (5\%) to/from the south via Upper Canada Drive and Trafalgar Road North,
- 3\% (3\%) to/from the south via Church Street.

The site-generated trip volumes and trip assignment used in the analysis for the anticipated development are illustrated in Figures 14 and 15.



## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.2 Traffic Growth Rate

The traffic growth rates for Trafalgar Road North and Wellington Road 22 were obtained from the County of Wellington. An annual growth rate of $2 \%$ was considered for these roadways from 2021 to 2031.

For the George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North and Howe Street at Trafalgar Road North intersections, traffic growth was applied to the through movements on Trafalgar Road North. For the intersection of Trafalgar Road North at Wellington Road 22, an annual growth rate of $2 \%$ was applied to all of the turning movements.

### 5.3 Future (2026) Total Background Traffic

The Future (2026) Total Background Traffic is based on the Existing (2021) Traffic volumes projected with traffic growth for five (5) years for Trafalgar Road North and Wellington Road 22 plus the site-generated trips from the anticipated developments owned by Carson Reid Homes Ltd, Thomasfield Homes Ltd, Dominion Packers \& Realties (Tavares) and Chantler. The site-generated trip volumes used in the analysis for the anticipated developments are illustrated in Figures 16 and 17.

The Future (2026) Total Background Traffic Volumes are illustrated in Figures 18 and 19 for the A.M. and P.M. Peak Hours.


| Figure No: | 16 |
| :--- | :--- |
| Date: | July 202022 |
| Prepared by: | B.w. |





| Figure No: | 18 |  |
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| Date: | July 202022 |  |
| Prepared by: | B.W. | $\mathbf{N}$ |

Future (2026) Total Background Traffic Volumes - P.M. Peak Hour

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.4 Future (2026) Total Background Traffic Analysis

For the Future (2026) Total Background Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

The signal timing plans and the lane configurations used in the Existing (2021) Traffic Analysis are used in the Future (2026) Total Background Traffic Analysis.

The results of the analysis are summarized in Table 9. The related calculations are provided in Appendix E.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

Table 9: Future (2026) Total Background Traffic - Level of Service

| Intersection | Turning Lane /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \\ \hline \hline \end{gathered}$ | V/C | LOS | Delay ${ }^{1}$ | $\begin{aligned} & \text { 95th } \\ & \text { Queue } \\ & (\mathrm{m}) \end{aligned}$ |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 1.02 | C | 31.9 | n/a | 0.96 | D | 37.7 | n/a |
|  | EB Approach | 0.61 | C | 23.3 | 64.7 | 0.57 | C | 23.5 | 54.3 |
|  | WB Approach | 1.02 | F | 81.8 | 100.1 | 0.93 | D | 50.6 | 122.7 |
|  | NBL | 0.12 | B | 13.3 | 9.6 | 0.34 | B | 16.7 | 26.5 |
|  | NB TR | 0.42 | B | 13.9 | 42.7 | 0.96 | D | 45.0 | 193.5 |
|  | SBL | 0.27 | B | 15.4 | 20.0 | 0.76 | E | 69.8 | 33.3 |
|  | SB TR | 0.57 | B | 18.9 | 71.0 | 0.38 | B | 14.9 | 49.8 |
| Trafalgar Road North <br> at <br> Howe Street <br> (Un-signalized) | Overall | 0.10 | A | 0.2 | n/a | 0.26 | A | 0.2 | n/a |
|  | WB Approach | 0.01 | A | 9.9 | 0.2 | 0.01 | B | 14.7 | 0.3 |
|  | NB Approach | 0.10 | A | 0.0 | 0.0 | 0.26 | A | 0.0 | 0.0 |
|  | SB Approach | 0.00 | A | 0.2 | 0.1 | 0.01 | A | 0.2 | 0.2 |
| Trafalgar Road North <br> at <br> George Street/ <br> Mill Street <br> (Un-signalized) | Overall | 0.12 | A | 1.5 | n/a | 0.30 | A | 4.6 | n/a |
|  | EB Approach | 0.02 | B | 11.4 | 0.5 | 0.26 | C | 19.1 | 8.4 |
|  | WB Approach | 0.12 | B | 14.4 | 3.2 | 0.30 | D | 25.8 | 9.6 |
|  | NB Approach | 0.01 | A | 0.5 | 0.3 | 0.06 | A | 1.7 | 1.5 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.02 | A | 0.6 | 0.4 |
| Trafalgar Road North at | Overall | 0.05 | A | 1.0 | n/a | 0.05 | A | 0.8 | n/a |
|  | EB Approach | 0.03 | B | 10.3 | 0.8 | 0.00 | A | 9.7 | 0.1 |
|  | WB Approach | 0.05 | B | 12.8 | 1.1 | 0.05 | C | 15.9 | 1.4 |
| Church Street <br> (Un-signalized) | NB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.4 | 0.3 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.2 | 0.1 |

Note 1: Delays are measured in seconds per vehicle.
Note 2: Signalized intersections are based on existing signal timing plans.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

## Trafalgar Road North at Wellington Road 22

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "C" during the A.M. Peak Hour and a Level of Service "D" during the P.M. Peak Hour. With the growth in background traffic, impacts to the intersection moderate during the A.M. and P.M. Peak Hours.

During the A.M. Peak Hour, the westbound approach will begin to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 due to the growth in background traffic. All of the other turning movements will continue to operate at a Level of Service "C" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "E" or better during the P.M. Peak Hour.

## Trafalgar Road North at Howe Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service "A" during the A.M. Peak Hour and at a Level of Service "B" or better during the P.M. Peak Hour.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.4 Future (2026) Total Background Traffic Analysis (Cont'd)

## Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. Peak Hour and low during the P.M. Peak Hour.

All of the turning movements will continue to operate at a Level of Service " B " or better during the A.M. Peak Hour and will begin to operate at a Level of Service "D" or better during the P.M. Peak Hour.

## Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2026) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service "B" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "C" or better during the P.M. Peak Hour.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.5 Future (2031) Total Background Traffic

The Future (2031) Total Background Traffic is based on the Existing (2021) Traffic volumes projected with traffic growth for ten (10) years for Trafalgar Road North and Wellington Road 22 plus the site-generated trips from the anticipated developments owned by Carson Reid Homes Ltd, Thomasfield Homes Ltd, Dominion Packers \& Realties (Tavares) and Chantler.

The Future (2031) Total Background Traffic Volumes are illustrated in Figures 20 and 21 for the A.M. and P.M. Peak Hours.

### 5.6 Future (2031) Total Background Traffic Analysis

For the Future (2031) Total Background Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

The signal timing plans and the lane configurations used in the Future (2026) Total Background Traffic Analysis are used in the Future (2031) Total Background Traffic Analysis.

The results of the analysis are summarized in Table 10. The related calculations are provided in Appendix E.



| Figure No: | 20 |  |
| :--- | :--- | :--- |
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| Prepared by: | B.W. | $\mathbf{N}$ |




| Figure No: | 21 |  |
| :--- | :--- | :--- |
| Date: | July 202022 |  |
| Prepared by: | B.W. | $\mathbf{N}$ |

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

Table 10: Future (2031) Total Background Traffic - Level of Service

| Intersection | Turning Lane <br> /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | $\begin{aligned} & \text { 95th } \\ & \text { Queue } \\ & (\mathrm{m}) \end{aligned}$ | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \\ \hline \end{gathered}$ |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 1.10 | D | 37.6 | n/a | 1.04 | D | 48.5 | n/a |
|  | EB Approach | 0.64 | C | 24.5 | 69.5 | 0.60 | C | 25.0 | 59.3 |
|  | WB Approach | 1.10 | F | 106.0 | 108.5 | 0.97 | E | 59.1 | 133.9 |
|  | NBL | 0.14 | B | 13.6 | 9.9 | 0.38 | B | 17.8 | 28.3 |
|  | NB TR | 0.45 | B | 14.6 | 46.8 | 1.04 | E | 65.2 | 213.2 |
|  | SBL | 0.31 | B | 16.2 | 22.3 | 0.85 | F | 86.5 | 36.6 |
|  | SB TR | 0.62 | B | 20.0 | 78.0 | 0.42 | B | 15.6 | 54.4 |
| Trafalgar Road North <br> at <br> Howe Street <br> (Un-signalized) | Overall | 0.11 | A | 0.2 | n/a | 0.29 | A | 0.2 | n/a |
|  | WB Approach | 0.01 | B | 10.1 | 0.2 | 0.01 | C | 15.8 | 0.3 |
|  | NB Approach | 0.11 | A | 0.0 | 0.0 | 0.29 | A | 0.0 | 0.0 |
|  | SB Approach | 0.00 | A | 0.1 | 0.1 | 0.01 | A | 0.2 | 0.2 |
| Trafalgar Road North <br> at <br> George Street/ <br> Mill Street <br> (Un-signalized) | Overall | 0.13 | A | 1.4 | n/a | 0.33 | A | 4.8 | n/a |
|  | EB Approach | 0.02 | B | 11.8 | 0.5 | 0.29 | C | 20.9 | 9.4 |
|  | WB Approach | 0.13 | C | 15.3 | 3.5 | 0.33 | D | 29.2 | 11.1 |
|  | NB Approach | 0.01 | A | 0.5 | 0.3 | 0.06 | A | 1.7 | 1.6 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.02 | A | 0.6 | 0.4 |
| Trafalgar Road North <br> at <br> Upper Canada Drive/ <br> Church Street <br> (Un-signalized) | Overall | 0.05 | A | 0.9 | n/a | 0.06 | A | 0.8 | n/a |
|  | EB Approach | 0.04 | B | 10.6 | 0.9 | 0.00 | A | 9.9 | 0.1 |
|  | WB Approach | 0.05 | B | 13.5 | 1.2 | 0.06 | C | 17.2 | 1.5 |
|  | NB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.4 | 0.3 |
|  | SB Approach | 0.00 | A | 0.1 | 0.0 | 0.01 | A | 0.2 | 0.1 |

Note 1: Delays are measured in seconds per vehicle.
Note 2: Signalized intersections are based on existing signal timing plans.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

## Trafalgar Road North at Wellington Road 22

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "D" during the A.M. Peak Hour and will continue to operate at a Level of Service "D" during the P.M. Peak Hour. With the growth in background traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

During the A.M. Peak Hour, the westbound approach will continue to operate at a Level of Service "F" with a volume over capacity ratio that is greater than 1.0 due to the growth in background traffic.

In addition, during the P.M. Peak Hour, the shared through-right turning lane at the northbound approach will begin to operate with a volume over capacity ratio that is greater than 1.0 and the southbound left turning movement will begin to operate at a Level of Service "F" due to the growth in background traffic.

## Trafalgar Road North at Howe Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

## 5. FUTURE TOTAL BACKGROUND TRAFFIC CONDITIONS (CONT'D)

### 5.6 Future (2031) Total Background Traffic Analysis (Cont'd)

## Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. Peak Hour and low during the P.M. Peak Hour.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and will continue to operate at a Level of Service "D" or better during the P.M. Peak Hour.

## Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2031) Total Background Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection minor during the A.M. and P.M. Peak Hours.

All of the turning movements will continue to operate at a Level of Service " B " or better during the A.M. Peak Hour and at a Level of Service "C" or better during the P.M. Peak Hour.

## 6. TRIP GENERATION AND DISTRIBUTION

### 6.1. Trip Generation

For the single detached homes (Land Use 210), townhouse units (Land Use 220) and School Block (Land Use 520), the trip generation rates and formulae from the ITE Trip Generation Manual were applied for the A.M. and P.M. Peak Hours.

Based on the pre-consultation comments provided by the Upper Grand District School Board, it is assumed that the proposed School Block within the Subject Subdivision will be an elementary school with a capacity for 450 students. ${ }^{6}$

Table 11 summarizes the trip generation rates and formulae along with the percentages of incoming and outgoing trips for the A.M. and P.M. Peak Hours.

Table 11: Trip Generation Rates and Formulae with Inbound and Outbound Percentages

| ITE Land Use | A.M. Peak Hour |  |  | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Fitted Curve Equation | \% In | \% Out | Fitted Curve Equation | \% In | \% Out |
| Single-Family Detached Housing <br> (LU 210) | $\begin{aligned} \mathrm{T}= & 0.71 \mathrm{X}+4.80 \\ & (\text { Note } 1) \end{aligned}$ | 25\% | 75\% | $\begin{aligned} \operatorname{Ln}(T)= & 0.96 \operatorname{Ln}(X)+0.20 \\ & (\text { Note } 1) \end{aligned}$ | 63\% | 37\% |
| Multifamily Housing (Low-Rise) <br> (LU 220) | $\begin{aligned} \operatorname{Ln}(\mathrm{T})= & 0.95 \operatorname{Ln}(\mathrm{X})-0.51 \\ & (\text { Note 1) } \end{aligned}$ | 23\% | 77\% | $\begin{aligned} \operatorname{Ln}(\mathrm{T})= & 0.89 \operatorname{Ln}(\mathrm{X})-0.02 \\ & (\text { Note 1) } \end{aligned}$ | 63\% | 37\% |
| Elementary School <br> (LU 520) | $\begin{gathered} 0.67 \\ \text { (Note 2) } \end{gathered}$ | 54\% | 46\% | $\begin{gathered} 0.17 \\ \text { (Note 2) } \end{gathered}$ | 48\% | 52\% |

Note 1: T represents the total number of trips and $X$ represents the number of dwelling units.
Note 2: Trip rate is per student.

[^5]
## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.2 Total Site-Generated Trips

The resulting number of trips generated was determined by the trip generation rates and formulae in Table 11 and the proposed land uses. It is anticipated that the Subject Property will comprise of 196 single detached homes, 174 townhouse units and an Elementary School with a capacity for 450 students.

The resulting number of trips generated is provided in Table $\mathbf{1 2}$ for the A.M. and P.M. Peak Hours of adjacent street traffic.

Table 12: Site-Generated Trips

| ITE Land Use | Quantity | A.M. Peak Hour |  |  | P.M. Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Trips <br> In | Trips <br> Out | Total | Trips <br> In | Trips <br> Out | Total |
| Single-Family Detached <br> Housing <br> (LU 210) | 196 <br> dwelling units | 36 | 108 | 144 | 122 | 72 | 194 |
| Multifamily Housing <br> (Low-Rise) <br> (LU 220) | 174 <br> dwelling units | 19 | 62 | 81 | 61 | 36 | 97 |
| Elementary School <br> (LU 520) | 450 students | 163 | 139 | 302 | 37 | 40 | 77 |
| TOTAL | 218 | 309 | $\mathbf{5 2 7}$ | 220 | 148 | $\mathbf{3 6 8}$ |  |

The proposed Residential Subdivision is expected to generate a total of 527 trips during the A.M. Peak Hour (218 inbound trips and 309 outbound trips) and 368 trips during the P.M. Peak Hour (220 inbound trips and 148 outbound trips).

## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.3 Trip Distribution and Assignment

For the single detached homes and the townhouse units, the 2016 Transportation Tomorrow Survey and the future road network was utilized for the assumed trip distribution and trip assignment. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in Appendix F.

The assumed trip distribution and assignment will be as follows:

- $47 \%(47 \%)$ to/from the east via Wellington Road 22,
- $11 \%$ ( $11 \%$ ) to/from the north via Trafalgar Road North and within the Study Area,
- $42 \%(42 \%)$ to/from the south via Trafalgar Road North.

The site-generated trip volumes and trip assignment used in the analysis for the single detached homes and the townhouse units are illustrated in Figures 22 and 23.

For site-generated trips entering the elementary school during the A.M. Peak Hour and site-generate trips leaving the elementary school during the P.M. Peak Hour, the trip distribution and assignment is based on the future residential land use within an assumed attendance area for the elementary school. The attendance area was determined by using the attendance area for the existing Ross R. MacKay Public Elementary School, which is bounded by Erin-Garafraxa Townline to the north, Winston Churchill Boulevard to the east, Sideroad 17 to the south and Fourth Line to the west. Based on the location of the proposed Elementary School and the existing Ross R. MacKay Public Elementary School, it was assumed that the Attendance Area will be separated evenly and at the midpoint of the two locations.



## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.3 Trip Distribution and Assignment (Cont'd)

For site-generated trips leaving the elementary school during the A.M. Peak Hour and site-generate trips entering the elementary school during the P.M. Peak Hour, the assumed trip distribution and assignment is based on the results of the 2016 Transportation Tomorrow Survey as per residential land use within the attendance area and the future road network. The Transportation Tomorrow Survey database query that was used to determine the trip distribution is provided in Appendix F. In addition, at the time this Study was prepared, the access locations for the elementary school were not determined. Therefore, this Study assumes that the elementary school will be serviced by a full-moves access along the proposed Street ' A ' frontage.

For the 2026 horizon year, the assumed trip distribution and assignment will be as follows:

## A.M. Peak Hour

- $47 \%$ from within the Subject Subdivision,
- $3 \%$ from the east and outside the vicinity of the Study Area via Orangeville Street and Howe Street,
- $41 \%$ from the east via Howe Street, Church Street or Mill Street,
- $8 \%$ from the south via Upper Canada Drive or George Street,
- $1 \%$ from the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.


## Total 100\% inbound

- $4 \%$ to the north via Trafalgar Road North,
- $46 \%$ to the east via Wellington Road 22,
- $43 \%$ to the south via Trafalgar Road North,
- $7 \%$ to the south and within the Study Area via Trafalgar Road North.

Total $100 \%$ outbound

## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.3 Trip Distribution and Assignment (Cont'd)

## P.M. Peak Hour

- $4 \%$ from the north via Trafalgar Road North,
- $46 \%$ from the east via Wellington Road 22,
- $43 \%$ from the south via Trafalgar Road North,
- $7 \%$ from the south and within the Study Area via Trafalgar Road North.

Total $100 \%$ inbound

- $47 \%$ to within the Subject Subdivision,
- $3 \%$ to the east and outside the vicinity of the Study Area via Church Street and Howe Street,
- $41 \%$ to the east via Howe Street, Church Street or Mill Street,
- $8 \%$ to the south via Upper Canada Drive or George Street,
- $1 \%$ to the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

Total 100\% outbound

The site-generated trip volumes and trip assignment used in the analysis for the Elementary School during the 2026 horizon year are illustrated in Figures 24 and 25.



## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.3 Trip Distribution and Assignment (Cont'd)

For the 2031 horizon year, with the occupancy of the anticipated development immediately north of the Subject Subdivision that is owned by the applicant, the assumed trip distribution and assignment will be as follows:

## A.M. Peak Hour

- $51 \%$ from within the Subject Subdivision and the anticipated development owned by the applicant,
- $3 \%$ from the east and outside the vicinity of the Study Area via Orangeville Street and Howe Street,
- $38 \%$ from the east via Howe Street, Church Street or Mill Street,
- $7 \%$ from the south via Upper Canada Drive or George Street,
- $1 \%$ from the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.

Total $100 \%$ inbound

- $4 \%$ to the north via Trafalgar Road North,
- $46 \%$ to the east via Wellington Road 22,
- $43 \%$ to the south via Trafalgar Road North,
- $7 \%$ to the south and within the Study Area via Trafalgar Road North.


## Total $100 \%$ outbound

## 6. TRIP GENERATION AND DISTRIBUTION (CONT'D)

### 6.3 Trip Distribution and Assignment (Cont'd)

## P.M. Peak Hour

- $4 \%$ from the north via Trafalgar Road North,
- $46 \%$ from the east via Wellington Road 22,
- $43 \%$ from the south via Trafalgar Road North,
- $7 \%$ from the south and within the Study Area via Trafalgar Road North.


## Total 100\% inbound

- $51 \%$ to within the Subject Subdivision and the anticipated development owned by the applicant,
- $3 \%$ to the east and outside the vicinity of the Study Area via Church Street and Howe Street,
- $38 \%$ to the east via Howe Street, Church Street or Mill Street,
- $7 \%$ to the south via Upper Canada Drive or George Street,
- $1 \%$ to the west and outside the vicinity of the Study Area via Station Street/proposed West Collector Road or Side Road 27.


## Total 100\% outbound

The site-generated trip volumes and trip assignment used in the analysis for the Elementary School during the 2031 horizon year are illustrated in Figures 26 and 27.

For the 2026 horizon year, the site-generated trip volumes and trip assignment used in the analysis for the proposed Residential Subdivision are illustrated in Figures 28 and 29.

For the 2031 horizon year, the site-generated trip volumes and trip assignment used in the analysis for the proposed Residential Subdivision are illustrated in Figures 30 and 31.







## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.1 Future (2026) Total Traffic

The Future (2026) Total Traffic is based on the Future (2026) Total Background Traffic Volumes plus the Site-Generated Traffic Volumes for the Subject Property. The Future (2026) Total Traffic Volumes are provided in Figures 32 and 33.

### 7.2 Future (2026) Total Traffic Analysis

For the Future (2026) Total Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

For the Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North and Upper Canada Drive/Church Street at Trafalgar Road North intersections, the signal timing plans and the lane configurations used in the Future (2026) Total Background Traffic Analysis are used in the Future (2026) Total Traffic Analysis.

Proposed Street 'A'/Howe Street at Trafalgar Road North was analyzed as an unsignalized intersection with stop-controls at the eastbound and westbound approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

Proposed Street 'E' at Trafalgar Road North was analyzed as an un-signalized intersection with a stop-control at the eastbound approach. The lane configuration used in the analysis comprises a shared through-left turning lane at the northbound approach; a shared left-right turning lane at the eastbound approach; and a shared through-right turning lane at the southbound approach.

Proposed Street 'A' at proposed Street ' $G$ ' was analyzed as a single lane roundabout that is yield-controlled at all approaches. The lane configuration used in the analysis comprises a shared left-right turning lane at the northbound approach; a shared throughright turning lane at the eastbound approach; and a shared through-left turning lane at the westbound approach.



| Figure No: | 33 |  |
| :--- | :--- | :--- |
| Date: | July $\mathbf{2 0 2 0 2 2}$ |  |
| Prepared by: | B.W. | $\mathbf{N}$ |

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2 Future (2026) Total Traffic Analysis (Cont'd)

The results of the analysis are summarized in Table 13. The related calculations are provided in Appendix E.

Table 13: Future (2026) Total Traffic - Level of Service

| Intersection | Turning Lane /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | 95th Queue (m) <br> (m) | V/C | LOS | Delay ${ }^{1}$ | 95th Queue $(m)$ <br> (m) |
| Trafalgar Road North <br> at <br> Wellington Road 22 (Signalized) | Overall | 1.07 | D | 37.0 | n/a | 1.38 | E | 72.0 | n/a |
|  | EB Approach | 0.61 | C | 23.3 | 64.7 | 0.59 | C | 24.5 | 56.0 |
|  | WB Approach | 1.07 | F | 95.1 | 108.6 | 1.07 | F | 84.6 | 158.0 |
|  | NBL | 0.19 | B | 15.1 | 10.4 | 0.39 | B | 18.0 | 27.8 |
|  | NB TR | 0.45 | B | 15.0 | 47.9 | 1.10 | F | 84.3 | 230.1 |
|  | SBL | 0.72 | C | 30.8 | 68.1 | 1.38 | F | 250.8 | 48.4 |
|  | SB TR | 0.77 | C | 26.0 | 107.6 | 0.45 | B | 16.2 | 58.8 |
| Trafalgar Road North <br> at Howe Street/ <br> Proposed Street 'A' <br> (Un-signalized) | Overall | 0.29 | A | 5.0 | n/a | 0.13 | A | 2.5 | n/a |
|  | EB Approach | 0.29 | B | 11.7 | 9.5 | 0.13 | B | 12.9 | 3.6 |
|  | WB Approach | 0.08 | B | 14.7 | 2.1 | 0.02 | D | 27.0 | 0.6 |
|  | NB Approach | 0.06 | A | 2.8 | 1.5 | 0.09 | A | 2.4 | 2.3 |
|  | SB Approach | 0.00 | A | 0.2 | 0.1 | 0.01 | A | 0.2 | 0.2 |
| Trafalgar Road North <br> at <br> George Street/ <br> Mill Street <br> (Un-signalized) | Overall | 0.23 | A | 1.8 | n/a | 0.50 | A | 6.2 | n/a |
|  | EB Approach | 0.05 | C | 17.7 | 1.3 | 0.41 | D | 32.0 | 15.1 |
|  | WB Approach | 0.23 | C | 19.2 | 7.0 | 0.50 | F | 52.3 | 19.1 |
|  | NB Approach | 0.01 | A | 0.5 | 0.3 | 0.07 | A | 1.7 | 1.7 |
|  | SB Approach | 0.00 | A | 0.0 | 0.0 | 0.03 | A | 0.8 | 0.6 |
| Trafalgar Road North at | Overall | 0.12 | A | 1.3 | n/a | 0.09 | A | 0.8 | n/a |
|  | EB Approach | 0.11 | C | 17.5 | 2.9 | 0.00 | B | 10.4 | 0.1 |
|  | WB Approach | 0.12 | C | 15.3 | 3.3 | 0.09 | C | 23.2 | 2.3 |
| Upper Canada Drive/ <br> Church Street <br> (Un-signalized) | NB Approach | 0.00 | A | 0.1 | 0.1 | 0.02 | A | 0.4 | 0.4 |
|  | SB Approach | 0.00 | A | 0.0 | 0.0 | 0.01 | A | 0.4 | 0.3 |

Note 1: Delays are measured in seconds per vehicle.
Note 2: Signalized intersections are based on existing signal timing plans.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT’D)

### 7.2 Future (2026) Total Traffic Analysis (Cont'd)

Table 13: Future (2026) Total Traffic - Level of Service

| Intersection | Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | 95th <br> Queue <br> (m) | V/C | LOS | Delay ${ }^{1}$ | 95th <br> Queue <br> (m) |
| Trafalgar Road North <br> at <br> Proposed Street 'E' <br> (Un-signalized) | Overall | 0.27 | A | 2.0 | n/a | 0.22 | A | 2.2 | n/a |
|  | EB Approach | 0.19 | B | 13.0 | 5.7 | 0.14 | B | 13.0 | 3.8 |
|  | NB Approach | 0.03 | A | 1.1 | 0.7 | 0.09 | A | 2.3 | 2.4 |
|  | SB Approach | 0.27 | A | 0.0 | 0.0 | 0.22 | A | 0.0 | 0.0 |
| Street 'A'atProposed Street 'G'(Roundabout) | Overall | 0.20 | A | 4.8 | n/a | 0.11 | A | 4.0 | n/a |
|  | EB Approach | 0.20 | A | 5.1 | 7.0 | 0.06 | A | 3.8 | 0.0 |
|  | WB Approach | 0.11 | A | 4.3 | 0.0 | 0.11 | A | 4.2 | 0.0 |
|  | NB Approach | 0.03 | A | 4.4 | 0.0 | 0.00 | A | 3.4 | 0.0 |

Note 1: Delays are measured in seconds per vehicle.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2 Future (2026) Total Traffic Analysis (Cont'd)

## Trafalgar Road North at Wellington Road 22

The analysis of the Future (2026) Total Traffic Conditions indicates that the signalized intersection will begin to operate at a Level of Service "D" during the A.M. Peak Hour and at a Level of Service "E" during the P.M. Peak Hour. With the inclusion of sitegenerated trips, impacts to the intersection are low during the A.M. Peak Hour and high during P.M. Peak Hour.

The westbound approach will continue to operate at a Level of Service " $F$ " with a volume over capacity ratio that is greater than 1.0 during the A.M. Peak Hour and will begin to operate at a Level of Service " $F$ " with a volume over capacity ratio that is greater than 1.0 during the P.M. Peak Hour.

During the P.M. Peak Hour, the shared through-right turning lane at the northbound approach and the left turning lane at the southbound approach will begin to operate at a Level of Service " $F$ " with a volume over capacity ratio that is greater than 1.0. In addition, during the A.M. and P.M. Peak Hours, the queue lengths at the southbound left turning lane may begin to result in a spillback of vehicles into the adjacent lane.

## Trafalgar Road North at Howe Street/proposed Street 'A'

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are moderate during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "B" or better during the A.M. Peak Hour and at a Level of Service "D" or better during the P.M. Peak Hour.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2 Future (2026) Total Traffic Analysis (Cont'd)

## Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are low during the A.M. Peak Hour and moderate during P.M. Peak Hour.

During the P.M. Peak Hour, the westbound approach will begin to operate at a Level of Service " $F$ " with an average delay of 52.3 seconds per vehicle. All of the other turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

## Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated trips, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and continue to operate at a Level of Service "C" or better during the P.M. Peak Hour.

## Trafalgar Road North at proposed Street 'E'

The analysis of the Future (2026) Total Traffic Conditions indicates that the un-signalized intersection will operate at a Level of Service "A" during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements will operate at a Level of Service "B" or better.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2 Future (2026) Total Traffic Analysis (Cont'd)

Proposed Street 'A' at proposed Street 'B'/proposed Street ' $\mathbf{G}$ '

The analysis of the Future (2026) Total Traffic Conditions indicates that all of the approaches at the roundabout will operate at a Level of Service "A" during the A.M. and P.M. Peak Hours.

### 7.2. $\quad$ Future (2026) Total Traffic Analysis - Right-Turn Lane Warrant Analysis

For the southbound right turning movements at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a right-turn lane warrant analysis was conducted using the principles provided in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways ${ }^{7}$. Based on the procedure to the right-turn lane warrant analysis, a right-turning lane should be considered when traffic volumes are 60 vehicles per hour or higher. With the southbound right turning movements at the concerned intersections operating with 12 vehicles per vehicle or less, right turning lanes are not warranted.

### 7.2.2 Future (2026) Total Traffic Analysis - Left-Turn Lane Warrant Analysis

For the northbound left turning movement at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a left-turn lane warrant analysis was undertaken. The analysis followed the procedure specified in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways.

At approximately 200 metres north of the proposed Residential Subdivision, travelling in the southbound direction, the posted speed limit on Trafalgar Road North changes from $60 \mathrm{~km} / \mathrm{h}$ to $40 \mathrm{~km} / \mathrm{h}$. Therefore, a design speed of $70 \mathrm{~km} / \mathrm{h}$ was assumed.

[^6]
## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2.2 Future (2026) Total Traffic Analysis - Left-Turn Lane Warrant Analysis

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street ' $E$ ' intersections, the analysis determined that a left-turning lane at the northbound approach is warranted during the P.M. Peak Hour.

For the Trafalgar Road North at Howe Street/proposed Street 'A' intersection, in order to match the construction of a northbound left-turning lane, a southbound left-turning lane will be recommended to create a balanced intersection.

The results of the analysis are provided in Figures 34 and 35.

### 7.2.3 Future (2026) Total Traffic Analysis - Signal Warrant Analysis

The Future (2026) Total Traffic Analysis indicates that the westbound approach for the Trafalgar Road North at George Street/Mill Street intersection will operate at a Level of Service "F" with an average delay of 52.3 seconds per vehicle during the P.M. Peak Hour.

Therefore, a signal warrant analysis was undertaken since the intersection is currently unsignalized. The analysis followed the procedures specified in Book 12 Justification 7 in the Ontario Traffic Manual and is provided in Appendix G ${ }^{8}$.

The signal warrant analysis indicates that the Trafalgar Road North at George Street/Mill Street intersection does not warrant traffic signals. Although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, an average delay of 52.3 seconds per vehicle is acceptable during the Weekday Peak Periods.

[^7]

A.M. PEAK HOUR

P.M. PEAK HOUR

TRAFFIC IMPACT STUDY PROPOSED RESIDENTIAL SUBDIVISION HILLSBURGH HEIGHTS INC. TOWN OF ERIN


## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2.4 Future (2026) Total Traffic Analysis - Recommended Improvements

For the Trafalgar Road North at Wellington Road 22 intersection, there are critical turning movements during the A.M. and P.M. Peak Hours.

In order to address the critical turning movements, the following improvements are recommended.

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a left turning lane at the eastbound approach with 35 metres of storage,
- Include a left turning lane at the westbound approach with 45 metres of storage,
- Extend the storage of the southbound left turning lane to 65 metres.

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street ' $E$ ' intersections, improvements were made based on the results of the left-turn lane warrant analysis. The left-turn lane warrant analysis indicates that a northbound left turning lane is warranted during the P.M. Peak Hour. For the Trafalgar Road North at Howe Street/proposed Street 'A' intersection, to avoid issues with safety due to the construction of a northbound left-turning lane, a southbound left-turning lane will be constructed.

The following improvements are recommended for the concerned intersections.

## Proposed Street 'A'/Howe Street at Trafalgar Road North

- Include a left turning lane at the northbound and southbound approaches with 15 metres of storage.


## Proposed Street 'E’ at Trafalgar Road North

- Include a left turning lane at the northbound approach with 15 metres of storage.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 52.3 seconds per vehicle, it is considered acceptable for Peak Period conditions.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2.4 Future (2026) Total Traffic Analysis - Recommended Improvements (Cont'd)

The traffic conditions with the recommended improvements are summarized in Table 14. The related calculations are provided in Appendix E.

Table 14: Future (2026) Total Traffic - Level of Service - with Improvements

| Intersection | Turning Lane /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | 95th Queue $(m)$ <br> (m) | V/C | LOS | Delay ${ }^{1}$ | $\begin{aligned} & \text { 95th } \\ & \text { Queue } \\ & (\mathrm{m}) \\ & \hline \end{aligned}$ |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 0.89 | C | 28.1 | n/a | 1.00 | D | 44.3 | n/a |
|  | EBL | 0.14 | C | 23.1 | 10.0 | 0.70 | E | 75.4 | 31.7 |
|  | EB TR | 0.89 | D | 50.1 | 80.0 | 0.51 | C | 30.7 | 52.7 |
|  | WBL | 0.72 | C | 34.1 | 36.2 | 0.54 | D | 39.4 | 40.8 |
|  | WB TR | 0.42 | B | 15.5 | 28.5 | 0.99 | E | 71.3 | 135.7 |
|  | NBL | 0.19 | B | 14.0 | 9.5 | 0.31 | B | 15.8 | 27.1 |
|  | NB TR | 0.47 | B | 13.8 | 42.4 | 1.00 | D | 52.8 | 245.9 |
|  | SBL | 0.73 | C | 29.8 | 60.9 | 0.70 | C | 32.0 | 21.7 |
|  | SB TR | 0.80 | C | 26.7 | 109.7 | 0.35 | B | 10.2 | 48.1 |
| Trafalgar Road North <br> at <br> Howe Street/ <br> Proposed Street 'A' <br> (Un-signalized) | Overall | 0.29 | A | 4.9 | n/a | 0.27 | A | 2.0 | n/a |
|  | EB Approach | 0.29 | B | 11.7 | 9.5 | 0.13 | B | 12.9 | 3.6 |
|  | WB Approach | 0.08 | B | 14.7 | 2.1 | 0.02 | D | 26.8 | 0.6 |
|  | NBL | 0.06 | A | 7.9 | 1.5 | 0.09 | A | 8.2 | 2.3 |
|  | NB TR | 0.11 | A | 0.0 | 0.0 | 0.27 | A | 0.0 | 0.0 |
|  | SBL | 0.00 | A | 7.6 | 0.1 | 0.01 | A | 8.3 | 0.2 |
|  | SB TR | 0.14 | A | 0.0 | 0.0 | 0.19 | A | 0.0 | 0.0 |
| Trafalgar Road North <br> at Proposed Street 'E' (Un-signalized) | Overall | 0.27 | A | 1.9 | n/a | 0.33 | A | 1.6 | n/a |
|  | EB Approach | 0.19 | B | 13.0 | 5.7 | 0.14 | B | 13.0 | 3.8 |
|  | NBL | 0.03 | A | 8.4 | 0.7 | 0.09 | A | 8.3 | 2.4 |
|  | NBT | 0.16 | A | 0.0 | 0.0 | 0.33 | A | 0.0 | 0.0 |
|  | SB Approach | 0.27 | A | 0.0 | 0.0 | 0.22 | A | 0.0 | 0.0 |

Note 1: Delays are measured in seconds per vehicle.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.2.4 Future (2026) Total Traffic Analysis - Recommended Improvements (Cont'd)

With the recommended improvements, the intersection of Trafalgar Road North at Wellington Road 22 will operate at a Level of Service C during the A.M. Peak Hour and a Level of Service "D" during the P.M. Peak Hour. All of the turning movements will operate at a Level of Service "D" or better during the A.M. Peak Hour and a Level of Service "E" or better during the P.M. Peak Hour.

The Trafalgar Road North at Howe Street/proposed Street 'A' intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours. All of the turning movements will operate at a Level of Service "B" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

The Trafalgar Road North at proposed Street ' $E$ ' intersection operates at a Level of Service "A" during the A.M. and P.M. Peak Hours. All of the turning movements will operate at a Level of Service "B" or better during the A.M. and P.M. Peak Hours.

### 7.3 Future (2031) Total Traffic

The Future (2031) Total Traffic is based on the Future (2031) Total Background Traffic Volumes plus the Site-Generated Traffic Volumes from the Subject Property and the Site-Generated Traffic Volumes from the anticipated development that is immediately north of the Subject Property and that is owned by the applicant. (Figures 14 and 15) The Future (2031) Total Traffic Volumes are provided in Figures 36 and 37.


| Figure No: | $\mathbf{3 6}$ |  |
| :--- | :--- | :--- |
| Date: | July $\mathbf{2 1 2 0 2 2}$ |  |
| Prepared by: | B.W. | $\mathbf{N}$ |



| Figure No: | 37 |  |
| :--- | :--- | :--- |
| Date: | July $\mathbf{2 1 2 0 2 2}$ |  |
| Prepared by: | B.W. | $\mathbf{N}$ |

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4 Future (2031) Total Traffic Analysis

For the Future (2031) Total Traffic Volumes, the LOS was analyzed using SYNCHRO 9.0 software.

For the Trafalgar Road North at Wellington Road 22, George Street/Mill Street at Trafalgar Road North, Upper Canada Drive/Church Street at Trafalgar Road North, Howe Street/proposed Street 'A' at Trafalgar Road North and proposed Street 'E' at Trafalgar Road North intersections, the signal timing plans and the lane configurations used in the Future (2026) Total Traffic Analysis are used in the Future (2031) Total Traffic Analysis.

The proposed Street 'A' at proposed Street 'G'/future Local Road intersection was analyzed as a single lane roundabout that is yield-controlled at all approaches. The lane configuration used in the analysis comprises a shared left-through-right turning lane at all approaches.

The results of the analysis are summarized in Table 15. The related calculations are provided in Appendix E.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4 Future (2031) Total Traffic Analysis (Cont'd)

Table 15: Future (2031) Total Traffic - Level of Service

| Intersection | Turning Lane <br> /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \\ \hline \hline \end{gathered}$ | V/C | LOS | Delay ${ }^{1}$ | 95th <br> Queue <br> (m) |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 1.16 | D | 46.4 | n/a | 1.59 | F | 99.0 | n/a |
|  | EB Approach | 0.65 | C | 24.6 | 69.6 | 0.66 | C | 27.7 | 62.2 |
|  | WB Approach | 1.16 | F | 126.8 | 118.8 | 1.16 | F | 117.3 | 175.8 |
|  | NBL | 0.23 | B | 16.5 | 11.1 | 0.43 | B | 19.5 | 30.0 |
|  | NB TR | 0.49 | B | 15.8 | 53.3 | 1.19 | F | 119.2 | 256.6 |
|  | SBL | 0.85 | D | 44.1 | 83.2 | 1.59 | F | 333.1 | 54.8 |
|  | SB TR | 0.84 | C | 30.3 | 136.2 | 0.49 | B | 16.9 | 65.4 |
| Trafalgar Road North <br> at <br> Howe Street/ <br> Proposed Street 'A' <br> (Un-signalized) | Overall | 0.37 | A | 5.8 | n/a | 0.23 | A | 4.0 | n/a |
|  | EB Approach | 0.37 | B | 13.3 | 13.7 | 0.23 | C | 16.6 | 7.0 |
|  | WB Approach | 0.14 | C | 17.6 | 3.7 | 0.14 | E | 37.0 | 3.9 |
|  | NB Approach | 0.08 | A | 2.9 | 2.0 | 0.13 | A | 3.2 | 3.7 |
|  | SB Approach | 0.01 | A | 0.4 | 0.2 | 0.02 | A | 0.7 | 0.6 |
| Trafalgar Road North <br> at <br> George Street/ <br> Mill Street <br> (Un-signalized) | Overall | 0.26 | A | 1.8 | n/a | 0.62 | A | 7.6 | n/a |
|  | EB Approach | 0.05 | C | 18.9 | 1.3 | 0.49 | E | 41.9 | 19.4 |
|  | WB Approach | 0.26 | C | 22.3 | 8.2 | 0.62 | F | 75.0 | 24.9 |
|  | NB Approach | 0.01 | A | 0.5 | 0.4 | 0.07 | A | 1.7 | 1.8 |
|  | SB Approach | 0.00 | A | 0.0 | 0.0 | 0.03 | A | 0.7 | 0.6 |
| Trafalgar Road North at | Overall | 0.17 | A | 1.6 | n/a | 0.12 | A | 1.1 | n/a |
|  | EB Approach | 0.17 | C | 22.2 | 4.7 | 0.04 | D | 25.6 | 1.1 |
| Upper Canada Drive/ <br> Church Street <br> (Un-signalized) | WB Approach | 0.15 | C | 16.7 | 4.0 | 0.12 | D | 25.9 | 3.2 |
|  | NB Approach | 0.00 | A | 0.1 | 0.1 | 0.02 | A | 0.4 | 0.4 |
|  | SB Approach | 0.00 | A | 0.1 | 0.1 | 0.02 | A | 0.5 | 0.4 |

Note 1: Delays are measured in seconds per vehicle.
Note 2: Signalized intersections are based on existing signal timing plans.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT’D)

### 7.4 Future (2031) Total Traffic Analysis (Cont'd)

Table 15: Future (2031) Total Traffic - Level of Service

| Intersection | Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | 95th <br> Queue <br> (m) | V/C | LOS | Delay ${ }^{1}$ | 95th <br> Queue <br> (m) |
| Trafalgar Road North <br> at <br> Proposed Street 'E' <br> (Un-signalized) | Overall | 0.31 | A | 1.9 | n/a | 0.26 | A | 2.2 | n/a |
|  | EB Approach | 0.22 | B | 14.1 | 6.5 | 0.16 | B | 14.4 | 4.5 |
|  | NB Approach | 0.03 | A | 1.1 | 0.8 | 0.10 | A | 2.3 | 2.5 |
|  | SB Approach | 0.31 | A | 0.0 | 0.0 | 0.26 | A | 0.0 | 0.0 |
| Proposed Street 'A' <br> at <br> Proposed Street 'G'/ <br> future Local Road <br> (Roundabout) | Overall | 0.28 | A | 5.5 | n/a | 0.19 | A | 4.9 | n/a |
|  | EB Approach | 0.28 | A | 6.1 | 7.0 | 0.14 | A | 4.7 | 0.0 |
|  | WB Approach | 0.14 | A | 4.9 | 7.0 | 0.19 | A | 5.4 | 7.0 |
|  | NB Approach | 0.08 | A | 5.3 | 0.0 | 0.03 | A | 4.1 | 0.0 |
|  | SB Approach | 0.06 | A | 4.4 | 0.0 | 0.08 | A | 4.6 | 0.0 |

Note 1: Delays are measured in seconds per vehicle.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4 Future (2031) Total Traffic Analysis (Cont'd)

## Trafalgar Road North at Wellington Road 22

The analysis of the Future (2031) Total Traffic Conditions indicates that the signalized intersection will continue to operate at a Level of Service "D" during the A.M. Peak Hour and will begin to operate at a Level of Service "F" during the P.M. Peak Hour. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. Peak Hour and high during P.M. Peak Hour.

The westbound approach will continue to operate at a Level of Service "F' with a volume over capacity ratio that is greater than 1.0 during the A.M. Peak Hour and will begin to operate at a Level of Service " F ' with a volume over capacity ratio that is greater than 1.0 during the P.M. Peak Hour.

During the P.M. Peak Hour, the shared through-right turning lane at the northbound approach will continue to operate with a volume over capacity ratio that is greater than 1.0 and will begin to operate at a Level of Service "F".

The left turning lane at the southbound approach will continue to operate at a Level of Service "F" and will begin to operate with a volume over capacity ratio that is greater than 1.0. The queue lengths at the southbound left turning lane may begin to result in a spill back of vehicles into the adjacent lane during the A.M. and P.M. Peak Hours.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4 Future (2031) Total Traffic Analysis (Cont'd)

## Trafalgar Road North at Howe Street/proposed Street 'A'

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are high during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "E" or better during the P.M. Peak Hour.

## Trafalgar Road North at George Street/Mill Street

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. Peak Hour and moderate during P.M. Peak Hour.

During the P.M. Peak Hour, the westbound approach will begin to operate at a Level of Service " $F$ " with an average delay of 75.0 seconds per vehicle. All of the other turning movements will continue to operate at a Level of Service "C" or better during the A.M. Peak Hour and will begin to operate at a Level of Service "E" or better during the P.M. Peak Hour.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4 Future (2031) Total Traffic Analysis (Cont'd)

## Trafalgar Road North at Upper Canada Drive/Church Street

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the inclusion of site-generated traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

All of the turning movements will begin to operate at a Level of Service "C" or better during the A.M. Peak Hour and a Level of Service "D" or better during the P.M. Peak Hour.

## Trafalgar Road North at proposed Street 'E'

The analysis of the Future (2031) Total Traffic Conditions indicates that the un-signalized intersection will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection are minimal during the A.M. and P.M. Peak Hours.

During the A.M. and P.M. Peak Hours, all of the turning movements will continue to operate at a Level of Service "B" or better.

## Proposed Street 'A' at proposed Street 'G'/future Local Road

The analysis of the Future (2031) Total Traffic Conditions indicates that all of the approaches at the roundabout will continue to operate at a Level of Service "A" during the A.M. and P.M. Peak Hours. With the growth in background traffic, impacts to the intersection are low during the A.M. and P.M. Peak Hours.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4.1 Future (2031) Total Traffic Analysis - Right-Turn Lane Warrant Analysis

For the southbound right turning movements at the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street 'E' intersections, a right-turn lane warrant analysis was conducted using the principles provided in the Ministry of Transportation Ontario's Geometric Design Standards for Ontario Highways. Based on the procedure to the right-turn lane warrant analysis, a right-turning lane should be considered when traffic volumes are 60 vehicles per hour or higher. With the southbound right turning movements at the concerned intersections operating with 13 vehicles per vehicle or less, right turning lanes are not warranted.

### 7.4.2 Future (2031) Total Traffic Analysis - Signal Warrant Analysis

The Future (2031) Total Traffic Analysis indicates that the westbound approach for the Trafalgar Road North at George Street/Mill Street intersection will operate at a Level of Service " $F$ " with an average delay of 75.0 seconds per vehicle during the P.M. Peak Hour.

Therefore, a signal warrant analysis was undertaken since the intersection is currently unsignalized. The analysis followed the procedures specified in Book 12 Justification 7 in the Ontario Traffic Manual and is provided in Appendix G.

The signal warrant analysis indicates that the Trafalgar Road North at George Street/Mill Street intersection does not warrant traffic signals. Although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, an average delay of 75.0 seconds per vehicle is acceptable during the Weekday Peak Periods.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements

For the Trafalgar Road North at Wellington Road 22 intersection, there are critical turning movements as a result of background traffic growth from the 2026 horizon year.

In order to address the critical turning movements, the following improvements are recommended beyond the recommendations made for the 2026 horizon:

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a right turning lane at the northbound approach with 20 metres of storage,
- Extend the storage for the left turning lane at the southbound approach to 70 metres,
- Include a right turning lane at the westbound approach with 35 metres of storage.

The traffic conditions with the recommended improvements are summarized in Table 16. The related calculations are provided in Appendix E.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements (Cont'd)

Table 16: Future (2031) Total Traffic - Level of Service - with Improvements

| Intersection | Turning Lane /Approach | A.M. Peak Hour |  |  |  | P.M. Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C | LOS | Delay ${ }^{1}$ | $\begin{gathered} \text { 95th } \\ \text { Queue } \\ (\mathrm{m}) \\ \hline \hline \end{gathered}$ | V/C | LOS | Delay ${ }^{1}$ | 95th Queue $(\mathrm{m})$ <br> (m) |
| Trafalgar Road North at Wellington Road 22 (Signalized) | Overall | 0.86 | C | 28.3 | n/a | 0.82 | B | 19.7 | n/a |
|  | EBL | 0.15 | C | 23.9 | 11.4 | 0.24 | C | 20.9 | 15.3 |
|  | EB TR | 0.86 | D | 45.6 | 79.2 | 0.54 | C | 21.3 | 36.9 |
|  | WBL | 0.79 | D | 40.6 | 43.5 | 0.54 | C | 28.5 | 29.9 |
|  | WBT | 0.18 | B | 17.6 | 20.0 | 0.51 | C | 24.3 | 39.9 |
|  | WBR | 0.17 | A | 5.4 | 8.1 | 0.57 | B | 15.0 | 32.3 |
|  | NBL | 0.22 | B | 15.7 | 10.6 | 0.35 | B | 12.4 | 21.3 |
|  | NBT | 0.35 | B | 14.6 | 36.1 | 0.82 | C | 22.8 | 135.6 |
|  | NBR | 0.16 | A | 3.1 | 7.1 | 0.26 | A | 5.1 | 16.5 |
|  | SBL | 0.72 | C | 28.0 | 68.1 | 0.78 | D | 47.3 | 44.6 |
|  | SB TR | 0.85 | C | 31.0 | 130.0 | 0.45 | B | 11.4 | 47.8 |
| Trafalgar Road North <br> at <br> Howe Street/ <br> Proposed Street 'A' <br> (Un-signalized) | Overall | 0.37 | A | 5.6 | n/a | 0.31 | A | 3.2 | n/a |
|  | EB Approach | 0.37 | B | 13.3 | 13.7 | 0.23 | C | 16.6 | 7.0 |
|  | WB Approach | 0.14 | C | 17.6 | 3.7 | 0.14 | E | 36.9 | 3.9 |
|  | NBL | 0.08 | A | 8.0 | 2.0 | 0.13 | A | 8.5 | 3.7 |
|  | NB TR | 0.13 | A | 0.0 | 0.0 | 0.31 | A | 0.0 | 0.0 |
|  | SBL | 0.01 | A | 7.7 | 0.2 | 0.02 | A | 8.5 | 0.6 |
|  | SB TR | 0.16 | A | 0.0 | 0.0 | 0.22 | A | 0.0 | 0.0 |
| Trafalgar Road North at Proposed Street 'E' (Un-signalized) | Overall | 0.31 | A | 1.8 | n/a | 0.39 | A | 1.5 | n/a |
|  | EB Approach | 0.22 | B | 14.1 | 6.5 | 0.16 | B | 14.4 | 4.5 |
|  | NBL | 0.03 | A | 8.6 | 0.8 | 0.10 | A | 8.6 | 2.5 |
|  | NBT | 0.18 | A | 0.0 | 0.0 | 0.39 | A | 0.0 | 0.0 |
|  | SB Approach | 0.31 | A | 0.0 | 0.0 | 0.26 | A | 0.0 | 0.0 |

Note 1: Delays are measured in seconds per vehicle.

## 7. FUTURE TOTAL TRAFFIC CONDITIONS (CONT'D)

### 7.4.3 Future (2031) Total Traffic Analysis - Recommended Improvements (Cont'd)

For Trafalgar Road North at Wellington Road 22, the intersection will operate at a Level of Service "C" during the A.M. Peak Hour and a Level of Service "B" during the P.M. Peak Hour. All of the turning movements will operate at a Level of Service "D" or better during the A.M. and P.M. Peak Hours.

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street ' $E$ ' intersections, with the improvements recommended for the 2026 horizon year, all of the turning movements will continue to operate at an acceptable Levels of Service during the A.M. and P.M. Peak Hours.

For the Trafalgar Road North at George Street/Mill Street intersection, although the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour, with an average delay of 75.0 seconds per vehicle, it is considered acceptable for Peak Period conditions.

## 8. REVIEW OF ROUNDABOUT - SWEPT PATH ANALYSIS

Using the preliminary design of the roundabout intersection at proposed Street 'A' at proposed Street ' $G$ '/future Local Road, the geometry of the roundabout was analyzed for fire emergency vehicles. Vehicle swept paths have been analyzed in AutoTURN software and are provided in Figure RA-1. The vehicle swept paths demonstrate that the proposed geometry and right of way is acceptable.


## 9. SIGHT DISTANCE ANALYSIS

For the Trafalgar Road North at Howe Street/proposed Street 'A' and Trafalgar Road North at proposed Street ' E ' intersections, the sight distances for vehicles exiting from the Subject Subdivision were reviewed. To evaluate the sight distances for the proposed accesses, the principles were taken from the Transportation Association of Canada's Geometric Design Guide for Canadian Roads ${ }^{9}$.

To determine the sight distances from the vertical plane, CANDEVCON LIMITED conducted a site visit on April 26, 2022. Pictures taken during the site visit that illustrate the site distances from the proposed accesses are provided in Appendix H. For the sight distances from the horizontal plane, issues are minimal since the alignment of Trafalgar Road North is relatively straight. To demonstrate that the sight distances provided from the horizontal plane will exceed the sight distances required, the sight distances for vehicles exiting Street ' A ' are provided in Figure 38.

At approximately 200 metres north of the proposed Residential Subdivision, travelling in the southbound direction, the posted speed limit on Trafalgar Road North changes from $60 \mathrm{~km} / \mathrm{h}$ to $40 \mathrm{~km} / \mathrm{h}$. Therefore, for vehicles leaving the Subject Subdivision by making a right-turn, the design speed for vehicles on Trafalgar Road North is $70 \mathrm{~km} / \mathrm{h}$, conservatively. For vehicles leaving the Subject Subdivision by making a left-turn or a through movement, the design speed for vehicles on Trafalgar Road North is $60 \mathrm{~km} / \mathrm{h}$.

Based on the results of the analysis, the sight distances provided exceed the sight distances required. The findings of the sight distance analysis are provided in Table 17.

[^8]

## 9. SIGHT DISTANCE ANALYSIS (CONT'D)

Table 17: The Required and Provided Sight Distances

| Departing From | Turning Movement | Sight Distance Required | Sight Distance Provided |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Vertical | Horizontal |
| Street 'A' | EBL | $133 \mathrm{~m}($ Note 1) | 220 m | $>300 \mathrm{~m}$ |
|  | EBT | $117 \mathrm{~m}($ Note 1) | 220 m | $>300 \mathrm{~m}$ |
|  | EBR | $126 \mathrm{~m}($ Note 2) | 240 m | $>300 \mathrm{~m}$ |
| Street 'E' | EBL | $133 \mathrm{~m}($ Note 1) | $>300 \mathrm{~m}$ | $>300 \mathrm{~m}$ |
|  | EBR | $126 \mathrm{~m}($ Note 2) | 220 m | $>300 \mathrm{~m}$ |

Note 1: The design speed for Trafalgar Road North is $60 \mathrm{~km} / \mathrm{h}$.
Note 2: The design speed for Trafalgar Road North is $70 \mathrm{~km} / \mathrm{h}$.

## 10. SUMMARY

The proposed Residential Subdivision is expected to generate a total of 527 trips during the A.M. Peak Hour (218 inbound trips and 309 outbound trips) and 368 trips during the P.M. Peak Hour (220 inbound trips and 148 outbound trips). During the A.M. and P.M. Peak Hours, traffic impacts from the trips generated by the proposed Residential Subdivision are moderate.

Vehicle access to the proposed Residential Subdivision from Trafalgar Road North is provided via the proposed Street ' $E$ ' that is located at the southeast corner of the Subject Property and the proposed Street 'A' that aligns with Howe Street to form a four legged intersection. Between the 2026 and 2031 horizon years, it is anticipated that the future Residential Subdivision owned by the applicant that is immediately north of the Subject Subdivision will be fully built-out and occupied. At the time this report was prepared, details to the future Residential Subdivision are preliminary. However, for the roundabout within the Subject Subdivision that is immediately west of the proposed Street 'A'/Howe Street at Trafalgar Road North intersection, the anticipated development will provide a local road that aligns with Street ' $G$ ' at Street ' $A$ ' to form the north leg.

The lands that are immediately west of the Subject Subdivision that are owned by the applicant lie outside of the Hillsburgh Urban Boundary and are designated under Agricultural and Greenland. The future development potential for these lands, since they lie outside the Urban Boundary, will only be recognized once the lands are brought into the Urban Boundary, which could take up to 30 years. Therefore, the potential development will be built-out and occupied after the 2031 horizon year.

The following recommendations should be considered for the full build-out 2026 horizon year:

## Trafalgar Road North at Wellington Road 22

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a left turning lane at the eastbound approach with 35 metres of storage,
- Include a left turning lane at the westbound approach with 45 metres of storage,
- Extend the storage of the southbound left turning lane to 65 metres.


## 10. SUMMARY (CONT'D)

## Proposed Street 'A'/Howe Street at Trafalgar Road North

- An un-signalized intersection with stop-controls at the eastbound and westbound approaches,
- A left turning lane with 15 metres of storage and a shared through-right turning lane at the northbound and southbound approaches,
- A shared left-through-right turning lane at the eastbound and westbound approaches.


## Proposed Street 'E' at Trafalgar Road North

- An un-signalized intersection with a stop-control at the eastbound approach,
- A left turning lane with 15 metres of storage and a through lane at the northbound approach,
- A shared left-right turning lane at the eastbound approach,
- A shared through-right turning lane at the southbound approach.


## Proposed Street 'A' at proposed Street 'G'

- A single lane roundabout intersection that is yield-controlled at all approaches.

With the exception of the Trafalgar Road North at George Street/Mill Street intersection, all of intersections will have turning movements that operate at a Level of Service "E" or better.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 52.3 seconds per vehicle, it is considered acceptable for Peak Period conditions.

## 10. SUMMARY (CONT'D)

To address the growth in background traffic, the following recommendations should be considered for the five (5) year post build-out 2031 horizon year (beyond the improvements as recommended for the 2026 horizon):

## Trafalgar Road North at Wellington Road 22

- Modify the signal timing plans for the Weekday A.M. and P.M. Peak Period,
- Include a right turning lane at the northbound approach with 20 metres of storage,
- Extend the storage for the left turning lane at the southbound approach to 70 metres,
- Include a right turning lane at the westbound approach with 35 metres of storage.


## Proposed Street 'A' at proposed Street ' $G$ '/future Local Road

- A north leg to the intersection that is a part of the anticipated development (lands owned by applicant) that is immediately north will be constructed.

With the exception of the Trafalgar Road North at George Street/Mill Street intersection, all of intersections will have turning movements that operate at a Level of Service "E" or better.

For the Trafalgar Road North at George Street/Mill Street intersection, the westbound approach operates at a Level of Service "F" during the P.M. Peak Hour. However, with an average delay of 75.0 seconds per vehicle, it is considered acceptable for Peak Period conditions.

In addition, the preliminary design of the roundabout at proposed Street 'A' at proposed Street 'G'/future Local Road demonstrates that adequate circulation will be provided for fire emergency vehicles.

## 10. SUMMARY (CONT'D)

Based on the analysis outlined in the Study, with the implementation of the recommendations as outlined, all the key intersections will operate at acceptable levels of service during the Weekday A.M. and P.M. Peak Hours under the 2026 and 2031 horizon years.

This Report was prepared by:

## CANDEVCON LIMITED



Brian Wong, P. Eng.
Intermediate Transportation Engineer


David Lee, P. Eng.
Project Manager

## APPENDIX A

## TERMS OF REFERENCE

## Traffic Impact Study - Terms of Reference

a) Assemble, review and confirm background data (i.e. traffic volume/flow on the adjacent road network during weekday peak hours) available from official sources, existing road geometry and access locations.
b) Gather/conduct turning movement counts (if necessary) for the Howe Street at Trafalgar Road North intersection during the Weekday AM and Weekday PM Peak Hours. (We may need to use historical traffic counts given the current covid-19 situation.)
c) Establish existing traffic patterns and historic travel growth rates for the study area.
d) Consult with the County of Wellington and the Town of Erin to confirm data as required (i.e. growth trends, other proposed development timing etc.), issues/developments to be addressed and any anticipated future road improvements.
e) Confirm with the County of Wellington and the Town of Erin for any future planned road improvements in the area.
f) Assess total future trips generated by the proposed Residential Subdivision during the Weekday AM and Weekday PM Peak Hours.
g) Develop the trip distribution and traffic assignment for the proposed Residential Subdivision during the Weekday AM and Weekday PM Peak Hours.
h) Establish the five (5) year time horizon post full build-out of the proposed Residential Subdivision to forecast future peak periods of street traffic.
i) Analyze peak period traffic operations at the following key site access points. (To be confirmed with the County of Wellington and the Town of Erin)

- Street 'A'/Howe Street at Trafalgar Road North,
- Street ' $F$ ' at Trafalgar Road North.
j) Complete traffic operations and volume-capacity analyses using the Synchro 9.0 software.
k) Assess existing and future total background and total traffic operations (five (5) year horizon post development) at the proposed key access points mentioned above.

1) Prepare a report to summarize the findings of the traffic impact analysis, as well as to recommend any improvements required to mitigate the traffic impacts (if any). Submit the final report to the County of Wellington and the Town of Erin for review/comments.

## Traffic Impact Study - Terms of Reference (Cont'd)

m) Provide and circulate copies of the final report to all applicable approval authorities (first submission only).


## David Lee

## From:

Sent:
To:
Cc:

Subject:
Attachments:

Joe Mullan [mullan@ainleygroup.com](mailto:mullan@ainleygroup.com)
October-21-21 1:42 PM
Brian Wong
Nick Colucci; Angela Sciberras; Tanjot Bal; Pasquale Costanzo; David Lee; Diarmuid Horgan
W21081-5916 Trafalgar Road North - Terms of Reference (Town of Erin) Erin Development Ownership Map.pdf

## Hi Brian:

We have review your proposed Terms of Reference for the Hillsburgh Heights (Briarwood) development and we provide the following comments:

1. Given the size of the development, we concur with the minimum of two access street from the Development onto Trafalgar Road. These access streets should be designed in accordance with the TAC Manual in relation to intersection spacing and corner clearance requirements etc.
2. Please utilize the Institute of Transportation Engineers Trip Generation Manual $10^{\text {th }}$ Edition for site trip estimate and using traffic count data and Transportation Tomorrow Survey data for trip distribution.
3. Given the size of the development, future horizons should include build out year of any phases if applicable, plus the full build out year of the development, along with five and ten years post full build out.
4. It is unlikely that there is any traffic data/turning movements for Trafalgar Road and the existing intersections through Hillsburgh, therefore, we require that Traffic Counts/turning movement data be obtained for the 3 hrs Weekday AM period ( 7 am to 10 am ) and 3 hrs Weekday PM from 3 pm to 6 pm ). This will account for any possible changes that have occurred to people's work schedules because of the pandemic, whereby the peak hour may occur later in the morning or earlier in the afternoon.
5. With respect to future growth within Hillsburgh, there are four other major developments that are proposing to develop within the next number of years. Given that Trafalgar Road is the main spine of Hillsburgh, all new developments will impact the amount of traffic utilizing Trafalgar Road; therefore, these new Developments need to be accounted for in the TIS for your clients development.

The details we have associated with these future developments in Hillsburgh are noted below and the locations of each are shown on the attached map

| $\#$ | Development Name | Number of <br> Proposed SDE's |
| :---: | :--- | :---: |
| 2 | Carson Reid Homes Ltd | 182 |
| 3 | Thomasfield Homes Ltd | 210 |
| 4 | Dominion Packers \& Realties <br> (Tavares) | 700 |
| 16 | Chantler | 213 |

6. Given the potential impact of these developments collectively ( 1,625 SDE's) on Trafalgar Road, please include the following intersections in the traffic counts and associated analysis within the TIS for your clients development.

- Trafalgar Road / Upper Canada Drive / Church Street
- Trafalgar Road / Mill Street/George Street
- Trafalgar Road / County 22

Should you have any questions regarding this information please do not hesitate to contact me
Regards,

J. A. Mullan, P.Eng.

President \& CEO

Tel: (705) 445-3451 Ext. 126
Cell: (705) 718-7230

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[^9]If you require any further information, please do not hesitate to contact me.
Brian Wong, P.Eng.

Intermediate Transportation Engineer

## CANDEVCON LIMITED CONSULTING ENGINEERS \& PLANNERS GTA WEST OFFICE (CORPORATE) 9358 Goreway Drive

Brampton, Ontario, L6P 0M7<br>(905)794-0600 OFFICE<br>(905)794-0611 FAX<br>E-mail: brian@candevcon.com

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David Lee

From:
Sent:
To:
Cc:
Subject:

Kooistra, Tim [tkooistra@dillon.ca](mailto:tkooistra@dillon.ca)
October-12-21 1:22 PM
Brian Wong
David Lee; Diarmuid Horgan; Pasquale Costanzo
Re: FW: W21081-5916 Trafalgar Road North - Terms of Reference (County of Wellington)

Hi Brian,
As promised, I am following up with regard to the proposed residential subdivision located at 5916 Trafalgar Road North (Wellington Road 24) within the Town of Erin and located immediately north of the community of Hillsburgh. As I noted during our phone call, Dillon Consulting Limited has been retained by the County of Wellington to review the proposed scope of work for traffic impact studies that may impact the County road network and associated intersections. As a result, this response is being provided on behalf of the County of Wellington for your consideration.

The required transportation impact study will need to consider the following:

- The Existing and/or Future Operational analysis at the intersections of:
- Wellington Road 24 (Trafalgar Road) and Howe Street / future Street 'A' - currently unsignalized
- Wellington Road 24 and Upper Canada Drive / Church Street - currently unsignalized
- Wellington Road 22 and Wellington Road 24 - signalized (signal timing is attached)
- Turning movement data will need to be collected at each of these three study area intersections.
- Future Operational analysis at:
- the proposed internal Street 'A' \& Street 'B' intersection - future roundabout.
- The proposed Wellington Road 24 (Trafalgar Road) and future Street 'E'
- Use a $2.0 \%$ per annum growth rate to forecast the traffic volumes to various horizon years including:
- 2021 (Existing)
- 2030 (Buildout)
- 2035 (5 years following build-out)
- The report should include a discussion as to whether or not a local road connection to McMurchy Lane and Upper Canada Drive could be introduced rather than connecting Street 'E' to Wellington Road 24.
- The trip generation and future traffic volumes will need to explicitly consider that a school will be constructed on the school block as discussed.
- Due to the vertical profile of Wellington Road 24, a safety assessment will need to be completed at both locations. As you can see across the corridor from where Street ' E ' was constructed, Barbour Drive features a cul-de-sac and no direct connection.
- Due to the vertical profile along Wellington Road 24 fronting the proposed residential development, sightline analysis needs to be completed at the locations of the two intersections are being proposed to connect to Wellington Road 24 (future Street 'A' \& future Street 'E'). Based on available speeds found along this portion of the corridor, a $70 \mathrm{~km} / \mathrm{h}$ design speed (posted $+30 \mathrm{~km} / \mathrm{h}$ ) should be used.
- The need for both a northbound left-turn lane and a southbound right-turn lane at the Howe Street / future Street 'A' intersection and the future Street 'E' intersection need to be explicitly assessed utilizing a $70 \mathrm{~km} / \mathrm{h}$ design speed.

Lastly, any background developments that may impact future traffic volumes in the study area (along Wellington Road 24) will need to be identified by Town of Erin staff.

As always, please let me know if you have any questions or comments on this matter.
Thanks,

| Tim |  |
| :---: | :---: |
|  | Tim Kooistra, C.E.T. <br> Dillon Consulting Limited 130 Dufferin Avenue Suite 1400 London, Ontario, N6A 5R2 T-519.438.1288 ext. 1330 |
| $x$ | $\left\lvert\, \begin{aligned} & F-519.672 .8209 \\ & M-519.851 .5403 \end{aligned}\right.$ |
|  | TKooistra@dillon.ca www.dillon.ca |
|  | $x$ $x$ $x$  |

On Thu, Oct 7, 2021 at 9:02 AM Pasquale Costanzo [pasqualec@wellington.ca](mailto:pasqualec@wellington.ca) wrote:
Hi Tim,

Could you review the attached terms of reference for a proposed subdivision at the north end of Hillsburgh and provide any comments.

Thank you


Pasquale Costanzo, C.E.T., CMMII Infrastructure Specialist
Technical Services Supervisor
County of Wellington, Roads Division
T 519.837.2601 x 2250
E pasqualec@wellington.ca

From: Brian Wong [brian@candevcon.com](mailto:brian@candevcon.com)
Sent: Wednesday, October 6, 2021 10:20 AM
To: Pasquale Costanzo [pasqualec@wellington.ca](mailto:pasqualec@wellington.ca)
Cc: David Lee < david@candevcon.com>; Diarmuid Horgan < dhorgan@candevcon.com>
Subject: W21081-5916 Trafalgar Road North - Terms of Reference (County of Wellington)

CAUTION: This email originated from outside the organization. Do not click links or open attachments unless you know the contents to be safe.

Good Morning Pasquale,

We are preparing a Traffic Impact Study for a proposed Residential Subdivision that is immediately west of Trafalgar Road North and north of Upper Canada Drive. Please find the Terms of Reference and the latest Preliminary Development Plan attached for your review and comment. In the meantime, can you please let me know whether the County has any recent turning movement counts for the intersection of Trafalgar Road North at Howe Street and can you please provide me with growth projections for Trafalgar Road North. The horizon year is anticipated for 2030.

If you require any further information, please do not hesitate to contact me.

Brian Wong, P.Eng.

Intermediate Transportation Engineer

## CANDEVCON LIMITED

CONSULTING ENGINEERS \& PLANNERS
GTA WEST OFFICE (CORPORATE)

## 9358 Goreway Drive

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## APPENDIX B

## TURNING MOVEMENT COUNTS

Traffic Monitoring • Services \& Products

## Project \#21-219 - Candevcon Limited

## Intersection Count Report

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |
| Site Code: | 2121900003 |
| Count Categories: | Cars, Trucks, Bicycles, Pedestrians |
| Count Period: | $07: 00-10: 00,15: 00-18: 00$ |
| Weather: | Clear |

# Traffic Count Map 

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Wellington Rd 22
2121900003
Erin
Oct 28, 2021


## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Wellington Rd 22
2121900003
Erin
Oct 28, 2021

Trafalgar Rd N - Traffic Summary

| Hour | North Approach Totals |  |  |  |  |  | South Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 53 | 176 | 24 | 0 | 253 | 0 | 3 | 85 | 27 | 0 | 115 | 0 | 368 |
| 08:00-09:00 | 59 | 148 | 19 | 0 | 226 | 0 | 12 | 110 | 41 | 0 | 163 | 0 | 389 |
| 09:00-10:00 | 33 | 117 | 21 | 0 | 171 | 0 | 5 | 124 | 25 | 0 | 154 | 0 | 325 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 60 | 137 | 25 | 0 | 222 | 0 | 23 | 257 | 46 | 0 | 326 | 0 | 548 |
| 16:00-17:00 | 50 | 138 | 22 | 0 | 210 | 0 | 22 | 283 | 68 | 0 | 373 | 0 | 583 |
| 17:00-18:00 | 21 | 136 | 22 | 0 | 179 | 0 | 20 | 296 | 59 | 0 | 375 | 0 | 554 |
| GRAND TOTAL | 276 | 852 | 133 | 0 | 1261 | 0 | 85 | 1155 | 266 | 0 | 1506 | 0 | 2767 |

## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Wellington Rd 22
2121900003
Erin
Oct 28, 2021

## Wellington Rd 22 - Traffic Summary

| Hour | East Approach Totals |  |  |  |  |  | West Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 34 | 34 | 19 | 0 | 87 | 0 | 15 | 36 | 23 | 0 | 74 | 0 | 161 |
| 08:00-09:00 | 43 | 41 | 27 | 0 | 111 | 0 | 26 | 55 | 26 | 0 | 107 | 0 | 218 |
| 09:00-10:00 | 26 | 32 | 42 | 0 | 100 | 0 | 13 | 33 | 7 | 0 | 53 | 0 | 153 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 38 | 55 | 94 | 0 | 187 | 0 | 24 | 28 | 15 | 0 | 67 | 0 | 254 |
| 16:00-17:00 | 43 | 62 | 81 | 0 | 186 | 0 | 44 | 54 | 11 | 0 | 109 | 0 | 295 |
| 17:00-18:00 | 32 | 56 | 70 | 0 | 158 | 0 | 43 | 33 | 4 | 0 | 80 | 0 | 238 |
| GRAND TOTAL | 216 | 280 | 333 | 0 | 829 | 0 | 165 | 239 | 86 | 0 | 490 | 0 | 1319 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafagar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{\text { b }}$ |  | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 5 | 40 | 0 | 0 | 45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 8 | 39 | 9 | 0 | 56 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 15 | 33 | 9 | 0 | 57 | 1 | 6 | 2 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 18 | 51 | 3 | 0 | 72 | 5 | 7 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 9 | 34 | 2 | 0 | 45 | 4 | 3 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 11 | 38 | 4 | 0 | 53 | 4 | 6 | 1 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 13 | 21 | 7 | 0 | 41 | 1 | 3 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 12 | 40 | 3 | 0 | 55 | 5 | 3 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 10 | 20 | 3 | 0 | 33 | 1 | 1 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 10 | 31 | 5 | 0 | 46 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 7 | 29 | 5 | 0 | 41 | 0 | 2 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 4 | 25 | 7 | 0 | 36 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 122 | 401 | 57 | 0 | 580 | 23 | 40 | 7 | 0 | 70 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\xrightarrow{+}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 14 | 31 | 1 | 0 | 46 | 6 | 6 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 14 | 29 | 11 | 0 | 54 | 4 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 8 | 21 | 3 | 0 | 32 | 5 | 4 | 2 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 6 | 34 | 7 | 0 | 47 | 3 | 12 | 1 | 0 | 16 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 9 | 33 | 5 | 0 | 47 | 1 | 2 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 15 | 33 | 4 | 0 | 52 | 3 | 2 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 9 | 26 | 9 | 0 | 44 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 13 | 40 | 4 | 0 | 57 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 7 | 36 | 7 | 0 | 50 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 6 | 36 | 6 | 0 | 48 | 1 | 2 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 2 | 22 | 5 | 0 | 29 | 2 | 3 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 2 | 37 | 4 | 0 | 43 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 105 | 378 | 66 | 0 | 549 | 26 | 33 | 3 | 0 | 62 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| GRAND TOTAL | 227 | 779 | 123 | 0 | 1129 | 49 | 73 | 10 | 0 | 132 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafagar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{+}{+}$ | $\bigcirc$ | Total | 4 | + |  | $\bigcirc$ | Total | 4 | - |  | ? | Total |  |  |
| 07:00 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 19 | 3 | 0 | 22 | 0 | 6 | 2 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | , | 26 | 6 | 0 | 33 | 0 | 4 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 2 | 21 | 13 | 0 | 36 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 2 | 18 | 12 | 0 | 32 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 4 | 15 | 11 | 0 | 30 | 0 | 5 | 1 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 2 | 25 | 6 | 0 | 33 | 1 | 3 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 3 | 34 | 7 | 0 | 44 | 0 | 4 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 1 | 21 | 6 | 0 | 28 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 2 | 25 | 5 | 0 | 32 | 0 | 9 | 1 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 24 | 4 | 0 | 28 | 0 | 7 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 2 | 29 | 6 | 0 | 37 | 0 | 7 | 2 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 19 | 262 | 79 | 0 | 360 | 1 | 57 | 14 | 0 | 72 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  |  |  | 4 | + | $\stackrel{1}{ }$ |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 4 | 43 | 11 | 0 | 58 | 1 | 6 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 6 | 74 | 7 | 0 | 87 | 1 | 4 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 7 | 53 | 12 | 0 | 72 | 2 | 3 | 2 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 2 | 67 | 12 | 0 |  | 0 | 7 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 5 | 59 | 10 | 0 | 74 | 0 | 5 | 3 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 6 | 74 | 18 | 0 | 98 | 3 | 2 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 4 | 69 | 14 | 0 | 87 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 3 | 70 | 23 | 0 | 96 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 7 | 81 | 13 | 0 | 101 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 5 | 79 | 13 | 0 | 97 | 1 | 3 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 3 | 55 | 16 | 0 | 74 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 3 | 69 | 16 | 0 |  | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 55 | 793 | 165 | 0 | 1013 | 10 | 43 | 8 | 0 | 61 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 74 | 1055 | 244 | 0 | 1373 | 11 | 100 | 22 | 0 | 133 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

East Approach - Wellington Rd 22

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{1}{1}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 10 | 8 | 4 | 0 | 22 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 9 | 8 | 5 | 0 | 22 | 2 | 1 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 11 | 11 | 6 | 0 | 28 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 14 | 7 | 6 | 0 | 27 | 5 | 0 | 4 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 9 | 11 | 7 | 0 | 27 | 2 | 1 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 5 | 10 | 5 | 0 | 20 | 1 | 1 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 4 | 10 | 2 | 0 | 16 | 3 | 1 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 6 | 7 | 10 | 0 | 23 | 2 | 1 | , | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 7 | 5 | 8 | 0 | 20 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 3 | 7 | 7 | 0 | 17 | 0 | 1 | , | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 5 | 6 | 11 | 0 | 22 | 3 | 4 | 3 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 83 | 96 | 71 | 0 | 250 | 20 | 11 | 17 | 0 | 48 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

East Approach - Wellington Rd 22


## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

West Approach - Wellington Rd 22

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  | ? | Total | - |  |  |  | Total | 4 | + |  | $\bigcirc$ | Total |  |  |
| 07:00 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 1 | 8 | 8 | 0 | 17 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 1 | 14 | 5 | 0 | 20 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 10 | 14 | 7 | 0 | 31 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 3 | 19 | 9 | 0 | 31 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 6 | 8 | 5 | 0 | 19 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 4 | 10 | 6 | 0 | 20 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 9 | 16 | 6 | 0 | 31 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 1 | 7 | 3 | 0 | 11 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | - |
| 09:15 | 5 | 9 | 2 | 0 |  | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 2 | 4 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 3 | 12 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 45 | 121 | 54 | 0 | 220 | 9 | 3 | 2 | 0 | 14 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

West Approach - Wellington Rd 22

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | - | $\stackrel{\rightharpoonup}{+}$ |  | Total | 4 | 1 |  |  | Total | 4 | - |  |  | Total |  |  |
| 15:00 | 4 | 5 | 3 | 0 | 12 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 7 | 7 | 1 | 0 | 15 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 5 | 8 | 4 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 6 | 7 | 3 | 0 | 16 | 1 | 1 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 12 | 12 | 3 | 0 | 27 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 11 | 12 | 1 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 6 | 14 | 3 | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 14 | 15 | 2 | 0 | 31 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 9 | 8 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 9 | 8 | 2 | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 12 | 4 | 0 | 0 | 16 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 13 | 12 | 0 | 0 | 25 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 108 | 112 | 22 | 0 | 242 | 3 | 3 | 8 | 0 | 14 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 153 | 233 | 76 | 0 | 462 | 12 | 6 | 10 | 0 | 28 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Peak Hour Diagram

# Ontario Traffic Inc． <br> Traffic Monitoring • Services \＆Products 

## Specified Period

From：
07：00：00
To：
10：00：00

One Hour Peak
From：
07：30：00
To：

## Intersection：

Site Code：
Trafalgar Rd N \＆Wellington Rd 22

Count Date：

2121900003
Oct 28， 2021


## Peds： 0

Wellington Rd 22

| \％ | 0.0 | 日 | Totals |  |
| :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 0 | 0 | 0 |
| 0 | 2 | 20 | 22 | 2 |
| 0 | 1 | 55 | 56 | \％ |
| 0 | 1 | 26 | 27 | 7 |


|  | West Approach |  |  |
| :---: | :---: | :---: | :---: |
|  | Out | In | Total |
| 日 | 101 | 64 | 165 |
| Do | 4 | 6 | 10 |
| ¢ं | 0 | 0 | 0 |
|  | 105 | 70 | 175 |



Peds： 0

| Totals |  |  | $\Rightarrow$ ？ |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 9 | 99 | 46 | 0 |
| 日 | 9 | 80 | 42 |  |
| 50 | 0 | 19 | 4 |  |
| \％${ }^{\text {d }}$ | 0 | 0 | 0 |  |

Trafalgar Rd $\mathbf{N}$

Wellington Rd 22

|  | Totals | $\square$ | 50 | \％ |
| :---: | :---: | :---: | :---: | :---: |
| C | 0 | 0 | 0 | 0 |
| 者 | 30 | 24 | 6 | 0 |
| $\leqslant$ | 39 | 37 | 2 | 0 |
| $\Gamma$ | 54 | 43 | 11 | 0 |

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N\& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Count Date: | 0 ct 28, 2021 |
| Period: | $07: 00-10: 00$ |

Peak Hour Data (07:30-08:30)

|  | North Approach Trafalgar Rd N |  |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  |  | East Approach Wellington Rd 22 |  |  |  |  |  | West Approach Wellington Rd 22 |  |  |  |  |  | Total Vehicl |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | 4 | - | $\rightarrow$ | $?$ | Peds | Total | 4 | - | $\stackrel{\rightharpoonup}{r}$ | $\cdots$ | Peds | Total | 4 | - | $\stackrel{\rightharpoonup}{r}$ | ? | Peds | Total |  | 个 | $\stackrel{\rightharpoonup}{r}$ |  | Peds | Total | es |
| 07:30 | 16 | 39 | 11 | 0 | 0 | 66 | 1 | 30 | 8 | 0 | 0 | 39 | 11 | 9 | 6 | 0 | 0 | 26 | 2 | 14 | 6 | 0 | 0 | 22 | 153 |
| 07:45 | 23 | 58 | 3 | 0 | 0 | 84 | 2 | 25 | 14 | 0 | 0 | 41 | 13 | 11 | 6 | 0 | 0 | 30 | 10 | 14 | 7 | 0 | 0 | 31 | 186 |
| 08:00 | 13 | 37 | 3 | 0 | 0 | 53 | 2 | 24 | 12 | 0 | 0 | 38 | 19 | 7 | 10 | 0 | 0 | 36 | 3 | 20 | 9 | 0 | 0 | 32 | 159 |
| 08:15 | 15 | 44 | 5 | 0 | 0 | 64 | 4 | 20 | 12 | 0 | 0 | 36 | 11 | 12 | 8 | 0 | 0 | 31 | 7 | 8 | 5 | 0 | 0 | 20 | 151 |
| Grand Total | 67 | 178 | 22 | 0 | 0 | 267 | 9 | 99 | 46 | 0 | 0 | 154 | 54 | 39 | 30 | 0 | 0 | 123 | 22 | 56 | 27 | 0 | 0 | 105 | 649 |
| $\begin{gathered} \text { Approach } \\ \% \end{gathered}$ | 25.1 | 66.7 | 8.2 | 0 |  | - | 5.8 | 64.3 | 29.9 | 0 |  | - | 43.9 | 31.7 | 24.4 | 0 |  | - | 21 | 53.3 | 25.7 | 0 |  | - |  |
| Totals \% | 10.3 | 27.4 | 3.4 | 0 |  | 41.1 | 1.4 | 15.3 | 7.1 | 0 |  | 23.7 | 8.3 | 6 | 4.6 | 0 |  | 19 | 3.4 | 8.6 | 4.2 | 0 |  | 16.2 |  |
| PHF | 0.73 | 0.77 | 0.5 | 0 |  | 0.79 | 0.56 | 0.83 | 0.82 | 0 |  | 0.94 | 0.71 | 0.81 | 0.75 | 0 |  | 0.85 | 0.55 | 0.7 | 0.75 | 0 |  | 0.82 | 0.87 |
| Cars | 53 | 156 | 18 | 0 |  | 227 | 9 | 80 | 42 | 0 |  | 131 | 43 | 37 | 24 | 0 |  | 104 | 20 | 55 | 26 | 0 |  | 101 | 563 |
| \% Cars | 79.1 | 87.6 | 81.8 | 0 |  | 85 | 100 | 80.8 | 91.3 | 0 |  | 85.1 | 79.6 | 94.9 | 80 | 0 |  | 84.6 | 90.9 | 98.2 | 96.3 | 0 |  | 96.2 | 86.7 |
| Trucks | 14 | 22 | 4 | 0 |  | 40 | 0 | 19 | 4 | 0 |  | 23 | 11 | 2 | 6 | 0 |  | 19 | 2 | 1 | 1 | 0 |  | 4 | 86 |
| \% Trucks | 20.9 | 12.4 | 18.2 | 0 |  | 15 | 0 | 19.2 | 8.7 | 0 |  | 14.9 | 20.4 | 5.1 | 20 | 0 |  | 15.4 | 9.1 | 1.8 | 3.7 | 0 |  | 3.8 | 13.3 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - | 0 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |

## Peak Hour Diagram

## Ontario Traffic Inc． <br> Traffic Monitoring • Services \＆Products

Specified Period
From：15：00：00
To：

One Hour Peak
From：
16：15：00
To： 17：15：00

## Intersection：

Site Code：
Trafalgar Rd N \＆Wellington Rd 22

Count Date：

2121900003
Oct 28， 2021

| Trafalgar Rd N |  |  |  |  | East Approach |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| お | 0 | 0 | 0 | 0 |  | Out | In | Total |
| 5 | 0 | 4 | 3 | 0 | $\theta$ | 186 | 161 | 347 |
| 日 |  | 135 | 44 | 0 |  | 5 | 4 | 9 |
| Totals |  | 39 | 47 | 0 | Oో | 0 | 0 | 0 |
|  |  | ， | $\rightarrow$ |  |  | 191 | 165 | 356 |

## Peds： 0

Weather conditions：

Clear
gnalized intersection＊＊

Wellington Rd 22

|  | Totals | $\square$ | 50 | \％ |
| :---: | :---: | :---: | :---: | :---: |
| C | 0 | 0 | 0 | 0 |
| 者 | 86 | 84 | 2 | 0 |
| $\leqslant$ | 58 | 57 | 1 | 0 |
| $\Gamma$ | 47 | 45 | 2 | 0 |

Wellington Rd 22


| West Approach |  |  |  |
| ---: | ---: | ---: | ---: |
| Out | In | Total |  |
| 95 | 101 | 196 |  |
| 2 | 6 | 8 |  |
|  | 0 | 0 | 0 |
| $\mathbf{9 7}$ | $\mathbf{1 0 7}$ | $\mathbf{2 0 4}$ |  |



Peds： 0

| － |  | $303$ | $\Rightarrow$ ？ |  |
| :---: | :---: | :---: | :---: | :---: |
| Totals | 25 |  | 68 | 0 |
| 日 | 20 | 294 | 68 |  |
| 50 | 5 | 9 | 0 |  |
| \％${ }^{\text {d }}$ | 0 | 0 | 0 |  |

Trafalgar Rd $\mathbf{N}$

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Wellington Rd 22 |
| :--- | :--- |
| Site Code: | 2121900003 |
| Count Date: | Oct 28, 2021 |
| Period: | $15: 00-18: 00$ |

Peak Hour Data (16:15-17:15)

|  | North Approach Trafalgar Rd N |  |  |  |  |  | South Approach Trafalgar Rd $\mathbf{N}$ |  |  |  |  |  | East Approach Wellington Rd 22 |  |  |  |  |  | West Approach Wellington Rd 22 |  |  |  |  |  | Total Vehicl |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | - | - | $\rightarrow$ | ? | Peds | Total | 4 | 个 | $\stackrel{\rightharpoonup}{r}$ | $?$ | Peds | Total | 4 | - | $\stackrel{ }{ }$ | $?$ | Peds | Total |  | - | $\stackrel{\rightharpoonup}{r}$ |  | Peds | Total | es |
| 16:15 | 18 | 35 | 4 | 0 | 0 | 57 | 9 | 76 | 18 | 0 | 0 | 103 | 12 | 17 | 19 | 0 | 0 | 48 | 11 | 12 | 1 | 0 | 0 | 24 | 232 |
| 16:30 | 9 | 27 | 9 | 0 | 0 | 45 | 5 | 72 | 14 | 0 | 0 | 91 | 8 | 10 | 18 | 0 | 0 | 36 | 6 | 14 | 3 | 0 | 0 | 23 | 195 |
| 16:45 | 13 | 41 | 4 | 0 | 0 | 58 | 3 | 71 | 23 | 0 | 0 | 97 | 15 | 16 | 26 | 0 | 0 | 57 | 14 | 16 | 3 | 0 | 0 | 33 | 245 |
| 17:00 | 7 | 36 | 7 | 0 | 0 | 50 | 8 | 84 | 13 | 0 | 0 | 105 | 12 | 15 | 23 | 0 | 0 | 50 | 9 | 8 | 0 | 0 | 0 | 17 | 222 |
| Grand Total | 47 | 139 | 24 | 0 | 0 | 210 | 25 | 303 | 68 | 0 | 0 | 396 | 47 | 58 | 86 | 0 | 0 | 191 | 40 | 50 | 7 | 0 | 0 | 97 | 894 |
| $\begin{array}{\|c\|} \hline \text { Approach } \\ \% \end{array}$ | 22.4 | 66.2 | 11.4 | 0 |  | - | 6.3 | 76.5 | 17.2 | 0 |  | - | 24.6 | 30.4 | 45 | 0 |  | - | 41.2 | 51.5 | 7.2 | 0 |  | - |  |
| Totals \% | 5.3 | 15.5 | 2.7 | 0 |  | 23.5 | 2.8 | 33.9 | 7.6 | 0 |  | 44.3 | 5.3 | 6.5 | 9.6 | 0 |  | 21.4 | 4.5 | 5.6 | 0.8 | 0 |  | 10.9 |  |
| PHF | 0.65 | 0.85 | 0.67 | 0 |  | 0.91 | 0.69 | 0.9 | 0.74 | 0 |  | 0.94 | 0.78 | 0.85 | 0.83 | 0 |  | 0.84 | 0.71 | 0.78 | 0.58 | 0 |  | 0.73 | 0.91 |
| Cars | 44 | 135 | 24 | 0 |  | 203 | 20 | 294 | 68 | 0 |  | 382 | 45 | 57 | 84 | 0 |  | 186 | 40 | 49 | 6 | 0 |  | 95 | 866 |
| \% Cars | 93.6 | 97.1 | 100 | 0 |  | 96.7 | 80 | 97 | 100 | 0 |  | 96.5 | 95.7 | 98.3 | 97.7 | 0 |  | 97.4 | 100 | 98 | 85.7 | 0 |  | 97.9 | 96.9 |
| Trucks | 3 | 4 | 0 | 0 |  | 7 | 5 | 9 | 0 | 0 |  | 14 | 2 | 1 | 2 | 0 |  | 5 | 0 | 1 | 1 | 0 |  | 2 | 28 |
| \% Trucks | 6.4 | 2.9 | 0 | 0 |  | 3.3 | 20 | 3 | 0 | 0 |  | 3.5 | 4.3 | 1.7 | 2.3 | 0 |  | 2.6 | 0 | 2 | 14.3 | 0 |  | 2.1 | 3.1 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - | 0 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |

Traffic Monitoring • Services \& Products

## Project \#21-219 - Candevcon Limited

## Intersection Count Report

Intersection: Trafalgar Rd N \& Mill St-George St<br>Municipality: Erin<br>Count Date: Oct 28,2021<br>Site Code: 2121900004<br>Count Categories: Cars, Trucks, Bicycles, Pedestrians<br>Count Period: $\quad 07: 00-10: 00,15: 00-18: 00$<br>Weather:<br>Clear

## Traffic Count Map

Ontario Traffic Inc.
Traffic Monitoring • Services \& Products

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Erin
Oct 28, 2021


## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Erin
Oct 28, 2021

## Trafalgar Rd N - Traffic Summary

| Hour | North Approach Totals |  |  |  |  |  | South Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 2 | 219 | 3 | 0 | 224 | 0 | 4 | 119 | 4 | 0 | 127 | 1 | 351 |
| 08:00-09:00 | 8 | 176 | 0 | 0 | 184 | 0 | 14 | 120 | 10 | 0 | 144 | 1 | 328 |
| 09:00-10:00 | 5 | 127 | 3 | 0 | 135 | 0 | 18 | 134 | 15 | 0 | 167 | 0 | 302 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 15 | 167 | 5 | 0 | 187 | 1 | 61 | 224 | 40 | 0 | 325 | 0 | 512 |
| 16:00-17:00 | 11 | 156 | 4 | 0 | 171 | 0 | 51 | 286 | 38 | 0 | 375 | 0 | 546 |
| 17:00-18:00 | 14 | 124 | 2 | 0 | 140 | 0 | 50 | 284 | 41 | 0 | 375 | 2 | 515 |
| GRAND TOTAL | 55 | 969 | 17 | 0 | 1041 | 1 | 198 | 1167 | 148 | 0 | 1513 | 4 | 2554 |

## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Erin
Oct 28, 2021

## Mill St - Traffic Summary

| Hour | East Approach Totals |  |  |  |  |  | West Approach Totals |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Includes Cars, Trucks, Bicycles |  |  |  |  | Left | Includes Cars, Trucks, Bicycles |  |  |  |  | Total |
|  |  | Thru | Right | U-Turn | Total | Peds |  | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 17 | 0 | 7 | 0 | 24 | 0 | 2 | 2 | 8 | 0 | 12 | 0 | 36 |
| 08:00-09:00 | 29 | 3 | 7 | 0 | 39 | 3 | 7 | 1 | 8 | 0 | 16 | 0 | 55 |
| 09:00-10:00 | 19 | 5 | 5 | 0 | 29 | 0 | 2 | 2 | 23 | 0 | 27 | 0 | 56 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 24 | 10 | 13 | 0 | 47 | 0 | 20 | 9 | 44 | 0 | 73 | 0 | 120 |
| 16:00-17:00 | 25 | 13 | 13 | 0 | 51 | 3 | 20 | 8 | 30 | 0 | 58 | 5 | 109 |
| 17:00-18:00 | 14 | 6 | 8 | 0 | 28 | 0 | 14 | 16 | 29 | 0 | 59 | 2 | 87 |
| GRAND TOTAL | 128 | 37 | 53 | 0 | 218 | 6 | 65 | 38 | 142 | 0 | 245 | 7 | 463 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | 1 | $\stackrel{+}{+}$ |  | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 1 | 25 | 2 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 57 | 0 | 0 | 57 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 1 | 52 | 0 | 0 | 53 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 59 | 1 | 0 | 60 | 0 | 12 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 1 | 43 | 0 | 0 | 44 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 2 | 32 | 0 | 0 | 34 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 2 | 35 | 0 | 0 | 37 | 0 | 10 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 3 | 36 | 0 | 0 | 39 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 24 | 1 | 0 | 25 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 2 | 27 |  | 0 | 30 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 2 | 30 | 1 | 0 | 33 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 0 | 27 | 0 | 0 | 27 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 14 | 447 | 6 | 0 | 467 | 1 | 75 | 0 | 0 | 76 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafagar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{+}$ | $\bigcirc$ | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 3 | 38 | 2 | 0 | 43 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 3 | 35 | 0 | 0 | 38 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 5 | 22 | 0 | 0 | 27 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 15:45 | 4 | 40 | 3 | 0 | 47 | 0 | 12 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 6 | 44 | 3 | 0 | 53 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 0 | 45 | 0 | 0 | 45 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 3 | 31 | 0 | 0 | 34 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 2 | 28 | 1 | 0 | 31 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 4 | 35 | 0 | 0 | 39 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 7 | 35 | 0 | 0 | 42 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 3 | 27 | 1 | 0 | 31 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 0 | 23 | 1 | 0 | 24 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBtotal | 40 | 403 | 11 | 0 | 454 | 0 | 44 | 0 | 0 | 44 | 0 |  |  | 0 | 0 |  | 1 |
| GRAND TOTAL | 54 | 850 | 17 | 0 | 921 | 1 | 119 | 0 | 0 | 120 | 0 | 0 | 0 | 0 | 0 |  | 1 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  |  | Total | 4 |  |  |  | Total | 4 | + | $\stackrel{1}{+}$ | ? | Total |  |  |
| 07:00 | 0 | 17 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 28 | 2 | 0 | 30 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 07:30 | 0 | 27 | 0 | 0 | 27 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 4 | 28 | 1 | 0 | 33 | 0 | 7 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 2 | 21 | 6 | 0 | 29 | 0 | 10 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 3 | 19 | 0 | 0 | 22 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 4 | 26 | 1 | 0 | 31 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 08:45 | 5 | 25 | 3 | 0 | 33 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 7 | 22 | 4 | 0 | 33 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 5 | 23 | 2 | 0 | 30 | 1 | 8 | 2 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 2 | 28 | 0 | 0 | 30 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 3 | 30 | 6 | 0 | 39 | 0 | 11 | 1 | 0 | 12 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 35 | 294 | 25 | 0 | 354 | 1 | 79 | 4 | 0 | 84 | 0 | 0 | 0 | 0 | 0 |  | 2 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{ }{+}$ | $\bigcirc$ | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 12 | 46 | 10 | 0 | 68 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 19 | 49 | 7 | 0 | 75 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 14 | 50 | 8 | 0 | 72 | 1 | 5 | 2 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 15 | 56 | 12 | 0 | 83 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 11 | 61 | 15 | 0 | 87 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 13 | 75 | 14 | 0 | 102 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 15 | 73 | 3 | 0 | 91 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 10 | 61 | 6 | 0 | 77 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 15 | 74 | 17 | 0 | 106 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 11 | 80 | 9 | 0 | 100 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 11 | 56 | 7 | 0 | 74 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 2 |
| 17:45 | 13 | 61 | 8 | 0 |  | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 159 | 742 | 116 | 0 | 1017 | 3 | 52 | 3 | 0 | 58 | 0 | 0 |  | 0 | 0 |  | 2 |
| GRAND TOTAL | 194 | 1036 | 141 | 0 | 1371 | 4 | 131 | 7 | 0 | 142 | 0 | 0 | 0 | 0 | 0 |  | 4 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

East Approach - Mill St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{1}$ | 2 | Total | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | 者 |  |  | Total |  |  |
| 07:00 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 5 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 8 | 0 | 3 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 3 | 0 | 4 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 10 | 0 | 3 | 0 | 13 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 3 | 0 | 1 | 0 | 4 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 7 | 2 | 0 | 0 | 9 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 3 |
| 08:45 | 5 | 1 | 1 | 0 | 7 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 6 | 0 | 2 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 5 | 2 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 3 | 1 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 4 | 2 | 2 | 0 | 8 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 60 | 8 | 17 | 0 | 85 | 5 | 0 | 2 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 3 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Erin
Oct 28, 2021

## East Approach - Mill St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | 1 | $\stackrel{\rightharpoonup}{\text { Pr }}$ | $\bigcirc$ | Total | 4 | + |  |  | Total | 4 | + |  | ? | Total |  |  |
| 15:00 | 6 | 1 | 3 | 0 | 10 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 8 | 5 | 2 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 1 | 2 | 3 | 0 | 6 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 4 | 2 | 5 | 0 | 11 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 6 | 3 | 6 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 6 | 5 | 2 | 0 | 13 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 3 |
| 16:30 | 7 | 1 | 5 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 5 | 4 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 4 | 1 | 3 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 3 | 3 | 3 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 1 | 1 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 6 | , | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 57 | 29 | 34 | 0 | 120 | 6 | 0 |  | 0 | 6 | 0 |  |  | 0 | 0 |  | 3 |
| GRAND TOTAL | 117 | 37 | 51 | 0 | 205 | 11 | 0 | 2 | 0 | 13 | 0 | 0 | 0 | 0 | 0 |  | 6 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |

West Approach - George St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{1}{ }$ | $\bigcirc$ | Total | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | 者 |  |  | Total |  |  |
| 07:00 | 1 | 1 | 3 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 1 | 1 | 3 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 3 | 1 | 4 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 4 | 0 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 0 | 9 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 0 | 1 | 5 | 0 | 6 | 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 1 | 1 | 4 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 10 | 5 | 36 | 0 | 51 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafaggar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

West Approach - George St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | - | 1 | $\stackrel{\rightharpoonup}{r}$ |  | Total | - | - |  |  | Total | - | - |  |  | Total |  |  |
| 15:00 | 9 | 1 | 14 | 0 | 24 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 5 | 1 | 9 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 1 | 2 | 10 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 |  | 0 |
| 15:45 | 2 | 5 | 10 | 0 | 17 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 4 | 2 | 11 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 |  | 0 |
| 16:15 | 4 | 6 | 8 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 3 |
| 16:30 | 4 | 0 | 7 | 0 | 11 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 2 |
| 16:45 | 4 | 0 | 4 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 2 | 10 | 11 | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 9 | 2 | 10 | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 1 | 2 | 3 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 |
| 17:45 | 2 | 2 | 5 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 47 | 33 | 102 | 0 | 182 | 3 | 0 | 1 | 0 | 4 | 4 | 0 | 0 | 0 | 4 |  | 7 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 57 | 38 | 138 | 0 | 233 | 4 | 0 | 4 | 0 | 8 | 4 | 0 | 0 | 0 | 4 |  | 7 |

## Peak Hour Diagram

# Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products 

## Specified Period

From:
07:00:00
To:
10:00:00

One Hour Peak
From:
07:15:00
To: 08:15:00

Intersection:
Site Code:
Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Oct 28, 2021

Major Road: Trafalgar Rd N runs N/S


## Peds: 0

| \% | 100 | $\square$ | Totals |  |
| :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 0 | 0 | ? |
| 0 | 0 | 1 |  | + |
| 0 | 0 | 1 | 1 | $\Rightarrow$ |
| 0 | 1 | 5 | 6 | 7 |


Peds: 1

|  |  | Totals | $\square$ | 50 | O\% |
| :---: | :---: | :---: | :---: | :---: | :---: |
| - | C | 0 | 0 | 0 | 0 |
| 0 | t | 11 | 10 | 1 | 0 |
|  | - | 0 | 0 | 0 | 0 |
|  | $F$ | 28 | 26 | 2 | 0 |


| West Approach |  |  |
| ---: | ---: | ---: |
| Out | In | Total |
| 7 | 7 | 14 |
| 1 | 0 | 1 |
| $\mathbf{8}$ | $\mathbf{7}$ | $\mathbf{1 5}$ |


|  |  | - |  |  | South Approach |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Totals | 6 | 133 | 10 | 0 |  | Out | In | Total |
| 日 | 6 | 104 | 9 | 0 | 日 | 119 | 242 | 361 |
| 02 | 0 | 29 | 1 | 0 | $\square$ | 30 | 36 | 66 |
| -10 | 0 | 0 | 0 | 0 | O\% | 0 | 0 | 0 |
| rafalgar Rd N |  |  |  |  |  | 149 | 278 | 427 |

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafaggar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Count Date: | Oct 28, 2021 |
| Period: | $07: 00-10: 00$ |

Peak Hour Data (07:15-08:15)

| Start Time | North Approach Trafalgar Rd $\mathbf{N}$ |  |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  |  | East Approach Mill St |  |  |  |  |  | West Approach George St |  |  |  |  |  | Total Vehicl es |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | - |  | $\stackrel{\rightharpoonup}{4}$ |  |  | Total | 4 |  |  |  | Peds | Total |  | $\uparrow$ |  |  |  | Total |  | $\uparrow$ |  |  | Peds | Total |  |
| 07:15 | 0 | 65 | 0 | 0 | 0 | 65 | 0 | 34 | 2 | 0 | 1 | 36 | 5 | 0 | 0 | 0 | 0 | 5 | 1 | 1 | 3 | 0 | 0 | 5 | 111 |
| 07:30 | 1 | 58 | 0 | 0 | 0 | 59 | 0 | 33 | 0 | 0 | 0 | 33 | 8 | 0 | 3 | 0 | 0 | 11 | 0 | 0 | 2 | 0 | 0 | 2 | 105 |
| 07:45 | 0 | 71 | 1 | 0 | 0 | 72 | 4 | 35 | 2 | 0 | 0 | 41 | 3 | 0 | 4 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 120 |
| 08:00 | 1 | 50 | 0 | 0 | 0 | 51 | 2 | 31 | 6 | 0 | 0 | 39 | 12 | 0 | 4 | 0 | 0 | 16 | 0 | 0 | 1 | 0 | 0 | 1 | 107 |
| Grand Total | 2 | 244 | 1 | 0 | 0 | 247 | 6 | 133 | 10 | 0 | 1 | 149 | 28 | 0 | 11 | 0 | 0 | 39 | 1 | 1 | 6 | 0 | 0 | 8 | 443 |
| $\begin{gathered} \text { Approach } \\ \% \end{gathered}$ | 0.8 | 98.8 | 0.4 | 0 |  | - | 4 | 89.3 | 6.7 | 0 |  | - | 71.8 | 0 | 28.2 | 0 |  | - | 12.5 | 12.5 | 75 | 0 |  | - |  |
| Totals \% | 0.5 | 55.1 | 0.2 | 0 |  | 55.8 | 1.4 | 30 | 2.3 | 0 |  | 33.6 | 6.3 | 0 | 2.5 | 0 |  | 8.8 | 0.2 | 0.2 | 1.4 | 0 |  | 1.8 |  |
| PHF | 0.5 | 0.86 | 0.25 | 0 |  | 0.86 | 0.38 | 0.95 | 0.42 | 0 |  | 0.91 | 0.58 | 0 | 0.69 | 0 |  | 0.61 | 0.25 | 0.25 | 0.5 | 0 |  | 0.4 | 0.92 |
| Cars | 2 | 211 | 1 | 0 |  | 214 | 6 | 104 | 9 | 0 |  | 119 | 26 | 0 | 10 | 0 |  | 36 | 1 | 1 | 5 | 0 |  | 7 | 376 |
| \% Cars | 100 | 86.5 | 100 | 0 |  | 86.6 | 100 | 78.2 | 90 | 0 |  | 79.9 | 92.9 | 0 | 90.9 | 0 |  | 92.3 | 100 | 100 | 83.3 | 0 |  | 87.5 | 84.9 |
| Trucks | 0 | 33 | 0 | 0 |  | 33 | 0 | 29 | 1 | 0 |  | 30 | 2 | 0 | 1 | 0 |  | 3 | 0 | 0 | 1 | 0 |  | 1 | 67 |
| \% Trucks | 0 | 13.5 | 0 | 0 |  | 13.4 | 0 | 21.8 | 10 | 0 |  | 20.1 | 7.1 | 0 | 9.1 | 0 |  | 7.7 | 0 | 0 | 16.7 | 0 |  | 12.5 | 15.1 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 1 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - | 1 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 100 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |

## Peak Hour Diagram

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

Specified Period
From: 15:00:00
To:

One Hour Peak
From:
15:45:00
To:

## Intersection:

Site Code:
Count Date:

Trafalgar Rd N \& Mill St-George St
2121900004
Oct 28, 2021

Weather conditions:

Clear




George St


## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Mill St-George St |
| :--- | :--- |
| Site Code: | 2121900004 |
| Count Date: | 0 ct 28, 2021 |
| Period: | $15: 00-18: 00$ |

Peak Hour Data (15:45-16:45)

|  | North Approach Trafalgar Rd N |  |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  |  | East Approach Mill St |  |  |  |  |  | West Approach George St |  |  |  |  |  | Total Vehicl |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | 4 | $\uparrow$ | $\stackrel{r}{ }$ |  | Peds | Total |  |  | $\stackrel{\rightharpoonup}{1}$ |  | Peds | Total |  | - | $\stackrel{ }{ }$ | $?$ | Peds | Total |  | $\uparrow$ | $\rightarrow$ |  | Peds | Total | es |
| 15:45 | 4 | 52 | 3 | 0 | 0 | 59 | 15 | 60 | 13 | 0 | 0 | 88 | 7 | 2 | 5 | 0 | 0 | 14 | 3 | 5 | 10 | 0 | 0 | 18 | 179 |
| 16:00 | 6 | 48 | 3 | 0 | 0 | 57 | 11 | 70 | 15 | 0 | 0 | 96 | 6 | 3 | 6 | 0 | 0 | 15 | 6 | 2 | 11 | 0 | 0 | 19 | 187 |
| 16:15 | 0 | 47 | 0 | 0 | 0 | 47 | 15 | 76 | 14 | 0 | 0 | 105 | 7 | 5 | 2 | 0 | 3 | 14 | 4 | 6 | 8 | 0 | 3 | 18 | 184 |
| 16:30 | 3 | 32 | 0 | 0 | 0 | 35 | 15 | 78 | 3 | 0 | 0 | 96 | 7 | 1 | 5 | 0 | 0 | 13 | 6 | 0 | 7 | 0 | 2 | 13 | 157 |
| Grand Total | 13 | 179 | 6 | 0 | 0 | 198 | 56 | 284 | 45 | 0 | 0 | 385 | 27 | 11 | 18 | 0 | 3 | 56 | 19 | 13 | 36 | 0 | 5 | 68 | 707 |
| Approach | 6.6 | 90.4 | 3 | 0 |  | - | 14.5 | 73.8 | 11.7 | 0 |  | - | 48.2 | 19.6 | 32.1 | 0 |  | - | 27.9 | 19.1 | 52.9 | 0 |  | - |  |
| Totals \% | 1.8 | 25.3 | 0.8 | 0 |  | 28 | 7.9 | 40.2 | 6.4 | 0 |  | 54.5 | 3.8 | 1.6 | 2.5 | 0 |  | 7.9 | 2.7 | 1.8 | 5.1 | 0 |  | 9.6 |  |
| PHF | 0.54 | 0.86 | 0.5 | 0 |  | 0.84 | 0.93 | 0.91 | 0.75 | 0 |  | 0.92 | 0.96 | 0.55 | 0.75 | 0 |  | 0.93 | 0.79 | 0.54 | 0.82 | 0 |  | 0.89 | 0.95 |
| Cars | 13 | 160 | 6 | 0 |  | 179 | 54 | 265 | 44 | 0 |  | 363 | 23 | 11 | 18 | 0 |  | 52 | 14 | 13 | 36 | 0 |  | 63 | 657 |
| \% Cars | 100 | 89.4 | 100 | 0 |  | 90.4 | 96.4 | 93.3 | 97.8 | 0 |  | 94.3 | 85.2 | 100 | 100 | 0 |  | 92.9 | 73.7 | 100 | 100 | 0 |  | 92.6 | 92.9 |
| Trucks | 0 | 19 | 0 | 0 |  | 19 | 2 | 19 | 1 | 0 |  | 22 | 4 | 0 | 0 | 0 |  | 4 | 3 | 0 | 0 | 0 |  | 3 | 48 |
| \% Trucks | 0 | 10.6 | 0 | 0 |  | 9.6 | 3.6 | 6.7 | 2.2 | 0 |  | 5.7 | 14.8 | 0 | 0 | 0 |  | 7.1 | 15.8 | 0 | 0 | 0 |  | 4.4 | 6.8 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 2 | 0 | 0 | 0 |  | 2 | 2 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 10.5 | 0 | 0 | 0 |  | 2.9 | 0.3 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 3 | - |  |  |  |  | 5 | - | 8 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |  |  |  | 37.5 | - |  |  |  |  | 62.5 | - |  |

Traffic Monitoring • Services \& Products

## Project \#21-219 - Candevcon Limited

## Intersection Count Report

Intersection: Trafalgar Rd N \& Upper Canada Dr-Church St<br>Municipality: Erin<br>Count Date: Oct 28,2021<br>Site Code: 2121900002<br>Count Categories: Cars, Trucks, Bicycles, Pedestrians<br>Count Period: $\quad 07: 00-10: 00,15: 00-18: 00$<br>Weather:<br>Clear

## Traffic Count Map

Trafalgar Rd N \& Upper Canada Dr-Church St

2121900002
Erin
Oct 28, 2021


## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

## Trafalgar Rd N - Traffic Summary

| Hour | North Approach Totals |  |  |  |  |  | South Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 2 | 200 | 1 | 0 | 203 | 0 | 1 | 120 | 4 | 0 | 125 | 4 | 328 |
| 08:00-09:00 | 1 | 142 | 1 | 0 | 144 | 2 | 7 | 107 | 5 | 0 | 119 | 0 | 263 |
| 09:00-10:00 | 4 | 103 | 1 | 0 | 108 | 0 | 1 | 118 | 6 | 0 | 125 | 2 | 233 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 4 | 179 | 3 | 0 | 186 | 0 | 8 | 245 | 15 | 0 | 268 | 1 | 454 |
| 16:00-17:00 | 6 | 190 | 2 | 0 | 198 | 1 | 16 | 325 | 10 | 0 | 351 | 5 | 549 |
| 17:00-18:00 | 7 | 139 | 2 | 0 | 148 | 0 | 13 | 300 | 9 | 0 | 322 | 1 | 470 |
| GRAND TOTAL | 24 | 953 | 10 | 0 | 987 | 3 | 46 | 1215 | 49 | 0 | 1310 | 13 | 2297 |

## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Upper Canada Dr-Church St
2121900002
Erin
Oct 28, 2021

Church St - Traffic Summary

| Hour | East Approach Totals |  |  |  |  |  | West Approach Totals |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Includes Cars, Trucks, Bicycles |  |  |  |  | Left | Includes Cars, Trucks, Bicycles |  |  |  | Peds | Total |
|  |  | Thru | Right | U-Turn | Total | Peds |  | Thru | Right | U-Turn | Total |  |  |
| 07:00-08:00 | 8 | 0 | 2 | 0 | 10 | 0 | 4 | 0 | 13 | 0 | 17 | 0 | 27 |
| 08:00-09:00 | 7 | 1 | 5 | 0 | 13 | 0 | 3 | 0 | 10 | 0 | 13 | 5 | 26 |
| 09:00-10:00 | 8 | 0 | 4 | 0 | 12 | 1 | 2 | 0 | 8 | 0 | 10 | 1 | 22 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 10 | 7 | 5 | 0 | 22 | 0 | 3 | 3 | 6 | 0 | 12 | 0 | 34 |
| 16:00-17:00 | 6 | 1 | 3 | 0 | 10 | 1 | 1 | 1 | 2 | 0 | 4 | 0 | 14 |
| 17:00-18:00 | 8 | 1 | 4 | 0 | 13 | 0 | 6 | 0 | 6 | 0 | 12 | 0 | 25 |
| GRAND TOTAL | 47 | 10 | 23 | 0 | 80 | 2 | 19 | 4 | 45 | 0 | 68 | 6 | 148 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafaggar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 0 | 40 | 0 | 0 | 40 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 2 | 42 | 0 | 0 | 44 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 0 | 41 | 0 | 0 | 41 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 51 | 1 | 0 | 52 | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 0 | 33 | 0 | 0 | 33 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 0 | 28 | 1 | 0 | 29 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 0 | 27 | 0 | 0 | 27 | 1 | 7 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 08:45 | 0 | 28 | 0 | 0 | 28 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 09:00 | 0 | 14 | 0 | 0 | 14 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 1 | 23 | 0 | 0 | 24 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 1 | 25 | 1 | 0 | 27 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 2 | 22 | 0 | 0 | 24 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 6 | 374 | 3 | 0 | 383 | 1 | 71 | 0 | 0 | 72 | 0 | 0 | 0 | 0 | 0 |  | 2 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  |  | Total | 4 |  |  | ? | Total | 4 | - |  | ? | Total |  |  |
| 15:00 | 4 | 31 | 1 | 0 | 36 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 0 | 37 | 0 | 0 | 37 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 0 | 32 | 1 | 0 | 33 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 0 | 50 | 1 | 0 | 51 | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 0 | 48 | 0 | 0 | 48 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 4 | 50 | 0 | 0 | 54 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 1 | 34 | 0 | 0 | 35 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 1 | 50 | 2 | 0 | 53 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 17:00 | 4 | 32 | 0 | 0 | 36 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 1 | 38 | 1 | 0 | 40 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 2 | 37 | 0 | 0 | 39 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 0 | 27 | 1 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBtotal | 17 |  | 7 | 0 | 490 | 0 | 42 | 0 | 0 | 42 | 0 |  |  | 0 | 0 |  | 1 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 23 | 840 | 10 | 0 | 873 | 1 | 113 | 0 | 0 | 114 | 0 | 0 | 0 | 0 | 0 |  | 3 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | 0 ct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{ }{ }$ | ? | Total | - | + |  |  | Total | 4 | + |  | $\bigcirc$ | Total |  |  |
| 07:00 | 1 | 18 | 0 | 0 | 19 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 19 | 2 | 0 | 21 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 2 |
| 07:30 | 0 | 29 | 1 | 0 | 30 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 2 |
| 07:45 | 0 | 30 | 1 | 0 | 31 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 2 | 19 | 0 | 0 | 21 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 1 | 18 | 0 | 0 | 19 | 0 | 7 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 2 | 21 | 1 | 0 | 24 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 2 | 25 | 1 | 0 | 28 | 0 | 4 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 14 | 0 | 0 | 14 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 0 | 19 | 0 | 0 | 19 | 0 | 9 | 2 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 1 | 28 | 2 | 0 | 31 | 0 | 5 | 1 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 09:45 | 0 | 25 | 1 | 0 | 26 | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| SUBTOTAL | 9 | 265 | 9 | 0 | 283 | 0 | 80 | 6 | 0 | 86 | 0 | 0 | 0 | 0 | 0 |  | 6 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  |  | Total | 4 |  |  | ? | Total | 4 | + |  | ? | Total |  |  |
| 15:00 | 1 | 52 | 0 | 0 | 53 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 5 | 54 | 2 | 0 | 61 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 0 | 55 | 4 | 0 | 59 | 1 | 4 | 1 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 15:45 | 1 | 62 | 8 | 0 | 71 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 2 | 73 | 1 | 0 | 76 | 0 | 9 | 1 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 5 |
| 16:15 | 6 | 81 | 2 | 0 | 89 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 5 | 78 | 5 | 0 | 88 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 3 | 75 | 1 | 0 | 79 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 3 | 85 | 3 | 0 | 91 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 17:15 | 2 | 86 | 0 | 0 | 88 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 5 | 63 | 1 | 0 | 69 | 1 | 2 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 2 | 56 | 5 | 0 | 63 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 35 | 820 | 32 | 0 | 887 | 2 | 50 | 2 | 0 | 54 | 0 | 0 |  | 0 | 0 |  | 7 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 44 | 1085 | 41 | 0 | 1170 | 2 | 130 | 8 | 0 | 140 | 0 | 0 | 0 | 0 | 0 |  | 13 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

East Approach - Church St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | - |  |  | Total |  |  |
| 07:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 3 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 2 | 0 | 2 | 0 | 4 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 3 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 3 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 4 | 0 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| SUBTOTAL | 23 | 0 | 11 | 0 | 34 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 1 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28,2021 |

East Approach - Church St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | 1 | $\stackrel{+}{+}$ | $\bigcirc$ | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 2 | 7 | 1 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 1 | 0 | 2 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 5 | 0 | 2 | 0 | 7 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 4 | 0 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 16:15 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 3 | 1 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 22 | 9 | 12 | 0 | 43 | 2 | 0 | 0 | 0 | 2 | 0 |  |  | 0 | 0 |  | 1 |
| GRAND TOTAL | 45 | 9 | 23 | 0 | 77 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 2 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafagar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

West Approach - Upper Canada Dr

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | 1 |  |  | Total | 4 | + |  | ? | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 0 | 8 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 |
| 08:45 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 0 | 4 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 1 | 0 | 4 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 |
| 09:45 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBtotal | 9 | 0 | 30 | 0 | 39 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 6 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

West Approach - Upper Canada Dr

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | 1 | $\stackrel{+}{+}$ | $\bigcirc$ | Total | 4 | + |  |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 1 | 3 | 0 | 0 | 4 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 0 | 0 | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 1 | 1 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 3 | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 8 | 4 | 14 | 0 | 26 | 2 | 0 | 0 | 0 | 2 | 0 | 0 |  | 0 | 0 |  | 0 |
| GRAND TOTAL | 17 | 4 | 44 | 0 | 65 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 6 |

## Peak Hour Diagram

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

Specified Period
From:
07:00:00
To:
10:00:00

One Hour Peak
From:
07:15:00
To: 08:15:00

Intersection:
Site Code:
Count Date:

Trafalgar Rd N \& Upper Canada Dr-Church St 2121900002

Oct 28, 2021


Upper Canada Dr

| - | 0.0 | 日 | Totals |  |
| :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 0 | 0 | 0 |
| 0 | 0 | 1 | 1 | 1 1 |
| 0 | 0 | 0 | 0 | $0 \Rightarrow$ |
| 0 | 0 | 16 | 16 | 5 |


\left.| West Approach |  |  |  |
| ---: | ---: | ---: | :---: |
| Out | In | Total |  |
| 17 | 3 | 20 |  |
|  | 0 | 1 |  |$\right) 1$



Peds: 4 0
$\stackrel{0}{0}$
0
0

Church St

|  | Totals | $\square$ |  | - ${ }^{\circ}$ |
| :---: | :---: | :---: | :---: | :---: |
| C | 0 | 0 | 0 | 0 |
| E | 4 | 4 | 0 | 0 |
| $\leqslant$ | 1 | 0 | 1 | 0 |
| $F$ | 10 | 10 | 0 | 0 |


| Totals |  |  | $\rightarrow$ ? |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 2 | 127 | 4 | 0 |
| 日 | 2 | 97 | 4 | 0 |
| $0 \cdot 1$ | 0 | 30 | 0 |  |
| \% ${ }^{\text {d }}$ | 0 | 0 | 0 |  |

Trafalgar Rd $\mathbf{N}$

| South Approach |  |  |  |
| ---: | ---: | ---: | :---: |
| Out | In | Total |  |
|  | 103 | 193 |  |
| 296 |  |  |  |
|  | 30 | 29 |  |
| 59 |  |  |  |
| 133 | $\mathbf{2 2 2}$ | $\mathbf{3 5 5}$ |  |

Weather conditions:

Clear


## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Count Date: | 0 ct 28, 2021 |
| Period: | $07: 00-10: 00$ |

Peak Hour Data (07:15-08:15)

|  | North Approach Trafalgar Rd N |  |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  |  | East Approach Church St |  |  |  |  |  | West Approach Upper Canada Dr |  |  |  |  |  | Total Vehicl |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | 4 | - | $\stackrel{ }{ }$ |  | Peds | Total | 4 | - | $\stackrel{\rightharpoonup}{r}$ | $\cdots$ | Peds | Total | 4 | - | $\stackrel{\rightharpoonup}{r}$ | ? | Peds | Total |  | $\uparrow$ | $\stackrel{\rightharpoonup}{r}$ |  | Peds | Total | es |
| 07:15 | 2 | 46 | 0 | 0 | 0 | 48 | 0 | 28 | 2 | 0 | 2 | 30 | 2 | 0 | , | 0 | 0 | 3 | 0 | 0 | 2 | 0 | 0 | 2 | 83 |
| 07:30 | 0 | 46 | 0 | 0 | 0 | 46 | 0 | 35 | 1 | 0 | 2 | 36 | 3 | 0 | 1 | 0 | 0 | 4 | 1 | 0 | 3 | 0 | 0 | 4 | 90 |
| 07:45 | 0 | 64 | 1 | 0 | 0 | 65 | 0 | 36 | 1 | 0 | 0 | 37 | 3 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 8 | 0 | 0 | 8 | 113 |
| 08:00 | 0 | 40 | 0 | 0 | 0 | 40 | 2 | 28 | 0 | 0 | 0 | 30 | 2 | 1 | 2 | 0 | 0 | 5 | 0 | 0 | 3 | 0 | 0 | 3 | 78 |
| Grand Total | 2 | 196 | 1 | 0 | 0 | 199 | 2 | 127 | 4 | 0 | 4 | 133 | 10 | 1 | 4 | 0 | 0 | 15 | 1 | 0 | 16 | 0 | 0 | 17 | 364 |
| $\begin{aligned} & \text { Approach } \\ & \% \end{aligned}$ | 1 | 98.5 | 0.5 | 0 |  | - | 1.5 | 95.5 | 3 | 0 |  | - | 66.7 | 6.7 | 26.7 | 0 |  | - | 5.9 | 0 | 94.1 | 0 |  | - |  |
| Totals \% | 0.5 | 53.8 | 0.3 | 0 |  | 54.7 | 0.5 | 34.9 | 1.1 | 0 |  | 36.5 | 2.7 | 0.3 | 1.1 | 0 |  | 4.1 | 0.3 | 0 | 4.4 | 0 |  | 4.7 |  |
| PHF | 0.25 | 0.77 | 0.25 | 0 |  | 0.77 | 0.25 | 0.88 | 0.5 | 0 |  | 0.9 | 0.83 | 0.25 | 0.5 | 0 |  | 0.75 | 0.25 | 0 | 0.5 | 0 |  | 0.53 | 0.81 |
| Cars | 2 | 167 | 1 | 0 |  | 170 | 2 | 97 | 4 | 0 |  | 103 | 10 | 0 | 4 | 0 |  | 14 | 1 | 0 | 16 | 0 |  | 17 | 304 |
| \% Cars | 100 | 85.2 | 100 | 0 |  | 85.4 | 100 | 76.4 | 100 | 0 |  | 77.4 | 100 | 0 | 100 | 0 |  | 93.3 | 100 | 0 | 100 | 0 |  | 100 | 83.5 |
| Trucks | 0 | 29 | 0 | 0 |  | 29 | 0 | 30 | 0 | 0 |  | 30 | 0 | 1 | 0 | 0 |  | 1 | 0 | 0 | 0 | 0 |  | 0 | 60 |
| \% Trucks | 0 | 14.8 | 0 | 0 |  | 14.6 | 0 | 23.6 | 0 | 0 |  | 22.6 | 0 | 100 | 0 | 0 |  | 6.7 | 0 | 0 | 0 | 0 |  | 0 | 16.5 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 4 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - | 4 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 100 | - |  |  |  |  | 0 | - |  |  |  |  | 0 | - |  |

## Peak Hour Diagram

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

Specified Period
From: 15:00:00
To:

One Hour Peak
From:
15:45:00
To:
16:45:00

Intersection:
Site Code:
Count Date:
Trafalgar Rd N \& Upper Canada Dr-Church St 2121900002
Oct 28, 2021




## Peds: 0

Upper Canada Dr


Peds: 5

Trafalgar RdN

Church St


Weather Clear
conditions:

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafagar Rd N \& Upper Canada Dr-Church St |
| :--- | :--- |
| Site Code: | 2121900002 |
| Count Date: | 0 ct 28, 2021 |
| Period: | $15: 00-18: 00$ |

Peak Hour Data (15:45-16:45)

|  | North Approach Trafalgar Rd N |  |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  |  | East Approach Church St |  |  |  |  |  | West Approach Upper Canada Dr |  |  |  |  |  | Total Vehicl |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | 4 | - | $\rightarrow$ |  | Peds | Total | 4 | - | $\stackrel{\rightharpoonup}{r}$ |  | Peds | Total | 4 | - |  |  | Peds | Total | - | 令 |  |  | Peds | Total | es |
| 15:45 | 0 | 61 | 1 | 0 | 0 | 62 | 1 | 66 | 8 | 0 | 0 | 75 | 6 | 0 | 2 | 0 | 0 | 8 | 0 | 0 | 1 | 0 | 0 | 1 | 146 |
| 16:00 | 0 | 52 | 0 | 0 | 0 | 52 | 2 | 82 | 2 | 0 | 5 | 86 | 4 | 0 | 2 | 0 | 1 | 6 | 0 | 0 | 1 | 0 | 0 | 1 | 145 |
| 16:15 | 4 | 52 | 0 | 0 | 0 | 56 | 6 | 82 | 2 | 0 | 0 | 90 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 147 |
| 16:30 | 1 | 35 | 0 | 0 | 0 | 36 | 5 | 85 | 5 | 0 | 0 | 95 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 132 |
| Grand Total | 5 | 200 | 1 | 0 | 0 | 206 | 14 | 315 | 17 | 0 | 5 | 346 | 12 | 0 | 4 | 0 | 1 | 16 | 0 | 0 | 2 | 0 | 0 | 2 | 570 |
| $\begin{gathered} \text { Approach } \\ \% \end{gathered}$ | 2.4 | 97.1 | 0.5 | 0 |  | - | 4 | 91 | 4.9 | 0 |  | - | 75 | 0 | 25 | 0 |  | - | 0 | 0 | 100 | 0 |  | - |  |
| Totals \% | 0.9 | 35.1 | 0.2 | 0 |  | 36.1 | 2.5 | 55.3 | 3 | 0 |  | 60.7 | 2.1 | 0 | 0.7 | 0 |  | 2.8 | 0 | 0 | 0.4 | 0 |  | 0.4 |  |
| PHF | 0.31 | 0.82 | 0.25 | 0 |  | 0.83 | 0.58 | 0.93 | 0.53 | 0 |  | 0.91 | 0.5 | 0 | 0.5 | 0 |  | 0.5 | 0 | 0 | 0.5 | 0 |  | 0.5 | 0.97 |
| Cars | 5 | 182 | 1 | 0 |  | 188 | 14 | 294 | 16 | 0 |  | 324 | 11 | 0 | 4 | 0 |  | 15 | 0 | 0 | 2 | 0 |  | 2 | 529 |
| \% Cars | 100 | 91 | 100 | 0 |  | 91.3 | 100 | 93.3 | 94.1 | 0 |  | 93.6 | 91.7 | 0 | 100 | 0 |  | 93.8 | 0 | 0 | 100 | 0 |  | 100 | 92.8 |
| Trucks | 0 | 18 | 0 | 0 |  | 18 | 0 | 21 | 1 | 0 |  | 22 | 1 | 0 | 0 | 0 |  | 1 | 0 | 0 | 0 | 0 |  | 0 | 41 |
| \% Trucks | 0 | 9 | 0 | 0 |  | 8.7 | 0 | 6.7 | 5.9 | 0 |  | 6.4 | 8.3 | 0 | 0 | 0 |  | 6.3 | 0 | 0 | 0 | 0 |  | 0 | 7.2 |
| Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |
| Peds |  |  |  |  | 0 | - |  |  |  |  | 5 | - |  |  |  |  | 1 | - |  |  |  |  | 0 | - | 6 |
| \% Peds |  |  |  |  | 0 | - |  |  |  |  | 83.3 | - |  |  |  |  | 16.7 | - |  |  |  |  | 0 | - |  |

Traffic Monitoring • Services \& Products

## Project \#21-219 - Candevcon Limited

## Intersection Count Report

Intersection: Trafalgar Rd N \& Howe St<br>Municipality: Erin<br>Count Date: Oct 28,2021<br>Site Code:<br>2121900001<br>Count Categories: Cars, Trucks, Bicycles, Pedestrians<br>Count Period: $\quad 07: 00-10: 00,15: 00-18: 00$<br>Weather:<br>Clear

# Traffic Count Map 

Ontario Traffic Inc.
Traffic Monitoring • Services \& Products

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Howe St
2121900001
Erin
Oct 28, 2021


## Traffic Count Summary

Trafalgar Rd N \& Howe St
2121900001
Erin
Oct 28, 2021

## Trafalgar Rd N - Traffic Summary

| Hour | North Approach Totals |  |  |  |  |  | South Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 0 | 155 | 0 | 0 | 155 | 0 | 0 | 119 | 1 | 0 | 120 | 0 | 275 |
| 08:00-09:00 | 3 | 166 | 0 | 0 | 169 | 0 | 0 | 125 | 1 | 0 | 126 | 0 | 295 |
| 09:00-10:00 | 0 | 112 | 0 | 0 | 112 | 0 | 0 | 132 | 4 | 0 | 136 | 0 | 248 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 7 | 194 | 0 | 0 | 201 | 0 | 0 | 250 | 5 | 0 | 255 | 0 | 456 |
| 16:00-17:00 | 5 | 183 | 0 | 0 | 188 | 0 | 0 | 325 | 5 | 0 | 330 | 0 | 518 |
| 17:00-18:00 | 3 | 163 | 0 | 0 | 166 | 0 | 0 | 308 | 8 | 0 | 316 | 0 | 482 |
| GRAND TOTAL | 18 | 973 | 0 | 0 | 991 | 0 | 0 | 1259 | 24 | 0 | 1283 | 0 | 2274 |

## Traffic Count Summary

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Howe St
2121900001
Erin
Oct 28, 2021

Howe St - Traffic Summary

| Hour | East Approach Totals |  |  |  |  |  | West Approach Totals |  |  |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Includes Cars, Trucks, Bicycles |  |  |  |  |  | Includes Cars, Trucks, Bicycles |  |  |  |  |  |  |
|  | Left | Thru | Right | U-Turn | Total | Peds | Left | Thru | Right | U-Turn | Total | Peds |  |
| 07:00-08:00 | 4 | 0 | 2 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 08:00-09:00 | 1 | 0 | 3 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 09:00-10:00 | 3 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 15:00-16:00 | 7 | 0 | 4 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |
| 16:00-17:00 | 2 | 0 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 17:00-18:00 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| GRAND TOTAL | 20 | 0 | 14 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 34 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | 1 |  |  | Total |  |  |
| 07:00 | 0 | 35 | 0 | 0 | 35 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 40 | 0 | 0 | 40 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 0 | 34 | 0 | 0 | 34 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 25 | 0 | 0 | 25 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 2 | 33 | 0 | 0 | 35 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 0 | 33 | 0 | 0 | 33 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 0 | 36 | 0 | 0 | 36 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 1 | 38 | 0 | 0 | 39 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 20 | 0 | 0 | 20 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 0 | 20 | 0 | 0 | 20 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 28 | 0 | 0 | 28 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 0 | 23 | 0 | 0 | 23 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 3 | 365 | 0 | 0 | 368 | 0 | 68 | 0 | 0 | 68 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

North Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | - | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + | $\stackrel{\rightharpoonup}{\text { Pr }}$ |  | Total | 4 | + |  |  | Total |  |  |
| 15:00 | 3 | 41 | 0 | 0 | 44 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 3 | 42 | 0 | 0 | 45 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 1 | 25 | 0 | 0 | 26 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 0 | 58 | 0 | 0 | 58 | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 1 | 42 | 0 | 0 | 43 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 2 | 56 | 0 | 0 | 58 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 2 | 34 | 0 | 0 | 36 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 0 | 44 | 0 | 0 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 1 | 39 | 0 | 0 | 40 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 2 | 44 | 0 | 0 | 46 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 0 | 40 | 0 | 0 | 40 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 0 |  | 0 | 0 | 34 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | , | 0 | 0 |  | 0 |
| SUBTOTAL | 15 | 499 | 0 | 0 | 514 | 0 | 41 | 0 | 0 | 41 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 18 | 864 | 0 | 0 | 882 | 0 | 109 | 0 | 0 | 109 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  | ? | Total | 4 | + |  | ? | Total | 4 | + |  |  | Total |  |  |
| 07:00 | 0 | 20 | 0 | 0 | 20 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 22 | 0 | 0 | 22 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 0 | 32 | 1 | 0 | 33 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 23 | 0 | 0 | 23 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 0 | 26 | 1 | 0 | 27 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 0 | 23 | 0 | 0 | 23 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 0 | 27 | 0 | 0 | 27 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 0 | 26 | 0 | 0 | 26 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 24 | 0 | 0 | 24 | 0 | 9 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:15 | 0 | 23 | 0 | 0 | 23 | 0 | 6 | 1 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 32 | 0 | 0 | 32 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 0 | 23 | 3 | 0 | 26 | 0 | 10 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBtotal | 0 | 301 | 5 | 0 | 306 | 0 | 75 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

South Approach - Trafalgar Rd N

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | - | - | $\stackrel{\rightharpoonup}{r}$ | $?$ | Total | - | - |  |  | Total | - | $\uparrow$ |  |  | Total |  |  |
| 15:00 | 0 | 58 | 0 | 0 | 58 | 0 | 10 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 0 | 55 | 0 | 0 | 55 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 0 | 53 | 1 | 0 | 54 | 0 | 7 | 1 | 0 | 8 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 0 | 57 | 2 | 0 | 59 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 0 | 77 | 1 | 0 | 78 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 0 | 78 | 3 | 0 | 81 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 0 | 74 | 1 | 0 | 75 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 0 | 80 | 0 | 0 | 80 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 0 | 76 | 4 | 0 | 80 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 0 | 89 | 2 | 0 | 91 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 0 | 66 | 1 | 0 | 67 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 0 | 66 | 1 | 0 | 67 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 0 | 829 | 16 | 0 | 845 | 0 | 54 | 2 | 0 | 56 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 0 | 1130 | 21 | 0 | 1151 | 0 | 129 | 3 | 0 | 132 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Municipality: | Erin |
| Count Date: | Oct 28, 2021 |

## East Approach - Howe St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + |  |  | Total | 4 | + |  |  | Total | 4 | + |  | ? | Total |  |  |
| 07:00 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:15 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:30 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 07:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:00 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:15 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:30 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 08:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | - |
| 09:15 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 09:45 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 6 | 0 | 6 | 0 | 12 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Traffic Count Data

## Ontario Traffic Inc.

Intersection:
Site Code:
Municipality:
Count Date:

Trafalgar Rd N \& Howe St
2121900001
Erin
Oct 28, 2021

## East Approach - Howe St

| Start Time | Cars |  |  |  |  | Trucks |  |  |  |  | Bicycles |  |  |  |  | Total Peds |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | + | $\stackrel{\rightharpoonup}{1}$ | 2 | Total | 4 | + | $\stackrel{\rightharpoonup}{1}$ |  | Total | 4 | 贯 |  |  | Total |  |  |
| 15:00 | 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:15 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:30 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 15:45 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:15 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:30 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 16:45 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:00 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:15 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:30 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| 17:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| SUBTOTAL | 12 | 0 | 8 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 |
| $\begin{aligned} & \text { GRAND } \\ & \text { TOTAL } \end{aligned}$ | 18 | 0 | 14 | 0 | 32 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 |

## Peak Hour Diagram

## Ontario Traffic Inc． <br> Traffic Monitoring • Services \＆Products

07：00：00
10：00：00

Specified Period
From：
To： ，

One Hour Peak
From：
08：00：00
To：
09：00：00

## Intersection：

Site Code：
Trafalgar Rd N \＆Howe St
2121900001
Count Date：

Weather conditions：Clear


Peds： 0


Peds： 0

| Totals | $1 \quad \rightarrow$ |  |  |
| :---: | :---: | :---: | :---: |
|  | 125 | 1 | 0 |
| 日 | 102 | 1 | 0 |
| 50 | 23 | 0 | 0 |
| O－ | 0 | 0 | 0 |

Trafalgar Rd $\mathbf{N}$
Howe St

|  | Totals | $\square$ | 0.0 | お8 |
| :---: | :---: | :---: | :---: | :---: |
| C | 0 | 0 | 0 | 0 |
| 官 | 3 | 3 | 0 | 0 |
| $F$ | 1 | 0 | 1 | 0 |


| South Approach |  |  |  |
| ---: | ---: | ---: | ---: |
|  | Out | In | Total |
|  | 103 | 140 | 243 |
| 0 | 23 | 27 | 50 |
|  | 0 | 0 | 0 |
| 126 | $\mathbf{1 6 7}$ | $\mathbf{2 9 3}$ |  |

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Count Date: | Oct 28, 2021 |
| Period: | $07: 00-10: 00$ |

Peak Hour Data (08:00-09:00)

|  | North Approach Trafalgar Rd N |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  | East Approach Howe St |  |  |  |  | West Approach |  |  |  | Total Vehicl es |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time |  |  |  | Peds | Total | - 1 |  |  |  | Total |  |  |  | Peds | Total |  | $\uparrow \quad$ ? | Peds | Total |  |
| 08:00 | 2 | 38 | 0 | 0 | 40 | 35 | 1 | 0 | 0 | 36 | 0 | 1 | 0 | 0 | 1 |  |  | 0 |  | 77 |
| 08:15 | 0 | 40 | 0 | 0 | 40 | 30 | 0 | 0 | 0 | 30 | 1 | 0 | 0 | 0 | 1 |  |  | 0 |  | 71 |
| 08:30 | 0 | 44 | 0 | 0 | 44 | 30 | 0 | 0 | 0 | 30 | 0 | 2 | 0 | 0 | 2 |  |  | 0 |  | 76 |
| 08:45 | 1 | 44 | 0 | 0 | 45 | 30 | 0 | 0 | 0 | 30 | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  | 75 |
| Grand Total | 3 | 166 | 0 | 0 | 169 | 125 | 1 | 0 | 0 | 126 | 1 | 3 | 0 | 0 | 4 |  |  | 0 | 0 | 299 |
| Approach \% | 1.8 | 98.2 | 0 |  | - | 99.2 | 0.8 | 0 |  | - | 25 | 75 | 0 |  | - |  |  |  | - |  |
| Totals \% | 1 | 55.5 | 0 |  | 56.5 | 41.8 | 0.3 | 0 |  | 42.1 | 0.3 | 1 | 0 |  | 1.3 |  |  |  | 0 |  |
| PHF | 0.38 | 0.94 | 0 |  | 0.94 | 0.89 | 0.25 | 0 |  | 0.88 | 0.25 | 0.38 | 0 |  | 0.5 |  |  |  | 0 | 0.97 |
| Cars | 3 | 140 | 0 |  | 143 | 102 | 1 | 0 |  | 103 | 0 | 3 | 0 |  | 3 |  |  |  | 0 | 249 |
| \% Cars | 100 | 84.3 | 0 |  | 84.6 | 81.6 | 100 | 0 |  | 81.7 | 0 | 100 | 0 |  | 75 |  |  |  | 0 | 83.3 |
| Trucks | 0 | 26 | 0 |  | 26 | 23 | 0 | 0 |  | 23 | 1 | 0 | 0 |  | 1 |  |  |  | 0 | 50 |
| \% Trucks | 0 | 15.7 | 0 |  | 15.4 | 18.4 | 0 | 0 |  | 18.3 | 100 | 0 | 0 |  | 25 |  |  |  | 0 | 16.7 |
| Bicycles | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 0 |
| Peds |  |  |  | 0 | - |  |  |  | 0 | - |  |  |  | 0 | - |  |  | 0 | - | 0 |
| \% Peds |  |  |  | 0 | - |  |  |  | 0 | - |  |  |  | 0 | - |  |  | 0 | - |  |

## Peak Hour Diagram

# Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products 

Specified Period
From: 15:00:00
To:
18:00:00

One Hour Peak
From:
15:45:00
To:
16:45:00

## Intersection:

Site Code:
Count Date:

Trafalgar Rd N \& Howe St
2121900001
Oct 28, 2021

Peds: 0

Weather conditions: Clear

| North Approach |  |  | Trafalgar Rd N |  |  |  | East Approach |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Out | In | Total | अ) | 0 | 0 | 0 |  | Out | In | Total |
| ( 195 | 286 | 481 | $5 \cdot 1$ | 18 | 0 | 0 | 日 | 3 | 12 | 15 |
| -6 18 | 19 | 37 | $\theta$ |  | 5 | 0 | 510 | 0 | 1 | 1 |
| -\% 0 | 0 | 0 | Totals | 208 | 5 | 0 | O 6 | 0 | 0 | 0 |
| 213 | 305 | 518 |  | $\downarrow$ |  |  |  | 3 | 13 | 16 |



| Totals | $305$ | $\Rightarrow$ ? |  |
| :---: | :---: | :---: | :---: |
| 日 | 286 | 7 | 0 |
| 50 | 19 | 1 | 0 |
| \% | 0 | 0 |  |

Trafalgar Rd N

Howe St

\left.| South Approach |  |  |  |
| ---: | ---: | ---: | :---: |
| Out | In | Total |  |
| 293 | 193 | 486 |  |
| 20 | 18 | 38 |  |
|  | 0 | 0 |  |$\right)$

## Peak Hour Summary

## Ontario Traffic Inc. <br> Traffic Monitoring • Services \& Products

| Intersection: | Trafalgar Rd N \& Howe St |
| :--- | :--- |
| Site Code: | 2121900001 |
| Count Date: | Oct 28, 2021 |
| Period: | 15:00-18:00 |

Peak Hour Data (15:45-16:45)

|  | North Approach Trafalgar Rd N |  |  |  |  | South Approach Trafalgar Rd N |  |  |  |  | East Approach Howe St |  |  |  |  | West Approach |  |  |  | Total Vehicl es |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time |  |  |  |  | Total | - 1 |  |  |  | Total |  |  |  | Peds | Total |  | $\uparrow \quad$ ? | Peds | Total |  |
| 15:45 | 0 | 69 | 0 | 0 | 69 | 61 | 3 | 0 | 0 | 64 | 1 | 0 | 0 | 0 | 1 |  |  | 0 |  | 134 |
| 16:00 | 1 | 46 | 0 | 0 | 47 | 84 | 1 | 0 | 0 | 85 | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  | 132 |
| 16:15 | 2 | 58 | 0 | 0 | 60 | 79 | 3 | 0 | 0 | 82 | 1 | 0 | 0 | 0 | 1 |  |  | 0 |  | 143 |
| 16:30 | 2 | 35 | 0 | 0 | 37 | 81 | 1 | 0 | 0 | 82 | 1 | 0 | 0 | 0 | 1 |  |  | 0 |  | 120 |
| Grand Total | 5 | 208 | 0 | 0 | 213 | 305 | 8 | 0 | 0 | 313 | 3 | 0 | 0 | 0 | 3 |  |  | 0 | 0 | 529 |
| Approach \% | 2.3 | 97.7 | 0 |  | - | 97.4 | 2.6 | 0 |  | - | 100 | 0 | 0 |  | - |  |  |  | - |  |
| Totals \% | 0.9 | 39.3 | 0 |  | 40.3 | 57.7 | 1.5 | 0 |  | 59.2 | 0.6 | 0 | 0 |  | 0.6 |  |  |  | 0 |  |
| PHF | 0.63 | 0.75 | 0 |  | 0.77 | 0.91 | 0.67 | 0 |  | 0.92 | 0.75 | 0 | 0 |  | 0.75 |  |  |  | 0 | 0.92 |
| Cars | 5 | 190 | 0 |  | 195 | 286 | 7 | 0 |  | 293 | 3 | 0 | 0 |  | 3 |  |  |  | 0 | 491 |
| \% Cars | 100 | 91.3 | 0 |  | 91.5 | 93.8 | 87.5 | 0 |  | 93.6 | 100 | 0 | 0 |  | 100 |  |  |  | 0 | 92.8 |
| Trucks | 0 | 18 | 0 |  | 18 | 19 | 1 | 0 |  | 20 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 38 |
| \% Trucks | 0 | 8.7 | 0 |  | 8.5 | 6.2 | 12.5 | 0 |  | 6.4 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 7.2 |
| Bicycles | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 0 |
| \% Bicycles | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 |  |  |  | 0 | 0 |
| Peds |  |  |  | 0 | - |  |  |  | 0 | - |  |  |  | 0 | - |  |  | 0 | - | 0 |
| \% Peds |  |  |  | 0 | - |  |  |  | 0 | - |  |  |  | 0 | - |  |  | 0 | - |  |

## APPENDIX C

## SIGNAL TIMING PLANS




|  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |
| Detector Rack. . . |  |  |  |  |  |  |  |  |  |
| Type 2 Runs as Type 1. . . . MMU Disable. |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| Diagnostic Enable. . . . . . |  |  |  |  |  |  |  |  |  |
| Peer-Peer Comm Enable. |  |  |  |  |  |  |  |  |  |
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Peer To Peer Addresses . . 255 | 255 | 255 | 255 | 255 | 255 | 255 | 255 | 255 | 255 |
| Port 2: |  |  |  |  |  |  |  |  |  |
| Port 2 Protocol . . . . . . . . Terminal |  |  |  |  |  |  |  |  |  |
| Port 2 Enable . . . . . . . . . YES |  |  |  |  |  |  |  |  |  |
| AB3418 Address. . |  |  |  |  |  |  |  |  |  |
| AB3418 Group Address. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 Response Delay . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 Single Flag Enable . . . No |  |  |  |  |  |  |  |  |  |
| AB3418 Drop-Out Time. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 TOD SF Select. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| Data Rate . . . . . . . . . . . 1200 bps |  |  |  |  |  |  |  |  |  |
| Data, Parity, Stop. . . . . . . 8, 0, 1 |  |  |  |  |  |  |  |  |  |
| Port 3: |  |  |  |  |  |  |  |  |  |
| Port 3 Protocol . . . . . . . . Telemetry |  |  |  |  |  |  |  |  |  |
| Port 3 Enable . . . . . . . . . NO |  |  |  |  |  |  |  |  |  |
| Telemetry Address . . . . . . . 0 |  |  |  |  |  |  |  |  |  |
| System Detector 9-16 Address. . 0 |  |  |  |  |  |  |  |  |  |
| Telemetry Response Delay. . . . 7800 |  |  |  |  |  |  |  |  |  |
| AB3418 Address. . . . . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 Group Address. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 Response Delay . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 Single Flag Enable . . . No |  |  |  |  |  |  |  |  |  |
| AB3418 Drop-Out Time. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| AB3418 TOD SF Select. . . . . . 0 |  |  |  |  |  |  |  |  |  |
| Duplex. . . . . . . . . . . . . Full |  |  |  |  |  |  |  |  |  |
| Data Rate . . . . . . . . . . . 1200 bps |  |  |  |  |  |  |  |  |  |
| Data, Parity, Stop. | 8, 0 | , 1 |  |  |  |  |  |  |  |



Supervisor Access Code. . . **** Data Change Access Code . . ****

MMU Compatibility Program (Info Only)


Phase

| Minimum Green | 5 | 30 | 5 | 15 | 5 | 30 | 5 | 15 | 5 | 5 | 5 | 5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bike Min Green | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Cond Serv Min Grn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Walk | 0 | 5 | 0 | 5 | 0 | 5 | 0 | 5 | 0 | 10 | 0 | 10 |
| Ped Clearance | 0 | 7 | 0 | 7 | 0 | 7 | 0 | 7 | 0 | 16 | 0 | 16 |
| Veh Extension | 5.0 | 5.0 | 5.0 | 4.0 | 5.0 | 5.0 | 5.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Alt Veh Exten | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Max Extension | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Max 1 | 35 | 35 | 35 | 27 | 35 | 35 | 35 | 27 | 35 | 35 | 35 | 35 |
| Max 2 | 40 | 15 | 40 | 40 | 40 | 15 | 40 | 40 | 40 | 40 | 40 | 40 |
| Max 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Det. Fail Max | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Yellow Change | 3.0 | 5.5 | 3.0 | 5.5 | 3.0 | 5.5 | 3.0 | 5.5 | 3.0 | 3.0 | 3.0 | 3.0 |
| Red Clearance | 1.0 | 2.0 | 1.0 | 2.0 | 1.0 | 2.0 | 1.0 | 2.0 | 1.0 | 1.0 | 1.0 | 1.0 |
| Red Revert | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Act. B4 Init | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


| Sec/Actuation 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Max Initial | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Time B4 Reduction | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Cars Waiting | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Time To Reduce | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Minimum Gap $0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0 \quad 0.0$

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| Phase | Phase |  |  | Cannot | Serve With |  | Phase <br> 4 |  | 2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12 | 11 | 10 | 98 | 76 | 65 |  | 3 |  |
| 1. | . | . | . | . . | - | - | . | . | - |
| 2. | - | - | - | . . | - | - | . | - |  |
| 3. | . | . | . | . . | - | - | . |  |  |
| 4. | . | . | . | . . | . . | . . |  |  |  |
| 5. | . | . | . | - | . . | . |  |  |  |
| 6. | . | . | . | . . | . |  |  |  |  |
| 7. | . | . | . | . . |  |  |  |  |  |
| 8. | - | - | - | - |  |  |  |  |  |
| 9. | . | . | . |  |  |  |  |  |  |
| 10. . | . | . |  |  |  |  |  |  |  |
| 11. |  |  |  |  |  |  |  |  |  |

Ped Start Phase Carry Over Phase

1

2

3

4

5

6

7

8

9

10

11

12

0

0

0

0

0

0

0

0

0

0

0

0

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Vehicle/Ped Phase as Overlap

| Ped | Ped Phase As Overlap |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Consists of Ped Phases |  |  |  |  |  |  |  |  |  |  |  |
| Ovlap |  |  |  |  |  |  |  |  |  |  |  |  |
| Phase | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| 1 | - | . | . | . | . | . | . | . | . | - | - | - |
| 2 | - | - | - | - | - | - | - | - | - | - | - | - |
| 3 | - | - | - | - | - | - | - | - | - | - | - | - |
| 4 | - | - | - | - | - | - | - | - | - | - | - | - |
| 5 | - | - | - | - | - | - | - | - | - | - | - | - |
| 6 | - | - | - | - | - | - | - | - | - | - | - | - |
| 7 | - | - | - | - | - | - | . | - | - | - | - | - |
| 8 | - | - | - | - | - | - | . | - | - | - | - | - |
| 9 | - | - | - | - | - | - | - | - | - | - | - | - |
| 10 | - | - | - | - | - | - | - | - | - | - | - | - |
| 11 | - | - | - | - | - | - | - | - | - | - | - | - |
| 12 | - | - | - | - | - | - | - | - | - | - | - | - |
|  |  |  |  | Veh | Pha | - | Ov | clap |  |  |  |  |
| Veh |  |  |  | ns | sts | of | eh | has |  |  |  |  |
| Ovlap |  |  |  |  |  |  |  |  |  |  |  |  |
| Phase | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| 1 | X | . | - | - | - | - | - | - | . | - | - | - |
| 2 | . | X | . | - | - | - | - | - | . | - | - | - |
| 3 | . | . | X | - | - | - | - | - | . | . | . | . |
| 4 | - | . | . | X | - | - | - | - | . | - | . | - |
| 5 | - | . | - | . | X | - | - | - | . | - | . | - |
| 6 | - | - | - | - | - | X | - | - | - | - | - | - |
| 7 | . | . | . | - | . | . | X | . | . | . | . | . |
| 8 | - | . | - | - | - | - | . | X | . | - | . | - |
| 9 | - | - | - | - | - | - | - | - | X | - | - | - |
| 10 | - | - | - | - | - | . | - | - | . | X | - | - |
| 11 | - | - | - | - | - | - | - | - | - | - | X | - |
| 12 | - | - | - | - | - | - | - | - | - | - | - | X |





Power Start, Remote Flash

Initialization Interval:
Power Start . . . . . . . . Yellow
External Start. . . . . . . Yellow
Power Start All Red Time. . 0
Power Start Flash Time. . . 0
Remote Flash Options:
Out of Flash Yellow . . . . NO
Out of Flash All Red. . . . NO
Minimum Recall. . . . . . . NO
Alternate Flash . . . . . . NO
Flash Thru Load Switches. . NO
Cycle Through Phases. . . . NO

|  | Phase |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| Guaranteed Passage | . | . | . | . | . | . | . | . | . | . | . | . |
| Call To NonActuated 1 | - | X | . | . | - | X | . | - | - | - | - | . |
| Call To NonActuated 2 | - | . | - | X | - | - | . | X | - | - | - | - |
| Dual Entry. | . | X | . | X | . | X | . | X | . | X | - | X |
| Conditional Service | X | . | X | . | X | . | X | . | X | . | X | . |
| Conditional Reservice | . | . | . | . | . | - | . | - | . | - | . |  |
| Actuated Rest in Walk | - | . | . | - | - | - | - | - | - | - | - |  |
| Flashing Walk | - | . | . | . | - | . | . | - | . | - |  |  |

Dual Entry. . . . . . . . . . ON Backup Protection Group 1
Conditional Service. . . . . . OFF
Ped Clearance Protection. . . . OF

|  | Five Section Left Turn Control |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Phases: | $5-2$ | $7-4$ | $1-6$ | $3-8$ | $11-10$ |

Left Turn Head.


Dimming:

|  |  |  |  |  |  |  | d | it |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 |
| Green/Walk. | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |
| Yellow/Ped Clear | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |
| Red/Don't Walk. | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |


|  | Locking | Log | Timers |  | Don't Res |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Det. | Memory | Enable | Extend | Delay | Extend | Type |
| 1 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 2 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 3 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 4 | NO | NO | 0.0 | 7 | - | 1 - Extend/Delay |
| 5 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 6 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 7 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 8 | NO | NO | 0.0 | 7 | - | 1 - Extend/Delay |
| 9 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 10 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 11 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 12 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 13 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 14 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 15 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 16 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 17 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 18 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 19 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 20 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 21 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 22 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 23 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 24 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 25 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 26 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 27 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 28 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 29 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 30 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 31 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 32 | NO | NO | 0.0 | 0 |  | 0 - Normal |

Detector Names

| Det | $1:$ | Detector |  |
| :---: | :---: | :---: | :---: |
| et | 2 | Detector | 2 |
| et | 3 | Detect | 3 |
| et | 4 | Detector |  |
| t | 5 | Detector | 5 |
| t | 6 | Detecto |  |
| t | 7 | Detector | 7 |
| t | 8: | Detecto |  |
| et | 9 | Detector |  |
| t | 10 | Detector |  |
| t | 11: | Detector |  |
|  | 12: | Detector |  |
| et | 13: | Detector |  |
| et | 14: | Detector |  |
| et | $15:$ | Detector |  |
| et |  | Detector |  |

```
Det 17: Detector 17
Det 18: Detector 18
Det 19: Detector 19
Det 20: Detector 20
Det 21: Detector 21
Det 22: Detector 22
Det 23: Detector 23
Det 24: Detector 24
Det 25: Detector 25
Det 26: Detector 26
Det 27: Detector 27
Det 28: Detector 28
Det 29: Detector 29
Det 30: Detector 30
Det 31: Detector 31
Det 32: Detector 32
```

Det 18: Detector 18
Det 19: Detector 19
Det 20: Detector 20
Det 21: Detector 21
Det 22: Detector 22
Det 23: Detector 23
Det 24: Detector 24
Det 25: Detector 25
Det 26: Detector 26
Det 27: Detector 27
Det 28: Detector 28
Det 29: Detector 29
Det 30: Detector 30
Det 31: Detector 31
Det 32: Detector 32

| 33 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 34 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 35 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 36 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 37 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 38 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 39 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 40 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 41 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 42 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 43 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 44 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 45 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 46 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 47 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 48 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 49 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 50 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 51 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 52 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 53 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 54 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 55 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 56 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 57 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 58 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 59 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 60 | NO | NO | 0.0 | 0 | - | 0 - Normal |
| 61 | NO | NO | 0.0 | 0 |  | 0 - Normal |
| 62 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 63 | NO | NO | 0.0 | 0 | . | 0 - Normal |
| 64 | NO | NO | 0.0 | 0 | . | 0 - Normal |

## Detector Names



Det 49: Detector 49
Det 50: Detector 50
Det 51: Detector 51
Det 52: Detector 52
Det 53: Detector 53
Det 54: Detector 54
Det 55: Detector 55
Det 56: Detector 56
Det 57: Detector 57
Det 58: Detector 58
Det 59: Detector 59
Det 60: Detector 60
Det 61: Detector 61
Det 62: Detector 62
Det 63: Detector 63
Det 64: Detector 64

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| Det. | Phase |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| 1 | - | . | . | . | . | . | . | . | . | . | - | . |
| 2 | - | - | - | - | . | - | - | - | - | - | - | - |
| 3 | - | . | - | . | - | - | - | - | - | - | . | - |
| 4 | . | . | - | . | . | . | . | . | . | - | . | - |
| 5 | - | . | . | . | . | . | . | . | . | . | . | - |
| 6 | - | . | - | . | . | . | . | . | . | . | . | - |
| 7 | - | . | - | . | . | . | . | . | . | . | . | . |
| 8 | - | . | - | . | . | . | . | . | . | . | . | - |
| 9 | - | . | - | . | . | . | - | . | . | . | . | . |
| 10 | - | . | - | . | . | . | - | . | . | . | . | - |
| 11 | - | - | - | . | . | - | - | . | . | . | . | - |
| 12 | - | . | - | . | . | . | . | . | . | . | . | - |
| 13 | . | . | . | . | . | . | . | . | . | . | . | . |
| 14 | . | . | . | . | . | . | . | - | . | . | . | - |
| 15 | - | . | - | - | . | - | . | - | . | - | - | . |
| 16 | - | - | - | - | - | - | - | - | . | - | - | . |
| 17 | - | - | - | - | - | - | . | - | - | - | - | . |
| 18 | - | . | - | - | . | . | - | - | . | . | . | . |
| 19 | - | . | - | - | . | . | - | - | . | . | . | . |
| 20 | - | . | - | - | . | . | - | - | . | . | . | . |
| 21 | - | - | - | - | - | - | - | - | . | - | . | . |
| 22 | - | . | - | - | - | . | - | - | . | . | . | - |
| 23 | - | . | - | - | - | . | - | - | . | - | - | - |
| 24 | . | . | - | - | . | . | - | - | . | - | - | - |
| 25 | - | . | - | - | . | . | - | - | - | . | . | . |
| 26 | . | . | - | - | - | - | - | - | . | - | - | - |
| 27 | . | . | - | - | - | . | - | - | . | . | . | . |
| 28 | . | . | - | - | - | - | - | - | - | . | - | . |
| 29 | . | . | - | - | - | - | - | - | - | - | - | . |
| 30 | . | . | . | - | - | . | - | - | - | . | - | . |
| 31 | - | - | - | - | - | - | - | . | . | - | - | - |
| 32 |  |  |  |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Det. | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| 33 | - | - | . | - | - | - | - | - | . | . | - | - |
| 34 | - | - | . | - | - | - | - | - | . | . | - | . |
| 35 | - | - | . | - | - | - | - | - | . | . | - | . |
| 36 | - | . | . | - | - | - | - | . | . | . | - | . |
| 37 | - | - | . | - | - | - | - | - | . | . | - | . |
| 38 | - | - | . | - | - | - | - | - | . | . | - | . |
| 39 | - | - | . | - | - | - | - | - | . | . | - | . |
| 40 | - | - | - | - | - | - | - | - | . | . | $\cdot$ | . |
| 41 | - | - | . | - | . | - | - | . | . | . | $\cdot$ | . |
| 42 | - | . | - | - | . | - | - | . | - | . | - | . |
| 43 | - | . | - | - | . | - | - | . | - | . | - | - |
| 44 | - | - | . | - | . | - | - | . | - | . | - | . |
| 45 | - | - | . | - | . | - | - | . | - | . | - | . |
| 46 | - | - | . | - | - | . | - | . | - | . | $\cdot$ | - |
| 47 | - | - | . | - | - | - | - | - | . | . | - | - |
| 48 | - | - | . | - | - | - | - | - | - | . | - | - |
| 49 | - | . | - | . | . | - | - | . | - | - | - | . |
| 50 | - | - | - | - | - | - | - | - | - | . | - | - |
| 51 | - | - | - | - | - | - | - | - | - | . | - | . |
| 52 | - | - | - | - | - | - | - | - | - | . | - | - |
| 53 | - | - | - | - | - | - | - | - | - | - | - | - |
| 54 | - | - | - | - | - | - | - | - | - | . | - | - |
| 55 | - | - | - | - | - | - | - | - | - | . | - | - |
| 56 | - | - | . | - | - | - | - | - | - | . | - | - |
| 57 | - | - | - | - | - | - | - | - | - | . | - | . |
| 58 | - | - | - | - | - | - | - | - | - | . | - | - |
| 59 | - | - | - | - | - | - | - | - | - | . | - | . |
| 60 | . | - | . | - | . | - | - | - | - | . | - | . |
| 61 | - | - | - | - | . | - | - | - | - | . | - | . |
| 62 | - | - | . | - | . | - | . | - | - | . | - | . |
| 63 | - | - | . | - | . | - | . | - | . | . | - | . |
| 64 | . | - | . | . | . | - | - | - | . | . | . | . |


*NOTE: System master designations cross referenced to local system detector numbers are:

SDA1 $=1 \& 9$
SDA2 $=2 \& 10$
SDB1 $=3 \& 11$
SDB2 $=4 \& 12$
SDC1 $=5 \& 13$
SDC2 $=6 \& 14$
SDD1 $=7 \& 15$
SDD2 = $8 \& 16$

| Plan | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 5 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 6 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 7 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 8 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| $\quad$ Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |  |
| *Fail Action | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |


| Plan | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 4 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 5 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 6 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 7 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 8 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| $\quad$ Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |  |
| *Fail Action | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |

*NOTE: $0=$ No Action, $1=$ Min Recall, $2=$ Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

| Plan |  | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 4 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 5 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 6 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 7 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  | ail Action | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |  |


| Plan | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 4 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 5 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 6 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 7 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 8 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| $\quad$ Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |  |
| *Fail Action | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |

*NOTE: $0=$ No Action, $1=$ Min Recall, $2=$ Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

| Plan |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 4 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 5 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 6 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 7 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 8 | Diagnostic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Scaling | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |


| Diagnostic | *No-Activity | *Max Presence |  |
| :---: | :---: | :---: | :---: |
| Number | Diagnostic Interval | Diagnostic Interval | Erratic Counts |
| 1 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |
| 3 | 0 | 0 | 0 |
| 4 | 0 | 0 | 0 |
| 5 | 0 | 0 | 0 |
| 6 | 0 | 0 | 0 |
| 7 | 0 | 0 | 0 |
| 8 | 0 | 0 | 0 |
| 9 | 0 | 0 | 0 |
| 10 | 0 | 0 | 0 |
| 11 | 0 | 0 | 0 |
| 12 | 0 | 0 | 0 |
| 13 | 0 | 0 | 0 |
| 14 | 0 | 0 | 0 |
| 15 | 0 | 0 | 0 |
| 16 | 0 | 0 | 0 |
| 17 | 0 | 0 | 0 |
| 18 | 0 | 0 | 0 |
| 19 | 0 | 0 | 0 |
| 20 | 0 | 0 | 0 |
| 21 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |
| 23 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |

NOTE: Scaling is specified in each detector diagnostic plan.


[^10]

Wellington County 24-36 2 \& 22 Hillsburg 7/4/2013 6:13
Preemptor 1
Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold. . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel .
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0

|  |  |  | Green | Yellow | Red |
| :--- | :--- | :---: | :---: | :---: | :---: |
| Minimum . . . . . . | 0 | 0.0 | 0.0 |  |  |
| Track Clear | . | . | 0 | 0.0 | 0.0 |
| Hold. . . . . . . . |  | 0.0 | 0.0 |  |  |


Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase

Out of Flash Color for Exit Phases
Green
Preemptor 2
Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0

|  | Green | Yellow | Red |
| :---: | :---: | :---: | :---: |
| Minimum | 0 | 0.0 | 0.0 |
| Track Clear | 0 | 0.0 | 0.0 |
| Hold. . |  | 0.0 | 0.0 |


Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase
Out of Flash Color for Exit Phases . . . . Green
Linked Preemptor
0
Preemptor 3
Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0

|  | Green | Yellow | Red |
| :---: | :---: | :---: | :---: |
| Minimum | 0 | 0.0 | 0.0 |
| Track Clear | 0 | 0.0 | 0.0 |
| Hold. . . |  | 0.0 | 0.0 |


Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase
Out of Flash Color for Exit Phases . . . . Green
Linked Preemptor . . . . 0
Preemptor 4
Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark .
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0
Green Yellow Red
Minimum . . . . . . 0 0.0 0.0
Track Clear . . . . 0 0.0 0.0
Hold.
$0.0 \quad 0.0$

Phase/Overlap 1
Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase
Out of Flash Color for Exit Phases . . . . Green
Linked Preemptor

## Preemptor 5

Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0

|  |  |  | Green | Yellow | Red |
| :--- | :--- | :---: | :---: | :---: | :---: |
| Minimum . . . . . . | 0 | 0.0 | 0.0 |  |  |
| Track Clear | . | . | 0 | 0.0 | 0.0 |
| Hold. . . . . . . . |  | 0.0 | 0.0 |  |  |


Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase
Out of Flash Color for Exit Phases . . . . Green
Linked Preemptor . . . . 0
Preemptor 6
Active . . . . . . . . . . . . Det Lock. . . . . . Ped Dark . . . . . .
Priority Preemption. . . . . . Yel-Red To Grn. . . Ped Active
Outputs Only During Hold . . . Flash All Outputs . Zero Ped Clr Time.
Terminate Overlap ASAP . . . . Terminate Phases. . Ped Clr Thru Yel
Don't Override Flash . . . . . Duration Time. . . 0
Flash During Hold. . . . . . . Delay Time . . . . 0
No CVM in Flash. . . . . . . . Inhibit Time . . . 0
Fast Flash Grn on Hold Phase. . Min Ped Clear. . . 0
Enable Max Time. . . . . . . . Max Time . . . . . 0
Exit Max . . . . . 0
Min Hold Time. . . 0
Hold Delay Time. . 0

| Green | Yellow | Red |
| :---: | :---: | :---: |
| 0 | 0.0 | 0.0 |
| 0 | 0.0 | 0.0 |
|  | 0.0 | 0.0 |


Terminate Overlap
Track Clearance Phase
Hold Phases
Exit Phases
Exit Calls on Phase
Out of Flash Color for Exit Phases . . . . Green
Linked Preemptor


```
Wellington County 24-36 2 & 22 Hillsburg 7/4/2013 6:13
NIC/TOD Clock/Calendar
Manual NIC Program Step . . . . . . . . 0
Manual TOD Program Step . . . . . . . . 0
NIC Resync Time . . . . . . . . . . . . 0000
Sync Reference is . . . . . . . . . . . Reference Time
Week 1 Begins on 1st Sunday . . . . . . NO If NO, then week containing Jan. 1
Disable Daylight Savings Time . . . . . NO
Daylight Savings
Begins Last Sunday in March . . . . . . NO If NO, then Second Sunday as per 2007 DST Law
```

```
TOD Weekly/Yearly
```

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |  |
| Sunday . . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Monday . . . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Tuesday. . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Wednesday . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Thursday . . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Friday. . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |
| Saturday . . | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Program No. |  |


|  |  |  |  |  |  |  |  | Wee | of | Yea |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 |
| Prog | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 |
| Prog | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 |  |
| Prog | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |  |


| Holiday | Type | Month | Day of Week/ <br> Day of Month | Week of Year/ Year | Program |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Fixed | 0 | 0 | 0 | 0 |
| 2 | Fixed | 0 | 0 | 0 | 0 |
| 3 | Fixed | 0 | 0 | 0 | 0 |
| 4 | Fixed | 0 | 0 | 0 | 0 |
| 5 | Fixed | 0 | 0 | 0 | 0 |
| 6 | Fixed | 0 | 0 | 0 | 0 |
| 7 | Fixed | 0 | 0 | 0 | 0 |
| 8 | Fixed | 0 | 0 | 0 | 0 |
| 9 | Fixed | 0 | 0 | 0 | 0 |
| 10 | Fixed | 0 | 0 | 0 | 0 |
| 11 | Fixed | 0 | 0 | 0 | 0 |
| 12 | Fixed | 0 | 0 | 0 | 0 |
| 13 | Fixed | 0 | 0 | 0 | 0 |
| 14 | Fixed | 0 | 0 | 0 | 0 |
| 15 | Fixed | 0 | 0 | 0 | 0 |
| 16 | Fixed | 0 | 0 | 0 | 0 |
| 17 | Fixed | 0 | 0 | 0 | 0 |
| 18 | Fixed | 0 | 0 | 0 | 0 |
| 19 | Fixed | 0 | 0 | 0 | 0 |
| 20 | Fixed | 0 | 0 | 0 | 0 |
| 21 | Fixed | 0 | 0 | 0 | 0 |
| 22 | Fixed | 0 | 0 | 0 | 0 |
| 23 | Fixed | 0 | 0 | 0 | 0 |
| 24 | Fixed | 0 | 0 | 0 | 0 |
| 25 | Fixed | 0 | 0 | 0 | 0 |
| 26 | Fixed | 0 | 0 | 0 | 0 |
| 27 | Fixed | 0 | 0 | 0 | 0 |
| 28 | Fixed | 0 | 0 | 0 | 0 |
| 29 | Fixed | 0 | 0 | 0 | 0 |
| 30 | Fixed | 0 | 0 | 0 | 0 |
| 31 | Fixed | 0 | 0 | 0 | 0 |
| 32 | Fixed | 0 | 0 | 0 | 0 |
| 33 | Fixed | 0 | 0 | 0 | 0 |
| 34 | Fixed | 0 | 0 | 0 | 0 |
| 35 | Fixed | 0 | 0 | 0 | 0 |
| 36 | Fixed | 0 | 0 | 0 | 0 |

Wellington County 24-36 2 \& 22 Hillsburg 7/4/2013 6:13

| Step $1 \quad$ Program 1 | Step B | Begins |  | 2300 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Flash. | Dimming | g Enab | ble. | - . . . |  |  |  |  |  |  |
| Red Rest | Alt Veh | Exte | ension | n . . . |  |  |  |  |  |  |
| Spare 5. | Det Log | g Enab | ble. | - |  |  |  |  |  |  |
| Spare 3. | Spare 4 | 4 | . . . | - . . . |  |  |  |  |  |  |
| Type 0 Dly Enable. | Spare 2 | 2 | - . . | - . . . |  |  |  |  |  |  |
| Det Diag Plan. . . . 0 |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Phase | Number |  |  |  |  |  |  |
|  | 12 | 3 | 45 | 56 | 7 | 8 | 9 | 10 | 11 | 12 |
| Max 2 Enable | X | . | . . | X | . | . | . | . | . |  |
| Max 3 Enable | . . | - | . . | . . | - | - | - | - | - |  |
| Veh Recall | . . | . | - . | . . | - | - | - | - | - |  |
| Veh Max Recall | - . | - | - . | - - | - | - | - | - | - |  |
| Ped Recall. | . . | . | - . | - . | - | - | . | - | - |  |
| Cond Service Inhibit. . . | - - | - | - . | - . | - | . | . | - | . |  |
| Phase Omit . . | - . | - | - . | - . | - | - | - | - | - |  |
| Special Function | - • | - | - - | - | - | - |  |  |  |  |

Alt Sequence . . . . . . . $\quad$. $\quad$.
Step 2 Program 1 Step Begins 0500


```
Det Diag Plan. . . . 0
```

| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Max 2 Enable
Max 3 Enable
Veh Recall
Veh Max Recall
Ped Recall
Cond Service Inhibit
Phase Omit
Special Function
A B C D E F

Alt Sequence
A B C D E F

## APPENDIX D

## LEVEL OF SERVICE DEFINITIONS

## LEVEL OF SERVICE DEFINITIONS

## Level of Service Criteria for Signalized Intersections

| Level of Service | Control Delay per Vehicle (seconds) | Interpretation |
| :---: | :---: | :---: |
| A | $\leq 10$ | Excellent. Progression is extremely favourable and most of the vehicles arrive during the green phase. Most vehicles do not stop at all |
| B | >10 \& $\leq 20$ | Very Good. Good progressing, short cycle lengths or both. More vehicles stop than with LOS " $A$ ", causing higher levels of average delay. |
| C | >20 \& $\leq 35$ | Good. Fair progressing, longer cycle lengths or both. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level, though many still pass through the intersection without stopping. |
| D | >35 \& $\leq 55$ | Fair. At level D , the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavourable progression, long cycle lengths, or high V/C ratio. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable. |
| E | $>55$ \& $\leq 80$ | Poor. This level is considered by many agencies to be the limit of acceptable delay. These high delay values generally indicate poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences. |
| F | >80 | Unsatisfactory. This level, considered to be unacceptable to most drivers, often occurs with oversaturation, that is, when arrival flow rates exceed the capacity of the intersection. It may also occurs at high $\mathrm{V} / \mathrm{C}$ ratios below 1.0 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors to such delays. These high delay values generally indicate poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences. |

Source: From Highway Capacity Manual Special Report 209-Table 9-1, Page 9-7

## LEVEL OF SERVICE DEFINITIONS

Level of Service Criteria for Two Way Stop Control (TWSC) Intersections

| Level of Service | Control Delay per Vehicle (seconds) | Interpretation |
| :---: | :---: | :---: |
| A | $\leq 10$ | Excellent. Large \& frequent gaps in traffic on the main roadway. Queuing on the minor street is rare |
| B | $>10$ \& $\leq 15$ | Very Good. Fewer gaps exist in the traffic on the main roadway. Queuing on the minor street is minimal. |
| C | $>15$ \& $\leq 25$ | Good. Fewer gaps exist in traffic on the main roadway. Delay on the minor approach becomes more noticeable. |
| D | >25 \& $\leq 35$ | Fair. Infrequent \& shorter gaps in traffic on the main roadway. Queuing lengths develop on the minor street. |
| E | $>35$ \& $\leq 50$ | Poor. Very infrequent gaps in traffic on the main roadway. Queuing lengths become noticeable. |
| F | >50 | Unsatisfactory. Very few gaps in traffic on the main roadway. Excessive delays with significant queue lengths on the minor street |

Source: From Highway Capacity Manual Special Report 209-Table 10-7, Page No.10-25

## APPENDIX E

SIGNALIZED AND UN-SIGNALIZED INTERSECTION CAPACITY ANALYSIS FOR EXISTING (2021), FUTURE ( 2026 \& 2031) TOTAL BACKGROUND AND FUTURE (2026 \& 2031) TOTAL TRAFFIC SENARIOS

|  | 4 | $\rightarrow$ | $\checkmark$ | $\%$ |  | 4 | 4 | 4 | \% | $V$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | $\uparrow$ |  | ${ }^{7}$ | 个 |  | ${ }^{7}$ | 个 |  |
| Traffic Volume (vph) | 26 | 67 | 32 | 65 | 47 | 36 | 11 | 119 | 55 | 80 | 214 | 26 |
| Future Volume (vph) | 26 | 67 | 32 | 65 | 47 | 36 | 11 | 119 | 55 | 80 | 214 | 26 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1746 | 0 | 0 | 1559 | 0 | 1805 | 1563 | 0 | 1492 | 1660 | 0 |
| Flt Permitted |  | 0.896 |  |  | 0.812 |  | 0.590 |  |  | 0.632 |  |  |
| Satd. Flow (perm) | 0 | 1580 | 0 | 0 | 1295 | 0 | 1121 | 1563 | 0 | 992 | 1660 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 25 |  |  | 23 |  |  | 39 |  |  | 10 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 144 | 0 | 0 | 170 | 0 | 13 | 200 | 0 | 92 | 276 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 16.3 |  |  | 16.3 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Actuated g/C Ratio |  | 0.24 |  |  | 0.24 |  | 0.54 | 0.54 |  | 0.54 | 0.54 |  |
| v/c Ratio |  | 0.37 |  |  | 0.52 |  | 0.02 | 0.23 |  | 0.17 | 0.30 |  |
| Control Delay |  | 20.0 |  |  | 25.2 |  | 8.5 | 7.9 |  | 9.7 | 9.9 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 20.0 |  |  | 25.2 |  | 8.5 | 7.9 |  | 9.7 | 9.9 |  |
| LOS |  | B |  |  | C |  | A | A |  | A | A |  |
| Approach Delay |  | 20.0 |  |  | 25.2 |  |  | 7.9 |  |  | 9.8 |  |
| Approach LOS |  | B |  |  | C |  |  | A |  |  | A |  |
| Queue Length 50th (m) |  | 12.6 |  |  | 16.2 |  | 0.7 | 9.6 |  | 5.4 | 16.9 |  |
| Queue Length 95th (m) |  | 25.5 |  |  | 31.9 |  | 3.3 | 22.2 |  | 14.1 | 34.5 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 637 |  |  | 523 |  | 609 | 867 |  | 539 | 907 |  |


| $\rangle$ | $\rightarrow$ |  | $t$ |  |  | 4 | $\uparrow$ | $p$ | $\downarrow$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.23 |  |  | 0.33 |  | 0.02 | 0.23 |  | 0.17 | 0.30 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 68.9 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 60 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.52 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 13.9 |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 83.7\% |  |  | ICU Level of Service E |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


|  | 4 | $\rightarrow$ | 7 | 7 |  | 4 | 4 | $\dagger$ | $p$ | $t$ | $\downarrow$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | \& |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 1 | 1 | 7 | 34 | 0 | 13 | 7 | 160 | 12 | 2 | 293 | 1 |
| Future Volume (Veh/h) | 1 | 1 | 7 | 34 | 0 | 13 | 7 | 160 | 12 | 2 | 293 | 1 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 1 | 1 | 8 | 37 | 0 | 14 | 8 | 174 | 13 | 2 | 318 | 1 |
| Pedestrians |  |  |  |  |  |  |  | 1 |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  | 1.2 |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  | 0 |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 533 | 526 | 320 | 528 | 520 | 180 | 319 |  |  | 187 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 533 | 526 | 320 | 528 | 520 | 180 | 319 |  |  | 187 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.4 | 7.2 | 6.5 | 6.3 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.5 | 3.6 | 4.0 | 3.4 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 99 | 92 | 100 | 98 | 99 |  |  | 100 |  |  |
| cM capacity (veh/h) | 450 | 456 | 687 | 444 | 460 | 844 | 1252 |  |  | 1399 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 10 | 51 | 195 | 321 |  |  |  |  |  |  |  |  |
| Volume Left | 1 | 37 | 8 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 8 | 14 | 13 | 1 |  |  |  |  |  |  |  |  |
| cSH | 623 | 510 | 1252 | 1399 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.02 | 0.10 | 0.01 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.4 | 2.6 | 0.2 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 10.9 | 12.8 | 0.4 | 0.1 |  |  |  |  |  |  |  |  |
| Lane LOS | B | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 10.9 | 12.8 | 0.4 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.5 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 32.2\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | $\stackrel{*}{ }$ | $\rightarrow$ | 7 | 7 |  | 4 | 4 | 4 | $p$ |  | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | $\leqslant$ |  |  | 4 |  |  | * |  |
| Traffic Volume (veh/h) | 1 | 0 | 19 | 12 | 1 | 5 | 2 | 152 | 5 | 2 | 235 | 1 |
| $\begin{array}{lllllllllllll}\text { Future Volume (Veh/h) } & 1 & 0 & 19 & 12 & 1 & 5 & 2 & 152 & 5 & 2 & \end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Hourly flow rate (vph) | 1 | 0 | 23 | 15 | 1 | 6 | 2 | 188 | 6 | 2 | 290 | 1 |
| Pedestrians 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 496 | 492 | 294 | 516 | 490 | 191 | 291 |  |  | 194 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 496 | 492 | 294 | 516 | 490 | 191 | 291 |  |  | 194 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 7.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.9 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 97 | 97 | 100 | 99 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 482 | 479 | 747 | 455 | 362 | 856 | 1282 |  |  | 1391 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 24 | 22 | 196 | 293 |  |  |  |  |  |  |  |  |
| Volume Left | 1 | 15 | 2 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 23 | 6 | 6 | 1 |  |  |  |  |  |  |  |  |
| cSH | 730 | 515 | 1282 | 1391 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.03 | 0.04 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.8 | 1.1 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 10.1 | 12.3 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Lane LOS | B | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 10.1 | 12.3 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 26.4\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 | $\rightarrow$ | $\geqslant$ | $F$ |  | 4 | 4 | $\dagger$ | \% | $V$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | $\$$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | F |  |
| Traffic Volume (vph) | 48 | 60 | 8 | 56 | 70 | 103 | 30 | 364 | 82 | 56 | 167 | 29 |
| Future Volume (vph) | 48 | 60 | 8 | 56 | 70 | 103 | 30 | 364 | 82 | 56 | 167 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1809 | 0 | 0 | 1720 | 0 | 1504 | 1803 | 0 | 1703 | 1812 | 0 |
| Flt Permitted |  | 0.747 |  |  | 0.876 |  | 0.623 |  |  | 0.426 |  |  |
| Satd. Flow (perm) | 0 | 1379 | 0 | 0 | 1525 | 0 | 986 | 1803 | 0 | 764 | 1812 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 5 |  |  | 59 |  |  | 19 |  |  | 15 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 128 | 0 | 0 | 252 | 0 | 33 | 490 | 0 | 62 | 216 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 16.7 |  |  | 16.7 |  | 35.9 | 35.9 |  | 35.9 | 35.9 |  |
| Actuated g/C Ratio |  | 0.25 |  |  | 0.25 |  | 0.53 | 0.53 |  | 0.53 | 0.53 |  |
| v/c Ratio |  | 0.37 |  |  | 0.60 |  | 0.06 | 0.51 |  | 0.15 | 0.22 |  |
| Control Delay |  | 23.2 |  |  | 23.2 |  | 9.1 | 12.6 |  | 10.2 | 9.1 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 23.2 |  |  | 23.2 |  | 9.1 | 12.6 |  | 10.2 | 9.1 |  |
| LOS |  | C |  |  | C |  | A | B |  | B | A |  |
| Approach Delay |  | 23.2 |  |  | 23.2 |  |  | 12.4 |  |  | 9.4 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | A |  |
| Queue Length 50th (m) |  | 13.2 |  |  | 21.6 |  | 1.8 | 34.2 |  | 3.6 | 12.0 |  |
| Queue Length 95th (m) |  | 26.9 |  |  | 42.9 |  | 6.8 | 70.6 |  | 11.6 | 27.8 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 554 |  |  | 644 |  | 523 | 966 |  | 405 | 969 |  |


| $\rangle$ | $\rightarrow$ |  | $t$ |  |  | , | $\uparrow$ | $p$ | $\downarrow$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.23 |  |  | 0.39 |  | 0.06 | 0.51 |  | 0.15 | 0.22 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 67.6 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 60 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.60 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 15.2 |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 73.7\% |  |  | ICU Level of Service D |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | 4 | $\rightarrow$ | \% | 4 |  | 4 | 4 | 4 | \% |  | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | $\leqslant$ |  |  | 4 |  |  | * |  |
| Traffic Volume (veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 378 | 20 | 6 | 240 | 1 |
| $\begin{array}{llllllllllll}\text { Future Volume (Veh/h) } & 0 & 0 & 2 & 14 & 0 & 5 & 17 & 378 & 20 & 6 & \end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 2 | 14 | 0 | 5 | 18 | 390 | 21 | 6 | 247 | 1 |
| Pedestrians 1105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  | 3.6 |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  | 1.2 |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 701 | 708 | 252 | 704 | 698 | 402 | 248 |  |  | 412 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 701 | 708 | 252 | 704 | 698 | 402 | 248 |  |  | 412 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.2 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.6 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 96 | 100 | 99 | 99 |  |  | 99 |  |  |
| cM capacity (veh/h) | 348 | 355 | 788 | 336 | 360 | 653 | 1330 |  |  | 1157 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 2 | 19 | 429 | 254 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 14 | 18 | 6 |  |  |  |  |  |  |  |  |
| Volume Right | 2 | 5 | 21 | 1 |  |  |  |  |  |  |  |  |
| cSH | 788 | 385 | 1330 | 1157 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.05 | 0.01 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.1 | 1.2 | 0.3 | 0.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 9.6 | 14.8 | 0.5 | 0.2 |  |  |  |  |  |  |  |  |
| Lane LOS | A | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 9.6 | 14.8 | 0.5 | 0.2 |  |  |  |  |  |  |  |  |
| Approach LOS | A | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 43.7\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 | $\rightarrow$ | 7 | 7 |  | 4 |  | $\dagger$ | 7 | $1$ | $\frac{1}{1}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | * |  | ${ }^{1}$ | t |  | ${ }^{1}$ | F |  |
| Traffic Volume (vph) | 30 | 170 | 120 | 169 | 86 | 40 | 39 | 172 | 94 | 93 | 351 | 30 |
| Future Volume (vph) | 30 | 170 | 120 | 169 | 86 | 40 | 39 | 172 | 94 | 93 | 351 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1735 | 0 | 0 | 1569 | 0 | 1805 | 1558 | 0 | 1492 | 1669 | 0 |
| Flt Permitted |  | 0.940 |  |  | 0.577 |  | 0.422 |  |  | 0.556 |  |  |
| Satd. Flow (perm) | 0 | 1639 | 0 | 0 | 931 | 0 | 802 | 1558 | 0 | 873 | 1669 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 43 |  |  | 11 |  |  | 47 |  |  | 7 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 367 | 0 | 0 | 339 | 0 | 45 | 306 | 0 | 107 | 437 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.61 |  |  | 1.02 |  | 0.12 | 0.42 |  | 0.27 | 0.57 |  |
| Control Delay |  | 23.3 |  |  | 81.8 |  | 13.3 | 13.9 |  | 15.4 | 18.9 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 23.3 |  |  | 81.8 |  | 13.3 | 13.9 |  | 15.4 | 18.9 |  |
| LOS |  | C |  |  | F |  | B | B |  | B | B |  |
| Approach Delay |  | 23.3 |  |  | 81.8 |  |  | 13.8 |  |  | 18.2 |  |
| Approach LOS |  | C |  |  | F |  |  | B |  |  | B |  |
| Queue Length 50th (m) |  | 40.1 |  |  | $\sim 51.7$ |  | 3.8 | 25.0 |  | 9.8 | 46.5 |  |
| Queue Length 95th (m) |  | 64.7 |  |  | \#100.1 |  | 9.6 | 42.7 |  | 20.0 | 71.0 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 602 |  |  | 333 |  | 364 | 733 |  | 396 | 762 |  |


| 4 |  |  |  |  |  | - | $\uparrow$ | 7 | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.61 |  |  | 1.02 |  | 0.12 | 0.42 |  | 0.27 | 0.57 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 75 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.02 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 31.9 |  |  |  | Intersection LOS: C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 109.2\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


|  | 4 | $\rightarrow$ | 7 | 7 | $4$ | 4 | 4 | 4 | $p$ |  | $\frac{1}{1}$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\$$ |  |  | * |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 1 | 1 | 8 | 34 | 0 | 13 | 12 | 212 | 14 | 2 | 335 | 1 |
| Future Volume (Veh/h) | 1 | 1 | 8 | 34 | 0 | 13 | 12 | 212 | 14 | 2 | 335 | 1 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 1 | 1 | 9 | 37 | 0 | 14 | 13 | 230 | 15 | 2 | 364 | 1 |
| Pedestrians 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) $\quad 1.2$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 646 | 640 | 366 | 642 | 632 | 238 | 365 |  |  | 245 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 646 | 640 | 366 | 642 | 632 | 238 | 365 |  |  | 245 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.4 | 7.2 | 6.5 | 6.3 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.5 | 3.6 | 4.0 | 3.4 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 99 | 90 | 100 | 98 | 99 |  |  | 100 |  |  |
| cM capacity (veh/h) | 377 | 391 | 647 | 370 | 395 | 784 | 1205 |  |  | 1333 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 11 | 51 | 258 | 367 |  |  |  |  |  |  |  |  |
| Volume Left | 1 | 37 | 13 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 9 | 14 | 15 | 1 |  |  |  |  |  |  |  |  |
| cSH | 575 | 433 | 1205 | 1333 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.02 | 0.12 | 0.01 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.5 | 3.2 | 0.3 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 11.4 | 14.4 | 0.5 | 0.1 |  |  |  |  |  |  |  |  |
|  | B | B | A | A |  |  |  |  |  |  |  |  |
| Lane LOS | 11.4 | 14.4 | 0.5 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.5 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 36.5\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 |  | 7 | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \& |  |  | * |  |  | \& |  |  | * |  |  |
| Traffic Volume (veh/h) | 1 | 0 | 19 | 12 | 1 | 5 | 2 | 168 | 5 | 2 | 259 | 1 |
| Future Volume (Veh/h) | 1 | 0 | 19 | 12 | 1 | 5 | 2 | 168 | 5 | 2 | 259 | 1 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Hourly flow rate (vph) | 1 | 0 | 23 | 15 | 1 | 6 | 2 | 207 | 6 | 2 | 320 | 1 |
| Pedestrians 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 545 | 542 | 324 | 566 | 539 | 210 | 321 |  |  | 213 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 545 | 542 | 324 | 566 | 539 | 210 | 321 |  |  | 213 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 7.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.9 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 97 | 96 | 100 | 99 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 447 | 449 | 719 | 422 | 336 | 835 | 1250 |  |  | 1369 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 24 | 22 | 215 | 323 |  |  |  |  |  |  |  |  |
| Volume Left | 1 | 15 | 2 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 23 | 6 | 6 | 1 |  |  |  |  |  |  |  |  |
| cSH | 701 | 481 | 1250 | 1369 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.03 | 0.05 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.8 | 1.1 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 10.3 | 12.8 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Lane LOS | B | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 10.3 | 12.8 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 27.7\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



## 

|  | 4 | $\rightarrow$ | 7 | 7 |  |  |  | 4 | $p$ | $V$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\ddagger$ |  |  | * |  | ${ }^{1}$ | $\uparrow$ |  | ${ }^{*}$ | F |  |
| Traffic Volume (vph) | 54 | 130 | 68 | 122 | 192 | 114 | 123 | 536 | 198 | 65 | 261 | 34 |
| Future Volume (vph) | 54 | 130 | 68 | 122 | 192 | 114 | 123 | 536 | 198 | 65 | 261 | 34 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1726 | 0 | 0 | 1761 | 0 | 1504 | 1783 | 0 | 1703 | 1819 | 0 |
| Flt Permitted |  | 0.792 |  |  | 0.802 |  | 0.540 |  |  | 0.114 |  |  |
| Satd. Flow (perm) | 0 | 1382 | 0 | 0 | 1432 | 0 | 855 | 1783 | 0 | 204 | 1819 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 27 |  |  | 26 |  |  | 32 |  |  | 11 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 277 | 0 | 0 | 470 | 0 | 135 | 807 | 0 | 71 | 324 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 26.0 |  |  | 26.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.34 |  |  | 0.34 |  | 0.46 | 0.46 |  | 0.46 | 0.46 |  |
| v/c Ratio |  | 0.57 |  |  | 0.93 |  | 0.34 | 0.96 |  | 0.76 | 0.38 |  |
| Control Delay |  | 23.5 |  |  | 50.6 |  | 16.7 | 45.0 |  | 69.8 | 14.9 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 23.5 |  |  | 50.6 |  | 16.7 | 45.0 |  | 69.8 | 14.9 |  |
| LOS |  | C |  |  | D |  | B | D |  | E | B |  |
| Approach Delay |  | 23.5 |  |  | 50.6 |  |  | 41.0 |  |  | 24.8 |  |
| Approach LOS |  | C |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th (m) |  | 30.2 |  |  | 64.1 |  | 12.8 | 112.6 |  | 8.7 | 30.3 |  |
| Queue Length 95th (m) |  | 54.3 |  |  | \#122.7 |  | 26.5 | \#193.5 |  | \#33.3 | 49.8 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 508 |  |  | 526 |  | 393 | 838 |  | 93 | 843 |  |


| $\stackrel{ }{*}$ |  |  | 7 |  |  | 4 | $\uparrow$ | $p$ | * | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.55 |  |  | 0.89 |  | 0.34 | 0.96 |  | 0.76 | 0.38 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other | Other |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 76 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.96 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 37.7 |  |  |  | Intersection LOS: D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 119.6\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer.Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | 4 |  | 7 | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \& |  |  | * |  |  | * |  |  | \$ |  |  |
| Traffic Volume (veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 417 | 20 | 6 | 265 | 1 |
| Future Volume (Veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 417 | 20 | 6 | 265 | 1 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 2 | 14 | 0 | 5 | 18 | 430 | 21 | 6 | 273 | 1 |
| Pedestrians 1105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  | 3.6 |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  | 1.2 |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 767 | 774 | 278 | 770 | 764 | 442 | 274 |  |  | 452 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 767 | 774 | 278 | 770 | 764 | 442 | 274 |  |  | 452 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.2 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.6 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 95 | 100 | 99 | 99 |  |  | 99 |  |  |
| cM capacity (veh/h) | 314 | 325 | 762 | 303 | 330 | 620 | 1301 |  |  | 1118 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 2 | 19 | 469 | 280 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 14 | 18 | 6 |  |  |  |  |  |  |  |  |
| Volume Right | 2 | 5 | 21 | 1 |  |  |  |  |  |  |  |  |
| cSH | 762 | 350 | 1301 | 1118 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.05 | 0.01 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.1 | 1.4 | 0.3 | 0.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 9.7 | 15.9 | 0.4 | 0.2 |  |  |  |  |  |  |  |  |
| Lane LOS | A | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 9.7 | 15.9 | 0.4 | 0.2 |  |  |  |  |  |  |  |  |
| Approach LOS | A | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 46.1\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 | $\rightarrow$ | 7 | 7 |  | 4 |  | $\dagger$ | 7 | $1$ | $\frac{1}{1}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  | ${ }^{7}$ | t |  | ${ }^{*}$ | F |  |
| Traffic Volume (vph) | 33 | 178 | 124 | 176 | 91 | 44 | 40 | 186 | 100 | 103 | 376 | 33 |
| Future Volume (vph) | 33 | 178 | 124 | 176 | 91 | 44 | 40 | 186 | 100 | 103 | 376 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1736 | 0 | 0 | 1569 | 0 | 1805 | 1559 | 0 | 1492 | 1669 | 0 |
| Flt Permitted |  | 0.935 |  |  | 0.564 |  | 0.390 |  |  | 0.532 |  |  |
| Satd. Flow (perm) | 0 | 1632 | 0 | 0 | 909 | 0 | 741 | 1559 | 0 | 835 | 1669 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 42 |  |  | 12 |  |  | 46 |  |  | 8 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 386 | 0 | 0 | 358 | 0 | 46 | 329 | 0 | 118 | 470 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.64 |  |  | 1.10 |  | 0.14 | 0.45 |  | 0.31 | 0.62 |  |
| Control Delay |  | 24.5 |  |  | 106.0 |  | 13.6 | 14.6 |  | 16.2 | 20.0 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 24.5 |  |  | 106.0 |  | 13.6 | 14.6 |  | 16.2 | 20.0 |  |
| LOS |  | C |  |  | F |  | B | B |  | B | B |  |
| Approach Delay |  | 24.5 |  |  | 106.0 |  |  | 14.5 |  |  | 19.2 |  |
| Approach LOS |  | C |  |  | F |  |  | B |  |  | B |  |
| Queue Length 50th (m) |  | 43.4 |  |  | $\sim 62.2$ |  | 3.9 | 27.9 |  | 11.0 | 51.3 |  |
| Queue Length 95th (m) |  | 69.5 |  |  | \#108.5 |  | 9.9 | 46.8 |  | 22.3 | 78.0 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 599 |  |  | 326 |  | 336 | 733 |  | 379 | 763 |  |


| 4 |  |  |  |  |  |  | 4 | 7 | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.64 |  |  | 1.10 |  | 0.14 | 0.45 |  | 0.31 | 0.62 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.10 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 37.6 |  |  |  | Intersection LOS: D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 111.0\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | $\stackrel{*}{ }$ | $\rightarrow$ | \% | 4 |  | 4 | 4 | 4 | \% |  | $\frac{1}{1}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\ddagger$ |  |  | $\ddagger$ |  |  | * |  |  | \$ |  |
| Traffic Volume (veh/h) | 1 | 0 | 19 | 12 | 1 | 5 | 2 | 185 | 5 | 2 | 286 | 1 |
| Future Volume (Veh/h) | 1 | 0 | 19 | 12 | 1 | 5 | 2 | 185 | 5 | 2 | 286 | 1 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Hourly flow rate (vph) | 1 | 0 | 23 | 15 | 1 | 6 | 2 | 228 | 6 | 2 | 353 | 1 |
| Pedestrians 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{array}{lr}\text { Walking Speed (m/s) } & 1.2 \\ \text { Percent Blockage } & 0\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type None None |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 599 | 596 | 358 | 620 | 593 | 231 | 354 |  |  | 234 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 599 | 596 | 358 | 620 | 593 | 231 | 354 |  |  | 234 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 7.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.9 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 97 | 96 | 100 | 99 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 411 | 419 | 689 | 388 | 310 | 813 | 1216 |  |  | 1345 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 24 | 22 | 236 | 356 |  |  |  |  |  |  |  |  |
| Volume Left | 1 | 15 | 2 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 23 | 6 | 6 | 1 |  |  |  |  |  |  |  |  |
|  | 670 | 446 | 1216 | 1345 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.04 | 0.05 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.9 | 1.2 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 10.6 | 13.5 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Lane LOS | B | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 10.6 | 13.5 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.9 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 29.1\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



## 

|  | 4 | $\rightarrow$ | $\square$ | 7 |  |  |  | 4 | \% | $V$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\ddagger$ |  |  | * |  | * | F |  | ${ }^{*}$ | F |  |
| Traffic Volume (vph) | 60 | 137 | 69 | 128 | 200 | 126 | 127 | 578 | 207 | 71 | 281 | 37 |
| Future Volume (vph) | 60 | 137 | 69 | 128 | 200 | 126 | 127 | 578 | 207 | 71 | 281 | 37 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1733 | 0 | 0 | 1759 | 0 | 1504 | 1785 | 0 | 1703 | 1818 | 0 |
| Flt Permitted |  | 0.765 |  |  | 0.794 |  | 0.510 |  |  | 0.114 |  |  |
| Satd. Flow (perm) | 0 | 1340 | 0 | 0 | 1416 | 0 | 808 | 1785 | 0 | 204 | 1818 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 25 |  |  | 28 |  |  | 31 |  |  | 11 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 293 | 0 | 0 | 499 | 0 | 140 | 862 | 0 | 78 | 350 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.60 |  |  | 0.97 |  | 0.38 | 1.04 |  | 0.85 | 0.42 |  |
| Control Delay |  | 25.0 |  |  | 59.1 |  | 17.8 | 65.2 |  | 86.5 | 15.6 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 25.0 |  |  | 59.1 |  | 17.8 | 65.2 |  | 86.5 | 15.6 |  |
| LOS |  | C |  |  | E |  | B | E |  | F | B |  |
| Approach Delay |  | 25.0 |  |  | 59.1 |  |  | 58.6 |  |  | 28.5 |  |
| Approach LOS |  | C |  |  | E |  |  | E |  |  | C |  |
| Queue Length 50th (m) |  | 33.1 |  |  | 70.6 |  | 13.6 | ~142.8 |  | 10.1 | 33.4 |  |
| Queue Length 95th (m) |  | 59.3 |  |  | \#133.9 |  | 28.3 | \#213.2 |  | \#36.6 | 54.4 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 486 |  |  | 514 |  | 367 | 828 |  | 92 | 832 |  |


| 4 |  |  |  |  |  |  | 4 | $p$ | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.60 |  |  | 0.97 |  | 0.38 | 1.04 |  | 0.85 | 0.42 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.04 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 48.5 |  |  |  | Intersection LOS: D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 123.6\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | 4 |  | 7 | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \& |  |  | * |  |  | * |  |  | $\uparrow$ |  |  |
| Traffic Volume (veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 461 | 20 | 6 | 293 | 1 |
| Future Volume (Veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 461 | 20 | 6 | 293 | 1 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 2 | 14 | 0 | 5 | 18 | 475 | 21 | 6 | 302 | 1 |
| Pedestrians 1105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  | 3.6 |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  | 1.2 |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 841 | 848 | 308 | 844 | 838 | 486 | 303 |  |  | 497 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 841 | 848 | 308 | 844 | 838 | 486 | 303 |  |  | 497 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.2 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.6 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 95 | 100 | 99 | 99 |  |  | 99 |  |  |
| cM capacity (veh/h) | 280 | 295 | 734 | 270 | 299 | 585 | 1269 |  |  | 1076 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 2 | 19 | 514 | 309 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 14 | 18 | 6 |  |  |  |  |  |  |  |  |
| Volume Right | 2 | 5 | 21 | 1 |  |  |  |  |  |  |  |  |
| cSH | 734 | 315 | 1269 | 1076 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.06 | 0.01 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.1 | 1.5 | 0.3 | 0.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 9.9 | 17.2 | 0.4 | 0.2 |  |  |  |  |  |  |  |  |
| Lane LOS | A | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 9.9 | 17.2 | 0.4 | 0.2 |  |  |  |  |  |  |  |  |
| Approach LOS | A | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 48.7\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 |  |  | 4 |  |  |  | $\dagger$ | 7 |  |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\uparrow$ |  | ${ }^{*}$ | t |  | ${ }^{*}$ | F |  |
| Traffic Volume (vph) | 30 | 170 | 120 | 169 | 86 | 66 | 39 | 195 | 94 | 237 | 482 | 30 |
| Future Volume (vph) | 30 | 170 | 120 | 169 | 86 | 66 | 39 | 195 | 94 | 237 | 482 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1735 | 0 | 0 | 1551 | 0 | 1805 | 1561 | 0 | 1492 | 1676 | 0 |
| Flt Permitted |  | 0.939 |  |  | 0.596 |  | 0.280 |  |  | 0.528 |  |  |
| Satd. Flow (perm) | 0 | 1637 | 0 | 0 | 949 | 0 | 532 | 1561 | 0 | 829 | 1676 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 43 |  |  | 19 |  |  | 41 |  |  | 5 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 367 | 0 | 0 | 369 | 0 | 45 | 332 | 0 | 272 | 588 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.61 |  |  | 1.07 |  | 0.19 | 0.45 |  | 0.72 | 0.77 |  |
| Control Delay |  | 23.3 |  |  | 95.1 |  | 15.1 | 15.0 |  | 30.8 | 26.0 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 23.3 |  |  | 95.1 |  | 15.1 | 15.0 |  | 30.8 | 26.0 |  |
| LOS |  | C |  |  | F |  | B | B |  | C | C |  |
| Approach Delay |  | 23.3 |  |  | 95.1 |  |  | 15.0 |  |  | 27.5 |  |
| Approach LOS |  | C |  |  | F |  |  | B |  |  | C |  |
| Queue Length 50th (m) |  | 40.1 |  |  | ~61.9 |  | 3.9 | 28.9 |  | 32.4 | 71.9 |  |
| Queue Length 95th (m) |  | 64.7 |  |  | \#108.6 |  | 10.4 | 47.9 |  | \#68.1 | 107.6 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 601 |  |  | 345 |  | 241 | 731 |  | 376 | 764 |  |


| 4 |  |  |  |  |  |  | 4 | 7 | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.61 |  |  | 1.07 |  | 0.19 | 0.45 |  | 0.72 | 0.77 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 75 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.07 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 37.0 |  |  |  | Intersection LOS: D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 113.0\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


|  | 4 | $\rightarrow$ | 7 | 7 | $4$ | 4 | 4 | 4 | $p$ |  | $\frac{1}{1}$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\$$ |  |  | * |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 5 | 1 | 8 | 34 | 0 | 36 | 12 | 262 | 14 | 2 | 617 | 1 |
| Future Volume (Veh/h) | 5 | 1 | 8 | 34 | 0 | 36 | 12 | 262 | 14 | 2 | 617 | 1 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 5 | 1 | 9 | 37 | 0 | 39 | 13 | 285 | 15 | 2 | 671 | 1 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) 1.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1033 | 1002 | 672 | 1004 | 994 | 292 | 672 |  |  | 300 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1033 | 1002 | 672 | 1004 | 994 | 292 | 672 |  |  | 300 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.4 | 7.2 | 6.5 | 6.3 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.5 | 3.6 | 4.0 | 3.4 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 97 | 100 | 98 | 82 | 100 | 95 | 99 |  |  | 100 |  |  |
| cM capacity (veh/h) | 199 | 241 | 430 | 208 | 243 | 730 | 928 |  |  | 1273 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
|  | 15 | 76 | 313 | 674 |  |  |  |  |  |  |  |  |
| Volume Total | 5 | 37 | 13 | 2 |  |  |  |  |  |  |  |  |
| Volume Left Volume Right | 9 | 39 | 15 | 1 |  |  |  |  |  |  |  |  |
| cSH | 299 | 328 | 928 | 1273 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.05 | 0.23 | 0.01 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 1.3 | 7.0 | 0.3 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 17.7 | 19.2 | 0.5 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | C | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 17.7 | 19.2 | 0.5 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | C | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 46.0\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\checkmark$ | $\checkmark$ |  | 4 | 4 | 9 | 7 |  | $\downarrow$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \& |  |  | \& |  |  | \& |  |  | \& |  |  |
| Traffic Volume (veh/h) | 10 | 0 | 19 | 12 | 1 | 27 | 2 | 245 | 5 | 2 | 543 | 1 |
| Future Volume (Veh/h) | 10 | 0 | 19 | 12 | 1 | 27 | 2 | 245 | 5 | 2 | 543 | 1 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Hourly flow rate (vph) | 12 | 0 | 23 | 15 | 1 | 33 | 2 | 302 | 6 | 2 | 670 | 1 |
| Pedestrians 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  | 3.6 |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  | 1.2 |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1017 | 986 | 674 | 1010 | 984 | 305 | 671 |  |  | 308 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1017 | 986 | 674 | 1010 | 984 | 305 | 671 |  |  | 308 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 7.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.9 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 94 | 100 | 95 | 93 | 99 | 96 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 207 | 249 | 456 | 208 | 171 | 740 | 929 |  |  | 1264 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 35 | 49 | 310 | 673 |  |  |  |  |  |  |  |  |
| Volume Left | 12 | 15 | 2 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 23 | 33 | 6 | 1 |  |  |  |  |  |  |  |  |
| cSH | 323 | 399 | 929 | 1264 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.11 | 0.12 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 2.9 | 3.3 | 0.1 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 17.5 | 15.3 | 0.1 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | C | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 17.5 | 15.3 | 0.1 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | C | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.3 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 41.2\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | \% | $\checkmark$ | 4 | 4 | 4 | 4 | 7 | , | $\dagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 14 | 0 | 195 | 1 | 27 | 4 | 79 | 177 | 1 | 4 | 224 | 2 |
| Future Volume (Veh/h) | 14 | 0 | 195 | 1 | 27 | 4 | 79 | 177 | 1 | 4 | 224 | 2 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 14 | 0 | 201 | 1 | 28 | 4 | 81 | 182 | 1 | 4 | 231 | 2 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 602 | 585 | 232 | 786 | 586 | 182 | 233 |  |  | 183 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 602 | 585 | 232 | 786 | 586 | 182 | 233 |  |  | 183 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 8.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 4.4 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 96 | 100 | 75 | 99 | 93 | 100 | 94 |  |  | 100 |  |  |
| cM capacity (veh/h) | 369 | 396 | 807 | 155 | 396 | 865 | 1335 |  |  | 1404 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 215 | 33 | 264 | 237 |  |  |  |  |  |  |  |  |
| Volume Left | 14 | 1 | 81 | 4 |  |  |  |  |  |  |  |  |
| Volume Right | 201 | 4 | 1 | 2 |  |  |  |  |  |  |  |  |
| cSH | 749 | 403 | 1335 | 1404 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.29 | 0.08 | 0.06 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 9.5 | 2.1 | 1.5 | 0.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 11.7 | 14.7 | 2.8 | 0.2 |  |  |  |  |  |  |  |  |
| Lane LOS | B | B | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 11.7 | 14.7 | 2.8 | 0.2 |  |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 5.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 55.0\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |




|  | 4 | $\rightarrow$ | $\geqslant$ | 7 |  | 4 | 4 | $\dagger$ | \% | $1$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | 4 |  | ${ }^{7}$ | 个 |  | ${ }^{7}$ | F |  |
| Traffic Volume (vph) | 54 | 130 | 68 | 122 | 192 | 217 | 123 | 629 | 198 | 116 | 306 | 34 |
| Future Volume (vph) | 54 | 130 | 68 | 122 | 192 | 217 | 123 | 629 | 198 | 116 | 306 | 34 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1726 | 0 | 0 | 1733 | 0 | 1504 | 1791 | 0 | 1703 | 1822 | 0 |
| Flt Permitted |  | 0.741 |  |  | 0.834 |  | 0.486 |  |  | 0.114 |  |  |
| Satd. Flow (perm) | 0 | 1293 | 0 | 0 | 1461 | 0 | 770 | 1791 | 0 | 204 | 1822 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 27 |  |  | 50 |  |  | 27 |  |  | 9 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 277 | 0 | 0 | 583 | 0 | 135 | 909 | 0 | 127 | 373 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.59 |  |  | 1.07 |  | 0.39 | 1.10 |  | 1.38 | 0.45 |  |
| Control Delay |  | 24.5 |  |  | 84.6 |  | 18.0 | 84.3 |  | 250.8 | 16.2 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 24.5 |  |  | 84.6 |  | 18.0 | 84.3 |  | 250.8 | 16.2 |  |
| LOS |  | C |  |  | F |  | B | F |  | F | B |  |
| Approach Delay |  | 24.5 |  |  | 84.6 |  |  | 75.8 |  |  | 75.8 |  |
| Approach LOS |  | C |  |  | F |  |  | E |  |  | E |  |
| Queue Length 50th (m) |  | 30.7 |  |  | ~95.6 |  | 13.1 | ~158.8 |  | ~26.2 | 36.5 |  |
| Queue Length 95th (m) |  | 56.0 |  |  | \#158.0 |  | 27.8 | \#230.1 |  | \#46.4 | 58.8 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 470 |  |  | 544 |  | 350 | 828 |  | 92 | 833 |  |


| 4 |  |  |  |  |  |  | 4 | 7 | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.59 |  |  | 1.07 |  | 0.39 | 1.10 |  | 1.38 | 0.45 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 150 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.38 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 72.0 |  |  |  | Intersection LOS: E |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 130.2\% |  |  |  | ICU Level of Service H |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | 4 |  |  | 7 |  | 4 | 4 | 4 | 7 |  | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | $\leqslant$ |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 616 | 20 | 11 | 369 | 3 |
| Future Volume (Veh/h) | 0 | 0 | 2 | 14 | 0 | 5 | 17 | 616 | 20 | 11 | 369 | 3 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 2 | 14 | 0 | 5 | 18 | 635 | 21 | 11 | 380 | 3 |
| Pedestrians 1105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{array}{lrr}\text { Walking Speed (m/s) } & 1.2 & 1.2 \\ \text { Percent Blockage } & 0 & 0\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type None |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1090 | 1096 | 386 | 1093 | 1088 | 646 | 383 |  |  | 657 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1090 | 1096 | 386 | 1093 | 1088 | 646 | 383 |  |  | 657 |  |  |
|  | 7.1 | 6.5 | 6.2 | 7.2 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) s queue free \% | 3.5 | 4.0 | 3.3 | 3.6 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
|  | 100 | 100 | 100 | 92 | 100 | 99 | 98 |  |  | 99 |  |  |
| cM capacity (veh/h) | 188 | 209 | 663 | 181 | 212 | 474 | 1187 |  |  | 939 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 2 | 19 | 674 | 394 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 14 | 18 | 11 |  |  |  |  |  |  |  |  |
| Volume Right | 2 | 5 | 21 | 3 |  |  |  |  |  |  |  |  |
|  | 663 | 216 | 1187 | 939 |  |  |  |  |  |  |  |  |
| cSH <br> Volume to Capacity | 0.00 | 0.09 | 0.02 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.1 | 2.3 | 0.4 | 0.3 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 10.4 | 23.2 | 0.4 | 0.4 |  |  |  |  |  |  |  |  |
| Lane LOS | B | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 10.4 | 23.2 | 0.4 | 0.4 |  |  |  |  |  |  |  |  |
| Approach LOS | B | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 55.5\% |  | U Level | Service |  |  | B |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | $\Rightarrow$ | $\rightarrow$ | $\geqslant$ | 7 | $\leftarrow$ |  | 4 | $\uparrow$ | $p$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  |  | ¢ |  |
| Traffic Volume (veh/h) | 4 | 6 | 53 | 4 | 0 | 0 | 101 | 412 | 10 | 6 | 288 | 9 |
| Future Volume (Veh/h) | 4 | 6 | 53 | 4 | 0 | 0 | 101 | 412 | 10 | 6 | 288 | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 4 | 7 | 58 | 4 | 0 | 0 | 110 | 448 | 11 | 7 | 313 | 10 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1006 | 1011 | 318 | 1067 | 1010 | 454 | 323 |  |  | 459 |  |  |
| vC1, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 1006 | 1011 | 318 | 1067 | 1010 | 454 | 323 |  |  | 459 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 98 | 97 | 92 | 98 | 100 | 100 | 91 |  |  | 99 |  |  |
| cM capacity (veh/h) | 204 | 217 | 723 | 168 | 217 | 611 | 1237 |  |  | 1113 |  |  |
| Direction, Lane\# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 69 | 4 | 569 | 330 |  |  |  |  |  |  |  |  |
| Volume Left | 4 | 4 | 110 | 7 |  |  |  |  |  |  |  |  |
| Volume Right | 58 | 0 | 11 | 10 |  |  |  |  |  |  |  |  |
| cSH | 522 | 168 | 1237 | 1113 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.13 | 0.02 | 0.09 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 3.6 | 0.6 | 2.3 | 0.2 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 12.9 | 27.0 | 2.4 | 0.2 |  |  |  |  |  |  |  |  |
| Lane LOS | B | D | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 12.9 | 27.0 | 2.4 | 0.2 |  |  |  |  |  |  |  |  |
| Approach LOS | B | D |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 2.5 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 57.6\% |  | CU Level | f Service |  |  | B |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 |  | 4 | 4 | $\frac{1}{1}$ | $\downarrow$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |  |
| Lane Configurations | * |  |  | $\uparrow$ | $\uparrow$ |  |  |
| Traffic Volume (veh/h) | 8 | 58 | 98 | 523 | 333 | 12 |  |
| Future Volume (Veh/h) | 8 | 58 | 98 | 523 | 333 | 12 |  |
| Sign Control | Stop |  |  | Free | Free |  |  |
| Grade | 0\% |  |  | 0\% | 0\% |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |  |
| Hourly flow rate (vph) | 9 | 63 | 107 | 568 | 362 | 13 |  |
| Pedestrians |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |
| Median type None None |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |
| vC , conflicting volume | 1150 | 368 | 375 |  |  |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1150 | 368 | 375 |  |  |  |  |
| tC , single (s) | 6.4 | 6.2 | 4.1 |  |  |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 3.3 | 2.2 |  |  |  |  |
| p0 queue free \% | 95 | 91 | 91 |  |  |  |  |
| cM capacity (veh/h) | 199 | 677 | 1183 |  |  |  |  |
| Direction, Lane \# | EB 1 | NB 1 | SB 1 |  |  |  |  |
| Volume Total | 72 | 675 | 375 |  |  |  |  |
| Volume Left | 9 | 107 | 0 |  |  |  |  |
| Volume Right | 63 | 0 | 13 |  |  |  |  |
| cSH | 521 | 1183 | 1700 |  |  |  |  |
| Volume to Capacity | 0.14 | 0.09 | 0.22 |  |  |  |  |
| Queue Length 95th (m) | 3.8 | 2.4 | 0.0 |  |  |  |  |
| Control Delay (s) | 13.0 | 2.3 | 0.0 |  |  |  |  |
| Lane LOS | B | A |  |  |  |  |  |
| Approach Delay (s) | 13.0 | 2.3 | 0.0 |  |  |  |  |
| Approach LOS B |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |
| Average Delay |  |  | 2.2 |  |  |  |  |
| Intersection Capacity Utilization |  |  | 65.2\% |  | CU Level of | Service | C |
| Analysis Period (min) |  |  | 15 |  |  |  |  |



|  | 4 | $\rightarrow$ | $\cdots$ | 4 |  | 4 | 4 | $\dagger$ | \% | $1$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\ddagger$ |  |  | \& |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | 个 |  |
| Traffic Volume (vph) | 33 | 178 | 124 | 176 | 91 | 76 | 40 | 214 | 100 | 264 | 522 | 33 |
| Future Volume (vph) | 33 | 178 | 124 | 176 | 91 | 76 | 40 | 214 | 100 | 264 | 522 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1736 | 0 | 0 | 1549 | 0 | 1805 | 1562 | 0 | 1492 | 1676 | 0 |
| Flt Permitted |  | 0.931 |  |  | 0.586 |  | 0.235 |  |  | 0.498 |  |  |
| Satd. Flow (perm) | 0 | 1625 | 0 | 0 | 931 | 0 | 446 | 1562 | 0 | 782 | 1676 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 42 |  |  | 20 |  |  | 40 |  |  | 5 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 386 | 0 | 0 | 394 | 0 | 46 | 361 | 0 | 303 | 638 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.65 |  |  | 1.16 |  | 0.23 | 0.49 |  | 0.85 | 0.84 |  |
| Control Delay |  | 24.6 |  |  | 126.8 |  | 16.5 | 15.8 |  | 44.1 | 30.3 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 24.6 |  |  | 126.8 |  | 16.5 | 15.8 |  | 44.1 | 30.3 |  |
| LOS |  | C |  |  | F |  | B | B |  | D | C |  |
| Approach Delay |  | 24.6 |  |  | 126.8 |  |  | 15.9 |  |  | 34.8 |  |
| Approach LOS |  | C |  |  | F |  |  | B |  |  | C |  |
| Queue Length 50th (m) |  | 43.4 |  |  | ~71.1 |  | 4.1 | 32.7 |  | 39.6 | 81.8 |  |
| Queue Length 95th (m) |  | 69.6 |  |  | \#118.8 |  | 11.1 | 53.3 |  | \#83.2 | \#136.2 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 597 |  |  | 339 |  | 202 | 731 |  | 355 | 764 |  |


| $\Rightarrow$ | $\rightarrow$ |  |  |  |  | 4 | 4 | \% | - | $\ddagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.65 |  |  | 1.16 |  | 0.23 | 0.49 |  | 0.85 | 0.84 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 75 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.16 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 46.4 |  |  |  | Intersection LOS: D |  |  |  |  |  |  |  |
|  |  |  |  | Intersection Capacity Utilization 117.4\% ICU Level of Service H |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| ~ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


|  | 4 | $\rightarrow$ | 7 | 7 | $4$ | 4 | 4 | 4 | $p$ |  | $\frac{1}{1}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\$$ |  |  | * |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 4 | 1 | 8 | 34 | 0 | 34 | 12 | 292 | 14 | 2 | 683 | 1 |
| Future Volume (Veh/h) | 4 | 1 | 8 | 34 | 0 | 34 | 12 | 292 | 14 | 2 | 683 | 1 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 4 | 1 | 9 | 37 | 0 | 37 | 13 | 317 | 15 | 2 | 742 | 1 |
| Pedestrians 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) 1.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1134 | 1104 | 744 | 1108 | 1098 | 324 | 743 |  |  | 332 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1134 | 1104 | 744 | 1108 | 1098 | 324 | 743 |  |  | 332 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.4 | 7.2 | 6.5 | 6.3 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.5 | 3.6 | 4.0 | 3.4 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 98 | 100 | 98 | 79 | 100 | 95 | 99 |  |  | 100 |  |  |
| cM capacity (veh/h) | 170 | 209 | 391 | 176 | 211 | 701 | 873 |  |  | 1239 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 14 | 74 | 345 | 745 |  |  |  |  |  |  |  |  |
| Volume Left | 4 | 37 | 13 | 2 |  |  |  |  |  |  |  |  |
| Volume Right | 9 | 37 | 15 | 1 |  |  |  |  |  |  |  |  |
|  | 272 | 281 | 873 | 1239 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.05 | 0.26 | 0.01 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 1.3 | 8.2 | 0.4 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 18.9 | 22.3 | 0.5 | 0.0 |  |  |  |  |  |  |  |  |
|  | C | C | A | A |  |  |  |  |  |  |  |  |
| Lane LOS | 18.9 | 22.3 | 0.5 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | C | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 50.0\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\checkmark$ | 7 |  | 4 | 4 | $\dagger$ | \% |  | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | $\leqslant$ |  |  | \$ |  |  | \& |  |
| Traffic Volume (veh/h) | 15 | 0 | 19 | 12 | 1 | 29 | 2 | 271 | 5 | 5 | 602 | 4 |
| Future Volume (Veh/h) | 15 | 0 | 19 | 12 | 1 | 29 | 2 | 271 | 5 | 5 | 602 | 4 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Hourly flow rate (vph) | 19 | 0 | 23 | 15 | 1 | 36 | 2 | 335 | 6 | 6 | 743 | 5 |
| Pedestrians 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{array}{lr}\text { Walking Speed (m/s) } & 1.2 \\ \text { Percent Blockage } & 0\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type <br> None <br> None |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1136 | 1102 | 750 | 1126 | 1102 | 338 | 748 |  |  | 341 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked voltC , single (s) | 1136 | 1102 | 750 | 1126 | 1102 | 338 | 748 |  |  | 341 |  |  |
|  | 7.1 | 6.5 | 6.2 | 7.1 | 7.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.9 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 89 | 100 | 94 | 91 | 99 | 95 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 170 | 212 | 413 | 172 | 142 | 709 | 870 |  |  | 1229 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 42 | 52 | 343 | 754 |  |  |  |  |  |  |  |  |
| Volume Left | 19 | 15 | 2 | 6 |  |  |  |  |  |  |  |  |
| Volume Right | 23 | 36 | 6 | 5 |  |  |  |  |  |  |  |  |
|  | 251 | 358 | 870 | 1229 |  |  |  |  |  |  |  |  |
| cSH <br> Volume to Capacity | 0.17 | 0.15 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Volume to Capacity Queue Length 95th ( m ) | 4.7 | 4.0 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 22.2 | 16.7 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Lane LOS | C | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 22.2 | 16.7 | 0.1 | 0.1 |  |  |  |  |  |  |  |  |
| Approach LOS | C | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.6 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 46.6\% |  | U Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\checkmark$ | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\dagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | * |  |  | \& |  |  | \& |  |  | \& |  |  |
| Traffic Volume (veh/h) | 18 | 0 | 227 | 1 | 39 | 4 | 95 | 216 | 1 | 12 | 257 | 3 |
| Future Volume (Veh/h) | 18 | 0 | 227 | 1 | 39 | 4 | 95 | 216 | 1 | 12 | 257 | 3 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 19 | 0 | 234 | 1 | 40 | 4 | 98 | 223 | 1 | 12 | 265 | 3 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 734 | 710 | 266 | 944 | 712 | 224 | 268 |  |  | 224 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 734 | 710 | 266 | 944 | 712 | 224 | 268 |  |  | 224 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 8.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 4.4 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 93 | 100 | 70 | 99 | 88 | 100 | 92 |  |  | 99 |  |  |
| cM capacity (veh/h) | 284 | 328 | 772 | 108 | 328 | 821 | 1296 |  |  | 1357 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 253 | 45 | 322 | 280 |  |  |  |  |  |  |  |  |
| Volume Left | 19 | 1 | 98 | 12 |  |  |  |  |  |  |  |  |
| Volume Right | 234 | 4 | 1 | 3 |  |  |  |  |  |  |  |  |
| cSH | 684 | 330 | 1296 | 1357 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.37 | 0.14 | 0.08 | 0.01 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 13.7 | 3.7 | 2.0 | 0.2 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 13.3 | 17.6 | 2.9 | 0.4 |  |  |  |  |  |  |  |  |
| Lane LOS | B | C | A | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 13.3 | 17.6 | 2.9 | 0.4 |  |  |  |  |  |  |  |  |
| Approach LOS | B | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 5.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 62.8\% |  | Level | Service |  |  | B |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



| Intersection |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh | 5.5 |  |  |  |  |  |  |  |
| Intersection LOS | A |  |  |  |  |  |  |  |
| Approach |  | EB |  | WB |  | NB |  | SB |
| Entry Lanes |  | 1 |  | 1 |  | 1 |  | 1 |
| Conflicting Circle Lanes |  | 1 |  | 1 |  | 1 |  | 1 |
| Adj Approach Flow, veh/h |  | 299 |  | 143 |  | 60 |  | 57 |
| Demand Flow Rate, veh/h |  | 305 |  | 146 |  | 62 |  | 58 |
| Vehicles Circulating, veh/h |  | 40 |  | 110 |  | 327 |  | 162 |
| Vehicles Exiting, veh/h |  | 180 |  | 277 |  | 18 |  | 93 |
| Follow-Up Headway, s |  | 3.186 |  | 3.186 |  | 3.186 |  | 3.186 |
| Ped Vol Crossing Leg, \#/h |  | 0 |  | 0 |  | 0 |  | 0 |
| Ped Cap Adj |  | 1.000 |  | 1.000 |  | 1.000 |  | 1.000 |
| Approach Delay, s/veh |  | 6.1 |  | 4.9 |  | 5.3 |  | 4.4 |
| Approach LOS |  | A |  | A |  | A |  | A |
| Lane | Left |  | Left |  | Left |  | Left |  |
| Designated Moves | LTR |  | LTR |  | LTR |  | LTR |  |
| Assumed Moves | LTR |  | LTR |  | LTR |  | LTR |  |
| RT Channelized |  |  |  |  |  |  |  |  |
| Lane Util | 1.000 |  | 1.000 |  | 1.000 |  | 1.000 |  |
| Critical Headway, s | 5.193 |  | 5.193 |  | 5.193 |  | 5.193 |  |
| Entry Flow, veh/h | 305 |  | 146 |  | 62 |  | 58 |  |
| Cap Entry Lane, veh/h | 1086 |  | 1012 |  | 815 |  | 961 |  |
| Entry HV Adj Factor | 0.981 |  | 0.983 |  | 0.976 |  | 0.980 |  |
| Flow Entry, veh/h | 299 |  | 143 |  | 60 |  | 57 |  |
| Cap Entry, veh/h | 1065 |  | 995 |  | 795 |  | 942 |  |
| VIC Ratio | 0.281 |  | 0.144 |  | 0.076 |  | 0.060 |  |
| Control Delay, s/veh | 6.1 |  | 4.9 |  | 5.3 |  | 4.4 |  |
| LOS | A |  | A |  | A |  | A |  |
| 95th \%tile Queue, veh | 1 |  | 1 |  | 0 |  | 0 |  |


|  | 4 | $\rightarrow$ | $\geqslant$ | 7 |  | 4 | 4 | $\dagger$ | \% | $V$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | 4 |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | F |  |
| Traffic Volume (vph) | 60 | 137 | 69 | 128 | 200 | 247 | 127 | 688 | 207 | 133 | 335 | 37 |
| Future Volume (vph) | 60 | 137 | 69 | 128 | 200 | 247 | 127 | 688 | 207 | 133 | 335 | 37 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 40.0 |  | 0.0 | 45.0 |  | 0.0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 0 | 1733 | 0 | 0 | 1728 | 0 | 1504 | 1792 | 0 | 1703 | 1822 | 0 |
| Flt Permitted |  | 0.697 |  |  | 0.830 |  | 0.450 |  |  | 0.114 |  |  |
| Satd. Flow (perm) | 0 | 1221 | 0 | 0 | 1450 | 0 | 713 | 1792 | 0 | 204 | 1822 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 25 |  |  | 54 |  |  | 26 |  |  | 10 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 293 | 0 | 0 | 632 | 0 | 140 | 983 | 0 | 146 | 409 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 | 15.0 |  | 15.0 | 15.0 |  | 30.0 | 30.0 |  | 30.0 | 30.0 |  |
| Minimum Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Total Split (s) | 34.5 | 34.5 |  | 34.5 | 34.5 |  | 42.5 | 42.5 |  | 42.5 | 42.5 |  |
| Total Split (\%) | 44.8\% | 44.8\% |  | 44.8\% | 44.8\% |  | 55.2\% | 55.2\% |  | 55.2\% | 55.2\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 7.5 |  |  | 7.5 |  | 7.5 | 7.5 |  | 7.5 | 7.5 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) |  | 27.0 |  |  | 27.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 |  |
| Actuated g/C Ratio |  | 0.35 |  |  | 0.35 |  | 0.45 | 0.45 |  | 0.45 | 0.45 |  |
| v/c Ratio |  | 0.66 |  |  | 1.16 |  | 0.43 | 1.19 |  | 1.59 | 0.49 |  |
| Control Delay |  | 27.7 |  |  | 117.3 |  | 19.5 | 119.2 |  | 333.1 | 16.9 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 27.7 |  |  | 117.3 |  | 19.5 | 119.2 |  | 333.1 | 16.9 |  |
| LOS |  | C |  |  | F |  | B | F |  | F | B |  |
| Approach Delay |  | 27.7 |  |  | 117.3 |  |  | 106.8 |  |  | 100.1 |  |
| Approach LOS |  | C |  |  | F |  |  | F |  |  | F |  |
| Queue Length 50th (m) |  | 34.1 |  |  | $\sim 111.8$ |  | 14.0 | ~183.6 |  | ~32.4 | 41.0 |  |
| Queue Length 95th (m) |  | 62.2 |  |  | \#175.8 |  | 30.0 | \#256.6 |  | \#54.8 | 65.4 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) |  |  |  |  |  |  | 40.0 |  |  | 45.0 |  |  |
| Base Capacity (vph) |  | 444 |  |  | 543 |  | 324 | 828 |  | 92 | 833 |  |



Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22



|  | 4 |  |  | 7 |  | 4 | 4 | 4 | 7 |  | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 6 | 0 | 2 | 14 | 0 | 9 | 17 | 695 | 20 | 14 | 416 | 10 |
| Future Volume (Veh/h) | 6 | 0 | 2 | 14 | 0 | 9 | 17 | 695 | 20 | 14 | 416 | 10 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 6 | 0 | 2 | 14 | 0 | 9 | 18 | 716 | 21 | 14 | 429 | 10 |
| Pedestrians 1105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) 3.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{array}{lrr}\text { Walking Speed (m/s) } & 1.2 & 1.2 \\ \text { Percent Blockage } & 0 & 0\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type None |  |  |  |  |  |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1234 | 1236 | 439 | 1232 | 1230 | 728 | 439 |  |  | 738 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1234 | 1236 | 439 | 1232 | 1230 | 728 | 439 |  |  | 738 |  |  |
|  | 7.1 | 6.5 | 6.2 | 7.2 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.6 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 96 | 100 | 100 | 90 | 100 | 98 | 98 |  |  | 98 |  |  |
| cM capacity (veh/h) | 148 | 172 | 620 | 145 | 173 | 427 | 1132 |  |  | 877 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 8 | 23 | 755 | 453 |  |  |  |  |  |  |  |  |
| Volume Left | 6 | 14 | 18 | 14 |  |  |  |  |  |  |  |  |
| Volume Right | 2 | 9 | 21 | 10 |  |  |  |  |  |  |  |  |
|  | 183 | 195 | 1132 | 877 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.04 | 0.12 | 0.02 | 0.02 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 1.1 | 3.2 | 0.4 | 0.4 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 25.6 | 25.9 | 0.4 | 0.5 |  |  |  |  |  |  |  |  |
|  | D | D | A | A |  |  |  |  |  |  |  |  |
| Lane LOS <br> Approach Delay (s) | 25.6 | 25.9 | 0.4 | 0.5 |  |  |  |  |  |  |  |  |
| Approach Delay (s) Approach LOS | D | D |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 1.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 56.4\% |  | CU Level | Service |  |  | B |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |




| Intersection |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh | 4.9 |  |  |  |  |  |  |  |
| Intersection LOS | A |  |  |  |  |  |  |  |
| Approach |  | EB |  | WB |  | NB |  | SB |
| Entry Lanes |  | 1 |  | 1 |  | 1 |  | 1 |
| Conflicting Circle Lanes |  | 1 |  | 1 |  | 1 |  | 1 |
| Adj Approach Flow, veh/h |  | 152 |  | 197 |  | 31 |  | 71 |
| Demand Flow Rate, veh/h |  | 155 |  | 200 |  | 32 |  | 72 |
| Vehicles Circulating, veh/h |  | 43 |  | 90 |  | 156 |  | 177 |
| Vehicles Exiting, veh/h |  | 206 |  | 98 |  | 42 |  | 113 |
| Follow-Up Headway, s |  | 3.186 |  | 3.186 |  | 3.186 |  | 3.186 |
| Ped Vol Crossing Leg, \#/h |  | 0 |  | 0 |  | 0 |  | 0 |
| Ped Cap Adj |  | 1.000 |  | 1.000 |  | 1.000 |  | 1.000 |
| Approach Delay, s/veh |  | 4.7 |  | 5.4 |  | 4.1 |  | 4.6 |
| Approach LOS |  | A |  | A |  | A |  | A |
| Lane | Left |  | Left |  | Left |  | Left |  |
| Designated Moves | LTR |  | LTR |  | LTR |  | LTR |  |
| Assumed Moves | LTR |  | LTR |  | LTR |  | LTR |  |
| RT Channelized |  |  |  |  |  |  |  |  |
| Lane Util | 1.000 |  | 1.000 |  | 1.000 |  | 1.000 |  |
| Critical Headway, s | 5.193 |  | 5.193 |  | 5.193 |  | 5.193 |  |
| Entry Flow, veh/h | 155 |  | 200 |  | 32 |  | 72 |  |
| Cap Entry Lane, veh/h | 1082 |  | 1033 |  | 967 |  | 947 |  |
| Entry HV Adj Factor | 0.983 |  | 0.985 |  | 0.983 |  | 0.982 |  |
| Flow Entry, veh/h | 152 |  | 197 |  | 31 |  | 71 |  |
| Cap Entry, veh/h | 1064 |  | 1017 |  | 951 |  | 930 |  |
| V/C Ratio | 0.143 |  | 0.194 |  | 0.033 |  | 0.076 |  |
| Control Delay, s/veh | 4.7 |  | 5.4 |  | 4.1 |  | 4.6 |  |
| LOS | A |  | A |  | A |  | A |  |
| 95th \%tile Queue, veh | 0 |  | 1 |  | 0 |  | 0 |  |

HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North \& Wellington Road 22

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| $\rangle$ |  |  |  |  |  | 4 | $\uparrow$ | 7 | , | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio 0.14 | 0.89 |  | 0.72 | 0.42 |  | 0.19 | 0.47 |  | 0.73 | 0.80 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other | Other |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 65 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 64.9 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 65 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.89 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 28.1 |  |  |  | Intersection LOS: C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 79.4\% |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two | ycles. |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - AM - with Improvements 3: Trafalgar Road North \& Street 'A'/Howe Street

|  | 4 | $\rightarrow$ | $\checkmark$ | 7 |  | 4 | 4 | $\dagger$ | \% | ( | $\frac{1}{1}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | * |  | ${ }^{7}$ | F |  | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Volume (veh/h) | 14 | 0 | 195 | 1 | 27 | 4 | 79 | 177 | 1 | 4 | 224 | 2 |
| Future Volume (Veh/h) | 14 | 0 | 195 | 1 | 27 | 4 | 79 | 177 | 1 | 4 | 224 | 2 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 14 | 0 | 201 | 1 | 28 | 4 | 81 | 182 | 1 | 4 | 231 | 2 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 602 | 585 | 232 | 784 | 586 | 182 | 233 |  |  | 183 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 602 | 585 | 232 | 784 | 586 | 182 | 233 |  |  | 183 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 8.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 4.4 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 96 | 100 | 75 | 99 | 93 | 100 | 94 |  |  | 100 |  |  |
| cM capacity (veh/h) | 369 | 396 | 807 | 156 | 396 | 865 | 1335 |  |  | 1404 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |  |  |
| Volume Total | 215 | 33 | 81 | 183 | 4 | 233 |  |  |  |  |  |  |
| Volume Left | 14 | 1 | 81 | 0 | 4 | 0 |  |  |  |  |  |  |
| Volume Right | 201 | 4 | 0 | 1 | 0 | 2 |  |  |  |  |  |  |
| cSH | 749 | 404 | 1335 | 1700 | 1404 | 1700 |  |  |  |  |  |  |
| Volume to Capacity | 0.29 | 0.08 | 0.06 | 0.11 | 0.00 | 0.14 |  |  |  |  |  |  |
| Queue Length 95th (m) | 9.5 | 2.1 | 1.5 | 0.0 | 0.1 | 0.0 |  |  |  |  |  |  |
| Control Delay (s) | 11.7 | 14.7 | 7.9 | 0.0 | 7.6 | 0.0 |  |  |  |  |  |  |
| Lane LOS | B | B | A |  | A |  |  |  |  |  |  |  |
| Approach Delay (s) | 11.7 | 14.7 | 2.4 |  | 0.1 |  |  |  |  |  |  |  |
| Approach LOS | B | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 4.9 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 45.4\% |  | U Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  | 14: Trafalgar Road North \& Street 'E'



HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North \& Wellington Road 22

|  | $\rangle$ | $\rightarrow$ |  | 7 |  |  | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | $\dagger$ |  | \% | $\hat{\dagger}$ |  | ${ }^{*}$ | ¢ |  | ${ }_{1}$ | $\hat{\beta}$ |  |
| Traffic Volume (vph) | 54 | 130 | 68 | 122 | 192 | 217 | 123 | 629 | 198 | 116 | 306 | 34 |
| Future Volume (vph) | 54 | 130 | 68 | 122 | 192 | 217 | 123 | 629 | 198 | 116 | 306 | 34 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 35.0 |  | 0.0 | 45.0 |  | 0.0 | 40.0 |  | 0.0 | 65.0 |  | 0.0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length ( m ) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 1805 | 1697 | 0 | 1736 | 1714 | 0 | 1504 | 1791 | 0 | 1703 | 1822 | 0 |
| FIt Permitted | 0.186 |  |  | 0.566 |  |  | 0.540 |  |  | 0.083 |  |  |
| Satd. Flow (perm) | 353 | 1697 | 0 | 1034 | 1714 | 0 | 855 | 1791 | 0 | 149 | 1822 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 28 |  |  | 59 |  |  | 25 |  |  | 11 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 59 | 218 | 0 | 134 | 449 | 0 | 135 | 909 | 0 | 127 | 373 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | pm+pt | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 |  | 2 | 2 |  | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 12.0 | 12.0 |  | 12.0 | 12.0 |  | 8.0 | 8.0 |  | 5.0 | 8.0 |  |
| Minimum Split (s) | 19.5 | 19.5 |  | 19.5 | 19.5 |  | 22.7 | 22.7 |  | 8.0 | 22.7 |  |
| Total Split (s) | 29.0 | 29.0 |  | 29.0 | 29.0 |  | 53.0 | 53.0 |  | 8.0 | 61.0 |  |
| Total Split (\%) | 32.2\% | 32.2\% |  | 32.2\% | 32.2\% |  | 58.9\% | 58.9\% |  | 8.9\% | 67.8\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 3.0 | 5.5 |  |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.3 | 2.3 |  | 0.0 | 2.3 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 7.5 | 7.5 |  | 7.5 | 7.5 |  | 7.8 | 7.8 |  | 3.0 | 7.8 |  |
| Lead/Lag |  |  |  |  |  |  | Lag | Lag |  | Lead |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  | Yes | Yes |  | Yes |  |  |
| Recall Mode | None | None |  | None | None |  | Max | Max |  | None | Max |  |
| Act Effct Green (s) | 21.5 | 21.5 |  | 21.5 | 21.5 |  | 45.2 | 45.2 |  | 58.0 | 53.2 |  |
| Actuated g/C Ratio | 0.24 | 0.24 |  | 0.24 | 0.24 |  | 0.50 | 0.50 |  | 0.64 | 0.59 |  |
| v/c Ratio | 0.70 | 0.51 |  | 0.54 | 0.99 |  | 0.31 | 1.00 |  | 0.70 | 0.35 |  |
| Control Delay | 75.4 | 30.7 |  | 39.4 | 71.3 |  | 15.8 | 52.8 |  | 32.0 | 10.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 75.4 | 30.7 |  | 39.4 | 71.3 |  | 15.8 | 52.8 |  | 32.0 | 10.2 |  |
| LOS | E | C |  | D | E |  | B | D |  | C | B |  |
| Approach Delay |  | 40.2 |  |  | 64.0 |  |  | 48.0 |  |  | 15.8 |  |
| Approach LOS |  | D |  |  | E |  |  | D |  |  | B |  |
| Queue Length 50th (m) | 9.8 | 29.8 |  | 21.4 | 72.4 |  | 13.8 | 154.4 |  | 7.9 | 31.0 |  |
| Queue Length 95th (m) | \#31.7 | 52.7 |  | 40.8 | \#135.7 |  | 27.1 | \#245.9 |  | \#21.7 | 48.1 |  |
| Internal Link Dist ( m ) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) | 35.0 |  |  | 45.0 |  |  | 40.0 |  |  | 65.0 |  |  |
| Base Capacity (vph) | 84 | 426 |  | 247 | 454 |  | 429 | 911 |  | 182 | 1081 |  |



Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


HCM Un-signalized Intersection Capacity Analysis 2026 Future Total Traffic - PM - with Improvements 3: Trafalgar Road North \& Street 'A'/Howe Street

|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | $\dagger$ | 7 |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{1}$ | $\dagger$ |  |
| Traffic Volume (veh/h) | 4 | 6 | 53 | 4 | 0 | 0 | 101 | 412 | 10 | 6 | 288 | 9 |
| Future Volume (Veh/h) | 4 | 6 | 53 | 4 | 0 | 0 | 101 | 412 | 10 | 6 | 288 | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 4 | 7 | 58 | 4 | 0 | 0 | 110 | 448 | 11 | 7 | 313 | 10 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1000 | 1011 | 318 | 1062 | 1010 | 454 | 323 |  |  | 459 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1000 | 1011 | 318 | 1062 | 1010 | 454 | 323 |  |  | 459 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 98 | 97 | 92 | 98 | 100 | 100 | 91 |  |  | 99 |  |  |
| cM capacity (veh/h) | 206 | 217 | 723 | 169 | 217 | 611 | 1237 |  |  | 1113 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |  |  |
| Volume Total | 69 | 4 | 110 | 459 | 7 | 323 |  |  |  |  |  |  |
| Volume Left | 4 | 4 | 110 | 0 | 7 | 0 |  |  |  |  |  |  |
| Volume Right | 58 | 0 | 0 | 11 | 0 | 10 |  |  |  |  |  |  |
| cSH | 523 | 169 | 1237 | 1700 | 1113 | 1700 |  |  |  |  |  |  |
| Volume to Capacity | 0.13 | 0.02 | 0.09 | 0.27 | 0.01 | 0.19 |  |  |  |  |  |  |
| Queue Length 95th (m) | 3.6 | 0.6 | 2.3 | 0.0 | 0.2 | 0.0 |  |  |  |  |  |  |
| Control Delay (s) | 12.9 | 26.8 | 8.2 | 0.0 | 8.3 | 0.0 |  |  |  |  |  |  |
| Lane LOS | B | D | A |  | A |  |  |  |  |  |  |  |
| Approach Delay (s) | 12.9 | 26.8 | 1.6 |  | 0.2 |  |  |  |  |  |  |  |
| Approach LOS | B | D |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 2.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 39.3\% |  | U Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  | 14: Trafalgar Road North \& Street 'E'



HCM Signalized Intersection Capacity Analysis 2031 Future Total Traffic - AM - with Improvements 11: Trafalgar Road North \& Wellington Road 22

|  | 4 |  |  |  |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ | 「 | ${ }^{7}$ | 4 | " | ${ }^{*}$ | $\hat{1}$ |  |
| Traffic Volume (vph) | 33 | 178 | 124 | 176 | 91 | 76 | 40 | 214 | 100 | 264 | 522 | 33 |
| Future Volume (vph) | 33 | 178 | 124 | 176 | 91 | 76 | 40 | 214 | 100 | 264 | 522 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 35.0 |  | 0.0 | 45.0 |  | 35.0 | 40.0 |  | 20.0 | 70.0 |  | 0.0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 1656 | 1733 | 0 | 1504 | 1810 | 1346 | 1805 | 1597 | 1482 | 1492 | 1676 | 0 |
| Flt Permitted | 0.689 |  |  | 0.291 |  |  | 0.244 |  |  | 0.606 |  |  |
| Satd. Flow (perm) | 1201 | 1733 | 0 | 461 | 1810 | 1346 | 464 | 1597 | 1482 | 952 | 1676 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 46 |  |  |  | 87 |  |  | 120 |  | 6 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Heavy Vehicles (\%) | 9\% | 2\% | 4\% | 20\% | 5\% | 20\% | 0\% | 19\% | 9\% | 21\% | 12\% | 18\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 38 | 348 | 0 | 202 | 105 | 87 | 46 | 246 | 115 | 303 | 638 | 0 |
| Turn Type | Perm | NA |  | pm+pt | NA | Perm | Perm | NA | Perm | Perm | NA |  |
| Protected Phases |  | 4 |  | 3 | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 |  | 3 | 8 | 8 | 2 | 2 | 2 | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 12.0 | 12.0 |  | 5.0 | 12.0 | 12.0 | 8.0 | 8.0 | 8.0 | 8.0 | 8.0 |  |
| Minimum Split (s) | 22.6 | 22.6 |  | 8.0 | 22.6 | 22.6 | 25.9 | 25.9 | 25.9 | 25.9 | 25.9 |  |
| Total Split (s) | 23.0 | 23.0 |  | 8.0 | 31.0 | 31.0 | 39.0 | 39.0 | 39.0 | 39.0 | 39.0 |  |
| Total Split (\%) | 32.9\% | 32.9\% |  | 11.4\% | 44.3\% | 44.3\% | 55.7\% | 55.7\% | 55.7\% | 55.7\% | 55.7\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 3.0 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.2 | 2.2 |  | 0.0 | 2.2 | 2.2 | 2.6 | 2.6 | 2.6 | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 7.7 | 7.7 |  | 3.0 | 7.7 | 7.7 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 |  |
| Lead/Lag | Lag | Lag |  | Lead |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? | Yes | Yes |  | Yes |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None | None | Max | Max | Max | Max | Max |  |
| Act Effict Green (s) | 14.8 | 14.8 |  | 27.5 | 22.8 | 22.8 | 30.9 | 30.9 | 30.9 | 30.9 | 30.9 |  |
| Actuated g/C Ratio | 0.21 | 0.21 |  | 0.40 | 0.33 | 0.33 | 0.44 | 0.44 | 0.44 | 0.44 | 0.44 |  |
| v/c Ratio | 0.15 | 0.86 |  | 0.79 | 0.18 | 0.17 | 0.22 | 0.35 | 0.16 | 0.72 | 0.85 |  |
| Control Delay | 23.9 | 45.6 |  | 40.6 | 17.6 | 5.4 | 15.7 | 14.6 | 3.1 | 28.0 | 31.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 23.9 | 45.6 |  | 40.6 | 17.6 | 5.4 | 15.7 | 14.6 | 3.1 | 28.0 | 31.0 |  |
| LOS | C | D |  | D | B | A | B | B | A | C | C |  |
| Approach Delay |  | 43.4 |  |  | 26.7 |  |  | 11.5 |  |  | 30.0 |  |
| Approach LOS |  | D |  |  | C |  |  | B |  |  | C |  |
| Queue Length 50th (m) | 4.2 | 40.2 |  | 18.7 | 10.1 | 0.0 | 3.8 | 21.4 | 0.0 | 32.7 | 75.1 |  |
| Queue Length 95th (m) | 11.4 | \#79.2 |  | \#43.5 | 20.0 | 8.1 | 10.6 | 36.1 | 7.1 | \#68.1 | \#130.0 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length ( m ) | 35.0 |  |  | 45.0 |  | 35.0 | 40.0 |  | 20.0 | 70.0 |  |  |
| Base Capacity (vph) | 264 | 417 |  | 257 | 606 | 509 | 206 | 709 | 725 | 423 | 748 |  |



Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - AM - with Improvements 3: Trafalgar Road North \& Street 'A'/Howe Street

|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | $\dagger$ | 7 | , | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\hat{F}$ |  |
| Traffic Volume (veh/h) | 18 | 0 | 227 | 1 | 39 | 4 | 95 | 216 | 1 | 12 | 257 | 3 |
| Future Volume (Veh/h) | 18 | 0 | 227 | 1 | 39 | 4 | 95 | 216 | 1 | 12 | 257 | 3 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 19 | 0 | 234 | 1 | 40 | 4 | 98 | 223 | 1 | 12 | 265 | 3 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 734 | 710 | 266 | 942 | 712 | 224 | 268 |  |  | 224 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 734 | 710 | 266 | 942 | 712 | 224 | 268 |  |  | 224 |  |  |
| $\begin{array}{llll}\text { tC, single (s) } & 7.1 & 6.5 & 6.2\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{tc}, 2 \text { stage (s) }$ |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 4.4 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 93 | 100 | 70 | 99 | 88 | 100 | 92 |  |  | 99 |  |  |
| cM capacity (veh/h) | 284 | 328 | 772 | 108 | 328 | 821 | 1296 |  |  | 1357 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |  |  |
| Volume Total | 253 | 45 | 98 | 224 | 12 | 268 |  |  |  |  |  |  |
| Volume Left | 19 | 1 | 98 | 0 | 12 | 0 |  |  |  |  |  |  |
| Volume Right | 234 | 4 | 0 | 1 | 0 | 3 |  |  |  |  |  |  |
|  | 684 | 331 | 1296 | 1700 | 1357 | 1700 |  |  |  |  |  |  |
| cSH <br> Volume to Capacity | 0.37 | 0.14 | 0.08 | 0.13 | 0.01 | 0.16 |  |  |  |  |  |  |
| Queue Length 95th (m) | 13.7 | 3.7 | 2.0 | 0.0 | 0.2 | 0.0 |  |  |  |  |  |  |
| Control Delay (s) | 13.3 | 17.6 | 8.0 | 0.0 | 7.7 | 0.0 |  |  |  |  |  |  |
|  | B | C | A |  | A |  |  |  |  |  |  |  |
| Approach Delay (s) | 13.3 | 17.6 | 2.4 |  | 0.3 |  |  |  |  |  |  |  |
| Approach LOS | B | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 5.6 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 50.7\% |  | U Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  | 14: Trafalgar Road North \& Street 'E'



HCM Signalized Intersection Capacity Analysis 11: Trafalgar Road North \& Wellington Road 22

|  | 4 | $\rightarrow$ |  | 7 |  |  | $4$ | $\dagger$ | $p$ |  | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{1}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 | ${ }^{*}$ | F |  |
| Traffic Volume (vph) | 60 | 137 | 69 | 128 | 200 | 247 | 127 | 688 | 207 | 133 | 335 | 37 |
| Future Volume (vph) | 60 | 137 | 69 | 128 | 200 | 247 | 127 | 688 | 207 | 133 | 335 | 37 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 35.0 |  | 0.0 | 45.0 |  | 35.0 | 40.0 |  | 20.0 | 70.0 |  | 0.0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 | 1 |  | 0 |
| Taper Length (m) | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  | 7.5 |  |  |
| Satd. Flow (prot) | 1805 | 1703 | 0 | 1736 | 1863 | 1583 | 1504 | 1845 | 1615 | 1703 | 1822 | 0 |
| Flt Permitted | 0.621 |  |  | 0.617 |  |  | 0.512 |  |  | 0.208 |  |  |
| Satd. Flow (perm) | 1180 | 1703 | 0 | 1127 | 1863 | 1583 | 811 | 1845 | 1615 | 373 | 1822 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 40 |  |  |  | 142 |  |  | 123 |  | 13 |  |
| Link Speed (k/h) |  | 70 |  |  | 70 |  |  | 40 |  |  | 40 |  |
| Link Distance (m) |  | 304.3 |  |  | 341.1 |  |  | 247.9 |  |  | 1456.2 |  |
| Travel Time (s) |  | 15.6 |  |  | 17.5 |  |  | 22.3 |  |  | 131.1 |  |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles (\%) | 0\% | 2\% | 14\% | 4\% | 2\% | 2\% | 20\% | 3\% | 0\% | 6\% | 3\% | 0\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 66 | 227 | 0 | 141 | 220 | 271 | 140 | 756 | 227 | 146 | 409 | 0 |
| Turn Type | Perm | NA |  | Perm | NA | Perm | Perm | NA | Perm | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 | 8 | 2 | 2 | 2 | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 12.0 | 12.0 |  | 12.0 | 12.0 | 12.0 | 8.0 | 8.0 | 8.0 | 8.0 | 8.0 |  |
| Minimum Split (s) | 22.6 | 22.6 |  | 22.6 | 22.6 | 22.6 | 25.9 | 25.9 | 25.9 | 25.9 | 25.9 |  |
| Total Split (s) | 22.6 | 22.6 |  | 22.6 | 22.6 | 22.6 | 37.4 | 37.4 | 37.4 | 37.4 | 37.4 |  |
| Total Split (\%) | 37.7\% | 37.7\% |  | 37.7\% | 37.7\% | 37.7\% | 62.3\% | 62.3\% | 62.3\% | 62.3\% | 62.3\% |  |
| Yellow Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |  |
| All-Red Time (s) | 2.2 | 2.2 |  | 2.2 | 2.2 | 2.2 | 2.6 | 2.6 | 2.6 | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 7.7 | 7.7 |  | 7.7 | 7.7 | 7.7 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None |  | None | None | None | Max | Max | Max | Max | Max |  |
| Act Effct Green (s) | 13.6 | 13.6 |  | 13.6 | 13.6 | 13.6 | 29.3 | 29.3 | 29.3 | 29.3 | 29.3 |  |
| Actuated g/C Ratio | 0.23 | 0.23 |  | 0.23 | 0.23 | 0.23 | 0.50 | 0.50 | 0.50 | 0.50 | 0.50 |  |
| v/c Ratio | 0.24 | 0.54 |  | 0.54 | 0.51 | 0.57 | 0.35 | 0.82 | 0.26 | 0.78 | 0.45 |  |
| Control Delay | 20.9 | 21.3 |  | 28.5 | 24.3 | 15.0 | 12.4 | 22.8 | 5.1 | 47.3 | 11.4 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 20.9 | 21.3 |  | 28.5 | 24.3 | 15.0 | 12.4 | 22.8 | 5.1 | 47.3 | 11.4 |  |
| LOS | C | C |  | C | C | B | B | C | A | D | B |  |
| Approach Delay |  | 21.2 |  |  | 21.2 |  |  | 17.9 |  |  | 20.8 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | C |  |
| Queue Length 50th (m) | 6.1 | 18.3 |  | 14.0 | 21.7 | 12.2 | 9.1 | 68.6 | 6.0 | 12.9 | 27.2 |  |
| Queue Length 95th (m) | 15.3 | 36.9 |  | 29.9 | 39.9 | 32.3 | 21.3 | \#135.6 | 16.5 | \#44.6 | 47.8 |  |
| Internal Link Dist (m) |  | 280.3 |  |  | 317.1 |  |  | 223.9 |  |  | 1432.2 |  |
| Turn Bay Length (m) | 35.0 |  |  | 45.0 |  | 35.0 | 40.0 |  | 20.0 | 70.0 |  |  |
| Base Capacity (vph) | 299 | 461 |  | 286 | 473 | 507 | 405 | 920 | 867 | 186 | 916 |  |


| $\rangle$ |  |  |  |  |  | 4 | $\uparrow$ | 7 | , | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v/c Ratio 0.22 | 0.49 |  | 0.49 | 0.47 | 0.53 | 0.35 | 0.82 | 0.26 | 0.78 | 0.45 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other | Other |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 60 |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 58.7 |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 60 |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.82 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 19.7 |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 91.3\% |  |  |  | ICU Level of Service F |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two | ycles. |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Trafalgar Road North \& Wellington Road 22


HCM Un-signalized Intersection Capacity Analysis 2031 Future Total Traffic - PM - with Improvements 3: Trafalgar Road North \& Street 'A'/Howe Street

|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 | \% | ( | $\frac{1}{*}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | F |  |
| Traffic Volume (veh/h) | 7 | 6 | 72 | 4 | 14 | 0 | 146 | 476 | 10 | 22 | 331 | 13 |
| Future Volume (Veh/h) | 7 | 6 | 72 | 4 | 14 | 0 | 146 | 476 | 10 | 22 | 331 | 13 |
| Sign Control |  | Stop |  | Stop |  |  | Free |  |  | Free |  |  |
| Grade | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 8 | 7 | 78 | 4 | 15 | 0 | 159 | 517 | 11 | 24 | 360 | 14 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1258 | 1261 | 367 | 1330 | 1262 | 522 | 374 |  |  | 528 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 1258 | 1261 | 367 | 1330 | 1262 | 522 | 374 | 528 |  |  |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | 4.1 |  |  |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | 2.2 |  |  |  |  |
| p0 queue free \% | 93 | 95 | 89 | 96 | 90 | 100 | 87 | 98 |  |  |  |  |
| cM capacity (veh/h) | 120 | 144 | 678 | 100 | 144 | 558 | 1184 | 1049 |  |  |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |  |  |  |  |  |  |
| Volume Total | 93 | 19 | 159 | 528 | 24 | 374 |  |  |  |  |  |  |
| Volume Left | 8 | 4 | 159 | 0 | 24 | 0 |  |  |  |  |  |  |
| Volume Right | 78 | 0 | 0 | 11 | 0 | 14 |  |  |  |  |  |  |
|  | 404 | 132 | 1184 | 1700 | 1049 | 1700 |  |  |  |  |  |  |
| cSH <br> Volume to Capacity | 0.23 | 0.14 | 0.13 | 0.31 | 0.02 | 0.22 |  |  |  |  |  |  |
| Queue Length 95th (m) | 7.0 | 3.9 | 3.7 | 0.0 | 0.6 | 0.0 |  |  |  |  |  |  |
| Control Delay (s) | 16.6 | 36.9 | 8.5 | 0.0 | 8.5 | 0.0 |  |  |  |  |  |  |
|  | C | E | A |  | A |  |  |  |  |  |  |  |
| Lane LOS | 16.6 | 36.9 | 2.0 |  | 0.5 |  |  |  |  |  |  |  |
| Approach LOS | C | E |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 3.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 44.6\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  | 14: Trafalgar Road North \& Street 'E'



## APPENDIX F

TRANSPORTATION TOMORROW SURVEY DATABASE QUERY

Tue Nov 092021 13:34:54 GMT-0500 (Eastern Standard Time) - Run Time: 2485ms

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig
Column: 2006 GTA zone of destination - gta06_dest

Filters:
(2006 GTA zone of origin - gta06_orig In 8370
and
Start time of trip - start_time In 700-859
and
Primary travel mode of trip - mode_prime In D
and
Trip purpose of origin - purp_orig In H

Trip 2016
Table:

| Location | Toronto | Brampton | Brampton | Brampton | Halton | Internal |  |  | Erin Village |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- | :---: | :---: |
|  | 299 | 3332 | 3375 | 3462 | 4183 | 8370 | 8380 Total |  |  |  |  |
| 8370 | 11 | 22 | 8 | 82 | 57 | 41 | 28 | 249 |  |  |  |
| Percentage | $4 \%$ | $9 \%$ | $3 \%$ | $33 \%$ | $23 \%$ | $16 \%$ | $11 \%$ |  |  |  |  |

Wed Nov 102021 13:45:46 GMT-0500 (Eastern Standard Time) - Run Time: 2788ms
Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig
Column: 2006 GTA zone of destination - gta06_dest

Filters:
(2006 GTA zone of origin - gta06_orig In 8370, 8371, 8373
and
Start time of trip - start_time In 700-859
and
Primary travel mode of trip - mode_prime In D
and
Trip purpose of origin - purp_orig $\ln \mathrm{H}$ )
Trip 2016
Table:

|  | Toronto | Toronto | Markham | Brampton | Brampton | Brampton | Brampton | Brampton | Halton | Halton | Guelph | Wellington | rnal | Erin Villag $\epsilon$ | Dufferin | External |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 299 | 309 | 2393 | 3332 | 3343 | 3375 | 3462 | 3721 | 4160 | 4183 | 8024 | 8365 | 8370 | 8380 | 8417 | 9057 |
| 8370 | 11 | 0 | 0 | 22 | 0 | 8 | 82 | 0 | 0 | 57 | 0 | 0 | 41 | 28 | 0 | 0 |
| 8371 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8373 | 0 | 0 | 8 | 0 | 46 | 0 | 0 | 0 | 8 | 0 | 30 | 17 | 0 | 0 | 17 | 17 |
|  | 11 | 12 | 8 | 22 | 46 | 8 | 82 | 12 | 8 | 57 | 30 | 17 | 41 | 28 | 17 | 17 |
|  | 3\% | 3\% | 2\% | 6\% | 12\% | 2\% | 21\% | 3\% | 2\% | 14\% | 8\% | 4\% | 10\% | 7\% | 4\% | 4\% |
| Trip Assignment | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 2 | 1 | 3 |  |

$150 \%$ south via Trafalgar 50\% east via Wellington Road 22
2 30\% east via Wellington Road 22 and 70\% south and within the Hillsburgh BA
3 north via Trafalgar

## APPENDIX G

## SIGNAL WARRANT ANALYSIS

|  | Trafalgar Road North at George <br> Street/Mill Street - Future (2026) <br> Total Traffic |
| :--- | :---: |
| Intersection |  |
| Number of Lanes on Main <br> Road (1 $\mathbf{2}$ 2 lane 2= more <br> than 2 lanes) | 1 |
| Rural (enter 1) or Urban <br> (enter 2) | 2 |
| Existing (enter 1) or New <br> (enter 2) intersection | 1 |
| T Intersection (yes =1 no $=$ <br> 2) | 2 |


| Peak Hour | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM | 12 | 262 | 14 | 2 | 617 | 1 | 5 | 1 | 8 | 34 | 0 | 36 |
| PM | 71 | 596 | 54 | 22 | 372 | 8 | 23 | 16 | 48 | 34 | 13 | 22 |
| Average Hourly Volume | 21 | 215 | 17 | 6 | 247 | 2 | 7 | 4 | 14 | 17 | 3 | 15 |


|  |  | Minimum Requirement 1 Lane |  | Minimum Requirement 2 lane |  | Initial Requirment | T intersection Factor | Existing/New Intersection Factor | Scenario Requirement | Scenario Volume |  |  | Compliance \% |  |  | Justification |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Justification | Description | Rural | Urban | Rural | Urban |  |  |  |  | AM | PM | AHV | AM | PM | AHV |  |
| 1A Minimum Veh. Volume | All Approaches | 480 | 720 | 600 | 900 | 720 | 1 | 1.2 | 864 | 992 | 1279 | 568 | 100\% | 100\% | 66\% | No |
| 1B Minimum Veh. Volume | Minor Street | 120 | 170 | 120 | 170 | 170 | 1 | 1.2 | 204 | 84 | 156 | 60 | 41\% | 76\% | 29\% |  |
| 2A Crossing Traffic | Major Street Volume | 480 | 720 | 600 | 900 | 720 | 1 | 1.2 | 864 | 908 | 1123 | 508 | 100\% | 100\% | 59\% |  |
| 2B Crossing Traffic | Crossing volume of Minor Street | 50 | 75 | 120 | 170 | 75 | 1 | 1.2 | 90 | 40 | 73 | 28.25 | 44\% | 81\% | 31\% | No |


|  | Trafalgar Road North at George <br> Street/Mill Street - Future (2031) <br> Total Traffic |
| :--- | :---: |
| Intersection | 1 |
| Number of Lanes on Main <br> Road (1 = 2 lane 2 $=$ more <br> than 2 lanes | 2 |
| Rural (enter 1) or Urban <br> (enter 2) | 1 |
| Existing (enter 1) or New <br> (enter 2) intersection | 2 |
| T Intersection (yes =1 no $=$ <br> 2) |  |


| Peak Hour | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM | 12 | 292 | 14 | 2 | 683 | 1 | 4 | 1 | 8 | 34 | 0 | 34 |
| PM | 71 | 596 | 54 | 22 | 372 | 8 | 23 | 16 | 48 | 34 | 13 | 22 |
| Average Hourly Volume | 21 | 222 | 17 | 6 | 264 | 2 | 7 | 4 | 14 | 17 | 3 | 14 |


|  |  | Minimum Requirement 1 Lane |  | Minimum Requirement 2 lane |  | Initial Requirment | T intersection Factor | Existing /New Intersection Factor | Scenario Requirement | Scenario Volume |  |  | Compliance \% |  |  | Justification |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Justification | Description | Rural | Urban | Rural | Urban |  |  |  |  | AM | PM | AHV | AM | PM | AHV |  |
| 1A Minimum Veh. Volume | All Approaches | 480 | 720 | 600 | 900 | 720 | 1 | 1.2 | 864 | 1085 | 1279 | 591 | 100\% | 100\% | 68\% | No |
| 1B Minimum Veh. Volume | Minor Street | 120 | 170 | 120 | 170 | 170 | 1 | 1.2 | 204 | 81 | 156 | 59 | 40\% | 76\% | 29\% |  |
| 2A Crossing Traffic | Major Street Volume | 480 | 720 | 600 | 900 | 720 | 1 | 1.2 | 864 | 1004 | 1123 | 532 | 100\% | 100\% | 62\% |  |
| 2B Crossing Traffic | Crossing volume of Minor Street | 50 | 75 | 120 | 170 | 75 | 1 | 1.2 | 90 | 39 | 73 | 28 | 43\% | 81\% | 31\% | No |

## APPENDIX H

## SITE VISIT PICTURES FOR SIGHT DISTANCE MEASUREMENTS



PROPOSED STREET 'A'/HOWE STREET AT TRAFALGAR ROAD NORTH - LOOKING NORTH


PROPOSED STREET 'A'/HOWE STREET AT TRAFALGAR ROAD NORTH - LOOKING SOUTH


PROPOSED STREET 'E' AT TRAFALGAR ROAD NORTH - LOOKING NORTH


PROPOSED STREET 'E' AT TRAFALGAR ROAD NORTH - LOOKING SOUTH


[^0]:    ${ }^{1}$ E-mail dated October $21^{\text {st }} 2021$.

[^1]:    2 Town of Erin's Official Plan - Office Consolidation, Town of Erin, October 2021.

[^2]:    ${ }^{3}$ Synchro 9 Traffic Signal Optimization and Simulation Modeling Software, Version 9, Trafficware Corporation, 2014.

[^3]:    4 Trip Generation Manual, $10^{\text {th }}$ Edition, Institute of Transportation Engineers, 2017.

[^4]:    5 Trip Generation Handbook (3 $3^{\text {rd }}$ Edition), Institute of Transportation Engineers, September 2017.

[^5]:    6 Re: Development Pre-Consultation Meeting D'Angelo Property, 5916 Trafalgar Road North, Erin (Hillsburgh), Adam Laranjeiro, July 13, 2021.

[^6]:    ${ }^{7}$ Geometric Design Standards for Ontario Highways, Ministry of Transportation Ontario.

[^7]:    8 Ontario Traffic Manual Book 12 - Traffic Signals, Ministry of Transportation Ontario, March 2012.

[^8]:    9 Geometric Design Guide for Canadian Roads, Transportation Association of Canada, June 2017.

[^9]:    From: Brian Wong [brian@candevcon.com](mailto:brian@candevcon.com)
    Sent: Wednesday, October 6, 2021 10:12 AM
    To: Nick Colucci [nick.colucci@erin.ca](mailto:nick.colucci@erin.ca)
    Cc: Joe Mullan [mullan@ainleygroup.com](mailto:mullan@ainleygroup.com); David Lee [david@candevcon.com](mailto:david@candevcon.com); Diarmuid Horgan [dhorgan@candevcon.com](mailto:dhorgan@candevcon.com)
    Subject: W21081-5916 Trafalgar Road North - Terms of Reference (Town of Erin)

    ## Good Morning Nick,

    We are preparing a Traffic Impact Study for a proposed Residential Subdivision that is immediately west of Trafalgar Road North and north of Upper Canada Drive. Please find the Terms of Reference and the latest Preliminary Development Plan attached for your review and comment. In the meantime, can you please provide the Traffic Impact Study and/or the Site Plan for any anticipated background developments within the vicinity of the proposed Residential Subdivision.

[^10]:    Units. . . . . . . . . . . . . Inches

