

HYDROGEOLOGICAL INVESTIGATION PROPOSED 13-LOT SUBDIVISION

Part Lot 13, Conc 2, Town of Erin Ospringe, Ontario

SUBMITTED TO:

Spirit of Pentecost c/o Mr. Terrell Heard 3029 Clayhill Road, PO Box 20059 Mississauga, Ontario NOB 1T0

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Spirit of Pentecost c/o Mr. Terrell Heard 3029 Clayhill Road, PO Box 20059 Mississauga, Ontario NOB 1T0

Dear Mr. Heard:

RE: HYDROGEOLOGICAL INVESTIGATION PROPOSED 13-LOT RESIDENTIAL SUBDIVISION Part Lot 13, Conc 2, Town of Erin (Ospringe)

This report summarizes the results of a hydrogeological investigation completed in support of a proposed 13-lot residential subdivision located in the hamlet of Ospringe, at Part Lot 13, Concession 2, Town of Erin.

The subdivision lots would be supplied with individual water supply wells and wastewater treatment systems. This investigation characterizes the hydrogeological setting and assesses the feasibility and potential impacts of these proposed individual services.

If you have any questions or concerns regarding the report, please contact the undersigned.

Yours truly, CHUNG & VANDER DOELEN ENGINEERING LTD.

anderson.

William (Sandy) Anderson, M.Sc., P.Eng. Senior Hydrogeologist and Engineer

TABLE OF CONTENTS

1.0	INTRODUCTION	1
2.0	INVESTIGATION SCOPE	1
2.1	BACKGROUND DATA REVIEW	1
2.2	WATER WELL INVENTORY	1
2.3	BOREHOLE DRILLING & MONITORING WELL INSTALLATION	2
2.4	WATER LEVEL MONITORING	2
2.5	WELL RESPONSE TESTING	2
2.6	WATER QUALITY TESTING	2
3.0	SITE CHARACTERIZATION	3
3.1	TOPOGRAPHY & DRAINAGE	3
3.2	GEOLOGIC SETTING	3
	3.2.1 Overburden	3
	3.2.2 Bedrock	4
3.3	HYDROGEOLOGIC SETTING	
	3.3.1 Water Table Configuration & Shallow Groundwater Flow	
	3.3.2 Shallow Groundwater Quality	5
	3.3.3 Till Aquitard & Aquifer Protection	
	3.3.4 Groundwater Recharge	6
	3.3.5 Bedrock Aquifer Groundwater Flow	6
	3.3.6 Groundwater Use	
4.0	SITE SERVICING REQUIREMENTS & IMPACT ASSESSMENT	7
4.1	WASTEWATER SYSTEMS & POTENTIAL IMPACT OF EFFLUENT	7
4.2	WATER SUPPLY & POTENTIAL IMPACT OF WATER TAKING	8
4.3	POTENTIAL IMPACT TO GROUNDWATER RECHARGE	10
4.4	ASSESSMENT OF WATER TABLE AND GRADING DESIGN	10
5.0	CONCLUSIONS & RECOMMENDATIONS	10
6.0	REFERENCES	12

LIST OF FIGURES

Figure 1	Site Location	Арр А
Figure 2	Quaternary Geology	Арр А
Figure 3	Paleozoic Geology	Арр А
Figure 4	Well Inventory Map	Арр А
Figure 5	Topography and Borehole Locations	Арр А
Figure 6	Interpreted Water Table (Spring 2018)	Арр А
Figure 7	Interpreted Water Table (Summer 2018)	Арр А
Figure 8	Future Supply Well Locations for Drawdown Analysis	Арр А

LIST OF TABLES

Table 1	Summary of Monitoring Well Water Levels & Elevations	Арр В
Table 2	Well Inventory Summary	Арр В
Table 3	Drawdown Calculations at Lot 9 Well	Арр В
Table 4	Nitrate Loading Calculation	Арр В



APPENDICES

Appendix A	Figures 1 to 8
Appendix B	Tables 1 to 4
Appendix C	Borehole Logs & Grain Size Analyses
Appendix D	Water Well Records
Appendix E	Well Response Test Analyses & Thomasfield Test Well Aquifer Parameters
Appendix F	ALS Laboratory Sample Report & Thomasfield Test Well Quality
Appendix G	Pre-Development Water Balance



1.0 INTRODUCTION

This report presents the findings of a hydrogeological investigation of a proposed 13-lot residential subdivision on a 3.62-hectare property in the hamlet of Ospringe (Figure 1). The legal description of the subject property is Part of Lot 13, Concession 2, Town of Erin (formerly Township of Erin), County of Wellington.

The objectives of the investigation are as follows:

- 1. To characterize the hydrogeological setting at the site.
- 2. To assess the site conditions in relation to water supply and sewage servicing requirements and make recommendations in this regard.
- 3. To identify the groundwater and surface water receptors, evaluate the potential impacts to these receptors from the proposed water taking and sewage effluent, and make recommendations to address these impacts, where appropriate.
- 4. To identify potential opportunities, if any, for enhancing groundwater recharge during postdevelopment.

2.0 INVESTIGATION SCOPE

2.1 BACKGROUND DATA REVIEW

The following background information and reports (Section 6.0 lists the specific reports) have been considered as part of this investigation:

- Regional-scale topographic mapping (Figure 1) and detailed site topographic mapping provided by IBI Group (Figure 5).
- Quaternary (surficial overburden) and Paleozoic (bedrock) geology mapping for the area (Figures 2 and 3).
- Water Well Records (Appendix D).
- The hydrogeological investigation report for the recently-approved adjacent 60-lot Thomasfield development.
- The supporting geotechnical, sewage system, stormwater management and functional servicing reports for the subject development.

2.2 WATER WELL INVENTORY

CVD completed an inventory of private wells within 100 m of the subject property in April and May, 2018. The data from the inventory is used to support the water servicing for the subject property and to identify potential groundwater receptors surrounding the property. Figure 4 identifies all private wells within the inventory area, including those residences with a confirmed well record, those with an unconfirmed well location and includes both drilled and shallow dug wells.



2.3 BOREHOLE DRILLING & MONITORING WELL INSTALLATION

Borehole drilling and monitoring well installation were completed on May 3, 2018 at six locations (BH1 to BH6, Figure 5) to depths ranging from 5 to 7 m. These boreholes/wells were drilled/installed to investigate the <u>shallow</u> subsurface geological and water table conditions at the property.

Standard hollow stem auger (HSA) drilling and installation methods were employed using a trackmounted auger rig. No water was introduced during drilling. Five representative soil samples were later analysed for grain size distribution at the CVD soils laboratory. Appendix C provides borehole logs, well installation details and grain size analysis results.

On May 11, 2018, each monitoring well was developed using Waterra polyethylene tubing and footvalve hand pumps and surveyed for elevation and location by CVD and referenced to a manhole lid located along Wellington Road 24 (geodetic elevation 414.48 mASL).

2.4 WATER LEVEL MONITORING

Water level monitoring was conducted on May 11 and July 31, 2018. Table 1 provides a summary of the groundwater levels, calculated elevations and the fluctuations between these spring and summer events.

2.5 WELL RESPONSE TESTING

The hydraulic conductivity (or permeability) of the surficial materials at the water table were estimated using 'rising-head' well response tests completed on May 11, 2018. The data were analyzed using Aquifer Test software and the results are provided in Appendix E.

2.6 WATER QUALITY TESTING

Shallow groundwater samples were collected on May 11, 2018 from monitoring wells BH1, BH2, BH4 and BH5. The samples were analysed for chloride and nitrate by ALS Laboratory Group and the analytical report is provided in Appendix E.



3.0 SITE CHARACTERIZATION

3.1 TOPOGRAPHY & DRAINAGE

Regional topography and drainage features are shown in Figure 1. The property is situated on top of and down the eastern flank of a 3-kilometer-long hill, oriented northwest-to-southeast. The peak elevation of the hill (about 419 mASL) is located immediately southwest of the property and the hill is fringed by the Eramosa River valley to the northeast, east and southeast at elevations below 380 mASL. The southerly-flowing Eramosa River, located about 1 to 1.5 km to the northeast and southeast, is the primary drainage feature in the area and is the ultimate receiver of surface water runoff from the property.

Site topography is gently rolling (Figures 5 and 6) and the highest site elevation is about 418 mASL along the southwest property boundary. From the highest point, the majority of the property slopes steadily eastward to a low of about 408 mASL in the easternmost corner. A small portion of the property, at the back of Lots 5, 6 and 7 in the westernmost corner, slopes northward to an elevation of about 314.5 mASL at the boundary.

There are no permanent water courses on the property. As a result, drainage is expected to occur primarily as sheet runoff, following topography mostly to the east and with a small portion moving off to the north from the western corner. The eastward drainage eventually follows roadside ditches along Co. Rd. 24 to a small tributary of the Eramosa River where it crosses beneath the road (Figure 1). The northward drainage moves toward the same tributary, but in this case, where it starts about 250 m northwest of the property (Figure 1).

3.2 GEOLOGIC SETTING

3.2.1 Overburden

Surface geological mapping for the area (by Karrow, 1968) is presented in Figure 2. The hill on which the property is located is one of a series of elongate hills, oriented northwest to southeast across the area, that are together known as the Guelph Drumlin Field. The mapping indicates that these drumlins and much of the intervening lands are underlain by the Wentworth Till (Deposit 5, Figure 2). However, in a later geological publication on the Cambridge Area (1987), Karrow determined that the Wentworth Till actually ends about 4 kilometers to the southeast at the Paris Moraine and that the till across this particular area is actually the older Port Stanley Till, a sandy silt to silt till. The mapping also indicates that the Eramosa River valley is underlain by a peat, muck and marl swamp (Deposit 10, Figure 2) and fringed by surficial outwash gravel (Deposit 7, Figure 2).

The site borehole data (Appendix C) indicate the property is underlain by deposits that vary from the expected Port Stanley Till mapped by Karrow and which is present throughout the adjacent Thomasfield property (GM BluePlan, 2016). The site is mostly underlain by an inter-layered 'sand and silt' to 'sandy silt' deposit at five locations (BH1, BH2, BH3, BH4 and BH6), with occasional minor inter-layers of fine sand



(BH3) or sand and gravel (BH1). Sandy silt till was encountered at BH5 throughout the 7-m drilling depth and a substantial 4-m layer of sandy silt till was encountered at BH2 between layers of sand and silt. The grain size analyses of some of the sand/silt samples (e.g., BH2 at 1 m and BH3 at 1 m) show a grain size distribution pattern that is very similar to the sandy silt till on site (e.g., BH5 at 1 m). On this basis, it is interpreted that the sand/silt deposits at the site are localized outwash, derived from the predominant sandy silt till deposit, but which are not sufficiently extensive to have been reflected in Karrow's mapping.

Information on the deeper overburden deposits is available from local water well records. Table 2 lists the overburden thicknesses from each well record within 100 m of the property; the thicknesses ranging from 23 to 49 m (75 to 162 feet), owing in part to ground topographic differences. The vast majority of the driller's descriptions of the overburden materials in the area are considered to be low-permeability till or till-like deposits (e.g., clay, clay & stones, clay & boulders, clay & gravel, etc.). None of the well records in the inventory area indicate the presence of any deep granular deposits above bedrock.

3.2.2 Bedrock

The Paleozoic Geology mapping for the area (Figure 3) indicates the property is located near an approximate boundary between underlying bedrock formations; with the younger Guelph Formation tan thinly-bedded dolostone to the northwest of the property and the older underlying Amabel Formation massive cryptocrystalline grey dolostone (with upper Eramosa Member dark brown shaley dolostone) to the southeast. Most local well records confirm the uppermost bedrock to be a 'brown' dolostone (or limestone, as drillers refer to it) on the order of 3 to 20 m thickness, and an underlying grey dolostone. The brown dolostone could be either the Eramosa Member or a thin portion of the Guelph Formation. The thickness of the Amabel Formation in this area (including the Eramosa and a possible small layer of the Guelph) is on the order of about 90 m (300 feet) (Telford, 1973).

The elevation of the bedrock surface beneath the property is typically in the 370 to 380 mASL range, based on the well record data.

3.3 HYDROGEOLOGIC SETTING

The hydrogeological setting at the property has three primary components:

- The *shallow water table zone* within the surficial silt/sand deposit and the upper weathered portion of the sandy silt till deposit.
- The low-permeability till *aquitard* that separates the water table zone and the deep regional aquifer.
- The *deep regional aquifer* within in the dolostone bedrock formations.



3.3.1 Water Table Configuration & Shallow Groundwater Flow

Figures 6 and 7 present, respectively, the water level elevation data from May 11 and July 31, 2018 monitoring rounds and interpreted water table contours using these data. These two sets of contours reflect the spring 'high' and summer 'low' water table configurations.

In general, the water table configuration mimics topography and the pattern of contours is essentially the same during both the high and low water table conditions. Shallow groundwater is directed northward in the southwestern 'upgradient' part of the property across lots 3, 4 and 5. Flow then splits, mimicking topography; a small portion continuing across Lots 6 and 7 then northward from the site, and a larger portion bending eastward past lots 1, 2, and 8 to 13.

Shallow groundwater flow velocity is calculated using the Darcy Equation, as follows:

V = ki/n

where V = average linear groundwater flow velocity [m/s]
k = horizontal hydraulic conductivity of soil media [m/s]
i = horizontal hydraulic gradient [m/m]

n = effective porosity of the media [unitless]

The *horizontal* hydraulic conductivity values for the water table zone deposits, based on the five well response test analyses, fall within about one order of magnitude, from 1.5×10^{-7} to 1.5×10^{-6} m/s (Appendix E). These values are consistent with literature values from Freeze and Cherry (1979) for the upper end of glacial till and the lower end of silty sands. The *horizontal* hydraulic gradient (or water table slope) is calculated to be in the 0.025 to 0.035 m/m range, based on the water table contours in Figures 6 and 7. Using these values and assuming a typical effective porosity of 0.3, the *horizontal* groundwater flow velocity through the water table zone is calculated to be in the 0.4 to 5.5-m/yr range, which is quite modest and reflects the very high silt content. For an approximately 7-m thick upper water table zone, moving across the approximately 150-m property width (perpendicular to flow), the shallow groundwater flux (or flow volume) is calculated to be in the 125 to 1750 m³/yr range.

3.3.2 Shallow Groundwater Quality

The nitrate and chloride concentrations from the four shallow groundwater samples have respective ranges of 0.2 to 4.9 mg/L and 3.1 to 19.1 mg/L. The average nitrate concentration is 3.1 mg/L and this suggests there are some modest background nitrate sources from on-site and/or upgradient agricultural use and/or septic effluent.

3.3.3 Till Aquitard & Aquifer Protection

The well record data indicate there is a till aquitard beneath the site and surrounding lands with a substantial thickness of <u>at least</u> 28 m (<u>conservatively</u> based on a high bedrock elevation of 380 mASL and



a low ground elevation of 408 mASL) and ranging up to about 50 m. The *vertical* hydraulic conductivity of the un-weathered sandy silt till aquitard is expected to be no greater than about 1×10^{-8} m/s (or about one order of magnitude lower than the lowest horizontal hydraulic conductivity from the water table zone). The large thickness and low permeability of this aquitard combine to provide a substantial degree of protection from possible surface contaminants moving to the bedrock aquifer.

3.3.4 Groundwater Recharge

The pre-development rate of groundwater recharge at the property has been estimated using the water balance method of Thornthwaite and Mather (1957). This method utilizes precipitation and temperature data to provide monthly approximations of the two primary water balance components: evapotranspiration and water surplus. Surplus is the amount of runoff plus the percolation plus the change in soil moisture storage and is equivalent to the precipitation minus the evapotranspiration. This water balance method is cited in the MECP document 'Hydrogeological Technical Information Requirements For Land Development Applications' (April, 1995) for use in the evaluation of septic system impact. The latter document also provides a method, based on site soil, vegetative cover and slope conditions, for estimating the proportions of the surplus that ultimately becomes runoff vs. groundwater recharge.

Appendix G provides water balance calculations using the climate 'Normals' from the Waterloo Wellington weather station, a mix of cultivated land and grass vegetation and a sand/silt loam soil condition. Results indicate a water balance of about 0.37 m/yr. Using applicable infiltration factors for topography, soil type and vegetative cover, as recommended in the 1995 technical document, the proportion of surplus which becomes recharge (0.24 m/yr or 8,680 m³/yr) has been estimated for the site and this calculation is also summarized in Appendix G.

3.3.5 Bedrock Aquifer Groundwater Flow

Figure 4 presents September 17, 2015 bedrock aquifer potentiometric elevations for the three test wells drilled on the adjacent Thomasfield Development property (GM BluePlan, 2016). Based on these data, groundwater flow in the bedrock aquifer beneath the Thomasfield property is interpreted to be eastward (Figure 4), which is consistent with flow toward the regional discharge feature, the Eramosa River. The River and valley deposits are likely to be hydraulically connected to the bedrock aquifer. Based on the proximity and similar setting, it is interpreted that the deep groundwater flow beneath the subject property is similarly directed eastward (or perhaps slightly northeastward) toward the River.

3.3.6 Groundwater Use

Sixteen (16) of the twenty-three (23) wells shown in the 100-m well inventory area (Table 2 and Figure 4) are bedrock aquifer wells, including two of the bedrock aquifer wells drilled for the adjacent Thomasfield Development, and the remaining seven (7) wells are shallow dug wells.



The Guelph-Amabel bedrock aquifer is renowned in Southern Ontario as a very reliable water supply resource, supporting aquifer yields upwards of 250 gallons/minute for many municipalities in the area (e.g., Guelph, Fergus, Erin and Cambridge). This is why most private wells in the area are drilled to this aquifer and there is rarely any problem whatsoever in obtaining an adequate supply for an individual residence. Table 2 summarizes the test pumping rates (ranging from 7 to 90 gallons/minute and averaging 18 gpm) for the fifteen wells that were tested within the inventory area. The high end of this range and the average are considerably higher than typical test rates for domestic wells. The specific capacities of the wells in the area are also summarized in Table 2 and the average is approximately 2 gpm/ft, which is considered excellent for domestic wells and suggests very little drawdown occurs during typical pumping durations for domestic wells.

The Guelph-Amabel aquifer is also renowned for excellent natural water quality, albeit typically hard and occasionally containing elevated but treatable iron or manganese concentrations. The water sample results from the bedrock aquifer test wells on the adjacent Thomasfield development (TW1, TW2, TW3) confirm the excellent water quality (Appendix F, GM BluePlan Table 4), with very low salts (<2 mg/L chloride, <16 mg/L sodium, and <10 mg/L sulphate), negligible nitrate (<0.1 mg/L), elevated but treatable hardness (160-180 mg/L), and occasionally modestly elevated manganese (0.01 to 0.2 mg/L).

There are well records for two of the shallow dug wells (6706466 and 6706697) and both indicate the wells were dug (or bored) into an 'apparent' surficial sand deposit. One of the two was not subjected to a pumping test by the driller and the other was pumped at a low rate of 2 gpm. Discussions with several of the neighbouring residents indicated that most dug wells provide insufficient water, causing some owners to replace their dug wells with drilled wells for this reason (e.g. 6713028, 6704436 and NR4). It is also noted that well 6706697 is actually located on the development property (Figure 4) and is used by an adjacent residence through an agreement with a previous owner of the development property.

Based on the apparent limited yield and the susceptibility to surface contamination, it is concluded that the water table "aquifer" is a tenuous water supply resource.

4.0 SITE SERVICING REQUIREMENTS & IMPACT ASSESSMENT

4.1 WASTEWATER SYSTEMS & POTENTIAL IMPACT OF EFFLUENT

The design of the proposed wastewater treatment systems and leaching beds is addressed under separate cover (Wastewater Servicing Assessment, Flowspec Engineering, Feb 2019). The systems proposed will be OBC-approved tertiary systems capable of 30% nitrogen removal and the leaching beds would be located in the backyard of each lot.

Based on the setting described in Section 3.0 of this report, the following hydrogeological receptors are identified for evaluation regarding the potential impact from septic effluent:

- The shallow groundwater that discharges to surface water (Eramosa River and tributary),
- The shallow water table aquifer that is used for water supply purposes, and
- The deep bedrock aquifer that is used for water supply purposes.



General impact from septic effluent is evaluated in terms of nitrate loading and compared to the Ontario Drinking Water Standard of 10 mg/L. Table 4 summarizes the calculated nitrate loading for the property, using the <u>conservative</u> method recommended by MECP (MOE Technical Guideline For Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment, 1996). This calculation utilizes the following assumptions:

- An annual groundwater recharge rate of 0.24 m/yr, based on the pre-development water balance calculations summarized in Section 3.3.4, and assuming post-development stormwater management will maintain this recharge rate by using a rural open-ditch road profile and SWM pond, where recharge would be enhanced.
- 1000 L/day effluent flow per household (Guideline recommended value)
- 40 g/day effluent mass per household (Guideline recommended value) reduced to 28 g/day using OBC-approved tertiary wastewater treatment systems that are capable of 30% nitrogen reduction.
- Dilution from recharge only (i.e., conservatively assuming no denitrification and no groundwater crossflow)

The resultant conservative nitrate loading is calculated to be 9.9 mg/L, which is below the typical standard for comparison of 10 mg/L. Given the conservative nature of the calculation, this loading value is considered to be protective of the shallow groundwater in the context of potential impact to groundwater discharge to off-site surface water resources. It is also expected to be sufficiently protective of potential impact to the deep bedrock aquifer, which has a high degree of hydraulic isolation from the intervening thick till aquitard.

Five shallow supply wells (6706466, 6706697, NR1, NR2 and NR3; Figure 4) are located in the immediate downgradient flow path from some of the proposed septic systems. The average nitrate loading, even with the recommended 30% nitrate-reducing systems, is not considered to be sufficiently protective of these shallow supplies because of the potential that individual effluent plumes could move over a relatively short distance toward a specific well. It is therefore recommended that one of the following two options be implemented to address the risk:

- 1. Further improve sewage treatment to OBC 75% nitrate reducing systems and provide disinfection prior to discharge to the leaching bed to remove health-related bacteria.
- 2. Decommission the five dug wells and provide each of the three residences and the church with a new secure bedrock aquifer supply well.

While both options are feasible to address the potential for impact, Option 1 would require an on-going monitoring program to ensure the treatment systems continue to be effective and that the dug well quality has not been adversely affected. Option 2 is preferred since it would not require a monitoring program and the owners would have much better and safer water supplies.

4.2 WATER SUPPLY & POTENTIAL IMPACT OF WATER TAKING

Individual bedrock aquifer supply wells are recommended for each of the thirteen development lots. It is recommended the wells be located in the front yards of each lot (Figure 8), leaving the larger area in



the backyards for leaching beds and amenities.

As described in Section 3.3.6, establishing adequate domestic supplies from this aquifer is not expected to be a problem whatsoever, given the numerous successful local bedrock wells with relatively high yields and specific capacities.

To evaluate the potential for mutual interference between supply wells, short-term and long-term aquifer drawdown at a representative well near the centre of the property (Lot 9) has been modelled using the Cooper and Jacob modified non-equilibrium equation:

- s = 0.183 Q/T log (2.25 Tt / (r2S))
- where: s = the water level drawdown, in m, at a specified distance from a pumping well Q = the constant pumping rate, in m³/day, from a pumping well
 - T = the aquifer transmissivity, in m²/day
 - S = the aquifer storativity (dimensionless)
 - r = the radial distance, in m, from a pumping well to the specified location
 - t = the time since pumping started

Representative values of T $(1.0x10^{-3} \text{ m}^2/\text{s})$ and S $(1.0x10^{-4} \text{ unitless})$ used in the calculations were from the 7.6-hour pumping tests of wells TW1 and TW2 located on the adjacent Thomasfield Development (see GM BluePlan Table 6, Appendix E). GM BluePlan 'monitoring well' analyses values were used (i.e., TW1 drawdown from pumping TW2, and vice versa) since these are reflective of a large portion of the aquifer based on the 225-m distance between the two wells.

The model results are summarized in Table 3. The cumulative drawdown at the representative well (Lot 9) from simultaneous pumping of all thirteen development wells, plus the four new wells recommended for neighbouring lots (Figure 8), has been calculated for the following scenarios:

- 1-day, 1-year and 5-year pumping durations at a conservatively high 'average-day' pumping rate of 1.5 m³/day (about 1 L/min).
- 2-hour pumping duration at a conservatively high 'peak-demand' pumping rate of 18 m³/day (about 12.5 L/min), to reflect an extreme worst-case scenario where all wells obtain the daily demand volume (1.5 m³) in a much shorter 2-hour period.

Not surprisingly, based on the high aquifer transmissivity and storativity values, the cumulative drawdown at the representative well (Lot 9) is very modest, both in the short-term peak-demand scenario (1.16 m in 2 hours) and long-term average-day scenario (0.33 m after 5 years). Based on this analysis, it is concluded that the additional seventeen wells will not cause significant aquifer drawdown, nor will there be interference with existing neighbouring wells and new development wells.

In respect to water quality, the data from the neighbouring test wells on the adjacent Thomasfield development (Appendix F, GM BluePlan Table 4), as described in Section 3.3.6, confirm the excellent water quality with very low salts, negligible nitrate, and elevated but treatable hardness and occasionally manganese.



4.3 POTENTIAL IMPACT TO GROUNDWATER RECHARGE

Based on the surficial silty/sandy setting and the positive recharge conditions at the property, as described in Section 3.0 of this report, along with the rural road cross-section, it is expected that the predevelopment groundwater recharge will be maintained during post-development due to enhanced recharge along the roadside ditches and SWM pond. In addition, the majority of the water taking from the deep aquifer will be recharged (via the leaching beds at 4745 m³/yr) into the shallow groundwater flow system. As a result, a large increase in recharge to the shallow water table is expected at the property.

4.4 ASSESSMENT OF WATER TABLE & GRADING DESIGN

The grading design for the development is provided in the functional servicing report by IBI Group (2019). The IBI grading plan is the base map in Figure 5 and it includes finished elevations along the roadside ditches, in the SWM pond, and across the development lots (e.g., along sideyard swales, in the front yards and at the central building footprints). This grading plan has been reviewed and, in all cases, the seasonally 'high' water table (i.e., Figure 6) is located below all proposed surface grades, including the roadside ditches and SWM Pond. In addition, the central building footprint grades have been set to provide at least a 3.3 m separation distance from the 'high' water table. Although specific building types, precise locations and main floor grades have not been set at this point, the proposed soil grades will allow basement floor separation distances of at least 0.5 m from the 'high' water table to be achieved. The backyard leaching beds, which will partially raised, will be easily kept more than the required 1.0 m from the 'high' water table.

5.0 CONCLUSIONS & RECOMMENDATIONS

Based on the results of the hydrogeological investigation described in this report, the following conclusions and recommendations are provided.

- 1. The subject property is underlain by moderate-permeability sand/silt and sand/silt till deposits and with a shallow water table varying in depth from about 0.6 to 3.2 m depending on location and season. Shallow groundwater flow beneath the property mimics topography, moving eastward and northward across and away from the property. The soils support positive recharge conditions and support construction of backyard leaching beds for the proposed individual on-site tertiary wastewater treatment systems.
- 2. A 28 to 50-m thick low-permeability till aquitard extends beneath the property, overlying a permeable dolostone bedrock aquifer that is renowned for its water supply capability. The thickness and low permeability of the aquitard combine to provide substantial hydraulic separation between the water table receiving sewage effluent and the bedrock aquifer; and thus substantial water quality protection is afforded to the aquifer.
- 3. Data from local well records and pumping tests of wells located on an adjacent development property confirm the viability of the dolostone bedrock to support individual bedrock supply



wells for the development, without concern for interference with existing neighbouring wells or the future wells on the development property. The wells are recommended for the front yards. Treatment for hardness and manganese (if present) is a viable option to future owners, if desired.

- 4. Tertiary wastewater treatment systems, designed in accordance with the Ontario Building Code and capable of a minimum of 30% nitrate removal, are recommended for the development lots. These will provide an improved level of treatment, for nitrate and other parameters, compared to conventional systems and provide good general protection of shallow groundwater and surface water resources (i.e., by maintaining average nitrate loading below 10 mg/L).
- 5. There are three residences and a church located directly downgradient from the development that are currently serviced by shallow wells. These supplies are already expected to be tenuous based on the modest yields available from the water table zone and the susceptibility of shallow groundwater to contamination (e.g., from septic effluent and agriculture). Two options have been identified to address the increased risk for contamination of these wells from the additional effluent loading at the development property. Option 1 would consist of increasing the level of nitrate removal to 75% in the proposed treatment systems and including bacterial disinfection. This option would require an on-going monitoring program to ensure its effectiveness. Option 2 would decommission the dug wells and provide the three residences and church with new individual bedrock aquifer supply wells. Both options are technically viable, however, Option 2 is preferred since it would not require a monitoring program and the neighbouring owners would have much better and safer water supplies.
- 6. To maintain (or enhance) groundwater recharge during post development, it is recommended that grassed swales/ditches be utilized along roadways and lot lines to promote infiltration of runoff. This recommendation has been incorporated into the site design.
- 7. The site grading plan design provides adequate separation distance from the seasonally high water table and the future buildings, leaching beds, roadside ditches and the SWM pond.

Respectfully submitted, CHUNG & VANDER DOELEN ENGINEERING LTD.



William (Sandy) Anderson, M.Sc., P.Eng. Senior Hydrogeologist and Engineer

Lauren Curnow, B.Sc. Environmental Scientist



6.0 REFERENCES

The following documents, maps, or other publications have been used in the preparation of this report.

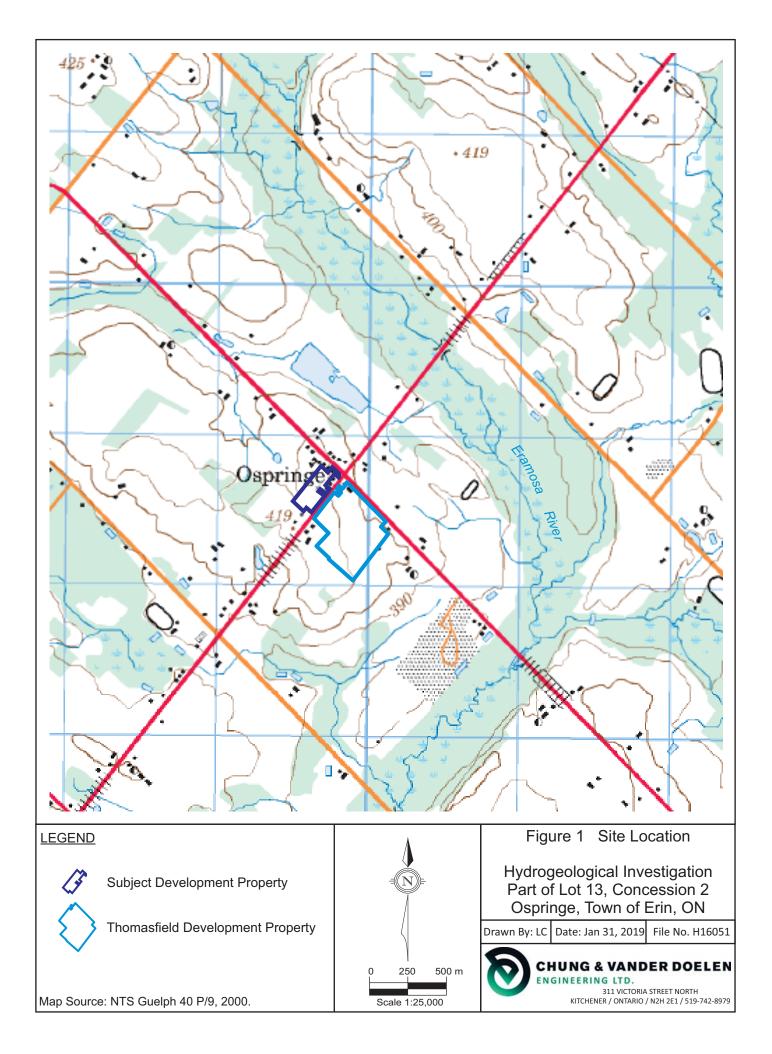
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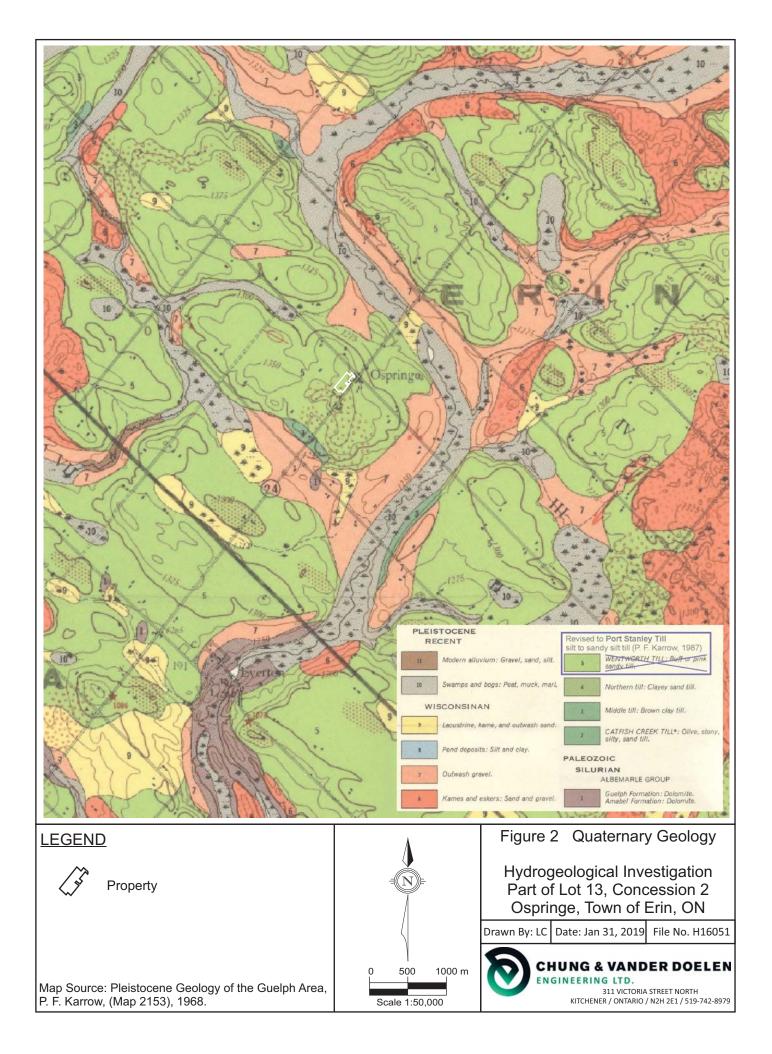


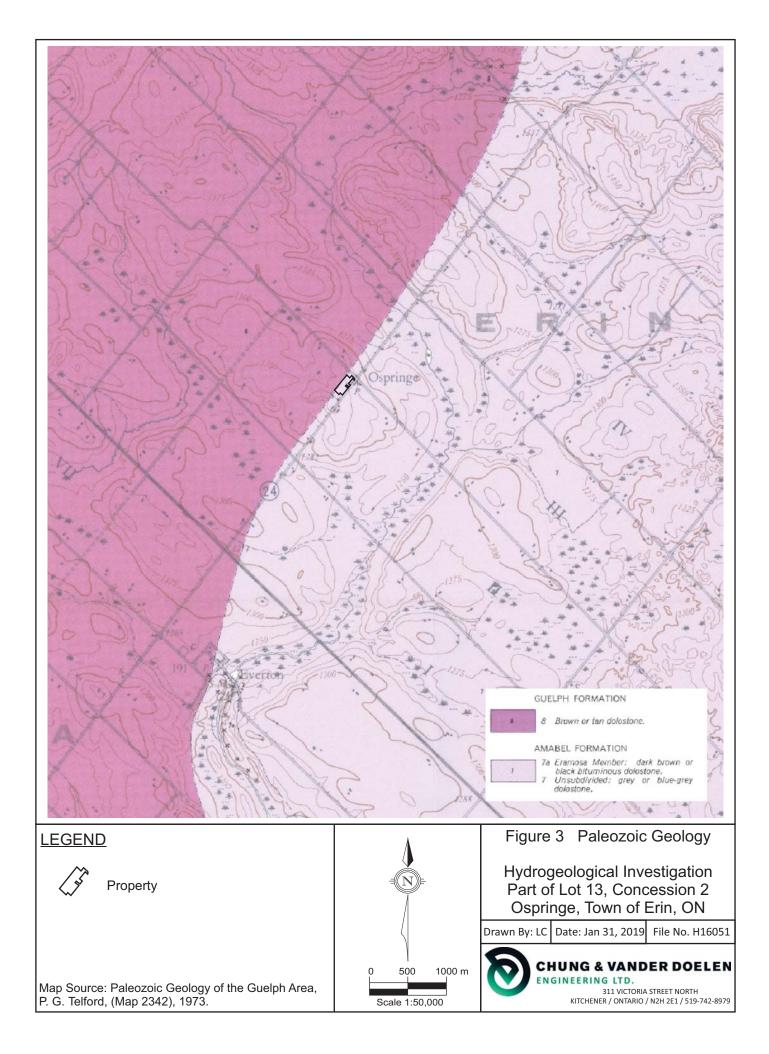
Hydrogeological Investigation 13-Lot Ospringe Subdivision Part Lot 13, Conc 2, Town of Erin February 27, 2019 FILE NO.: H16051 Page A

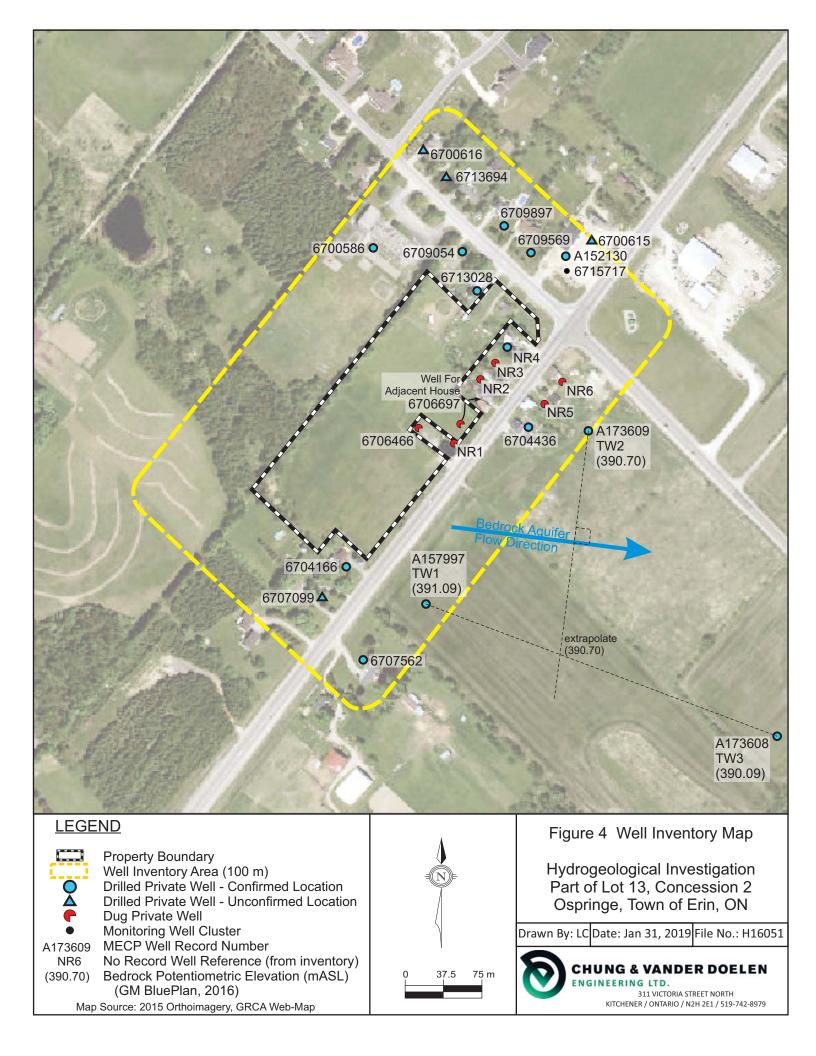
APPENDIX A Figures 1 to 8

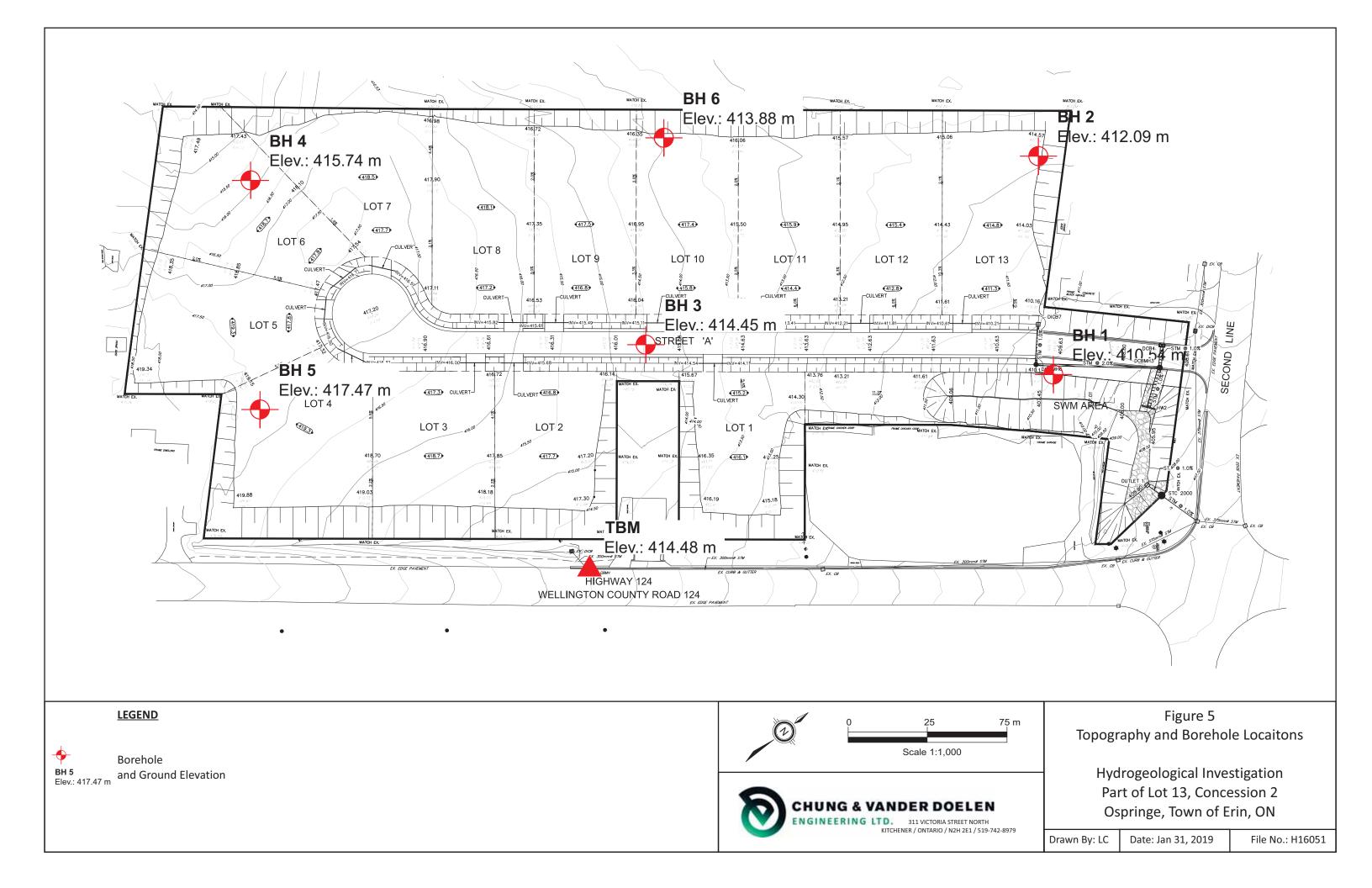


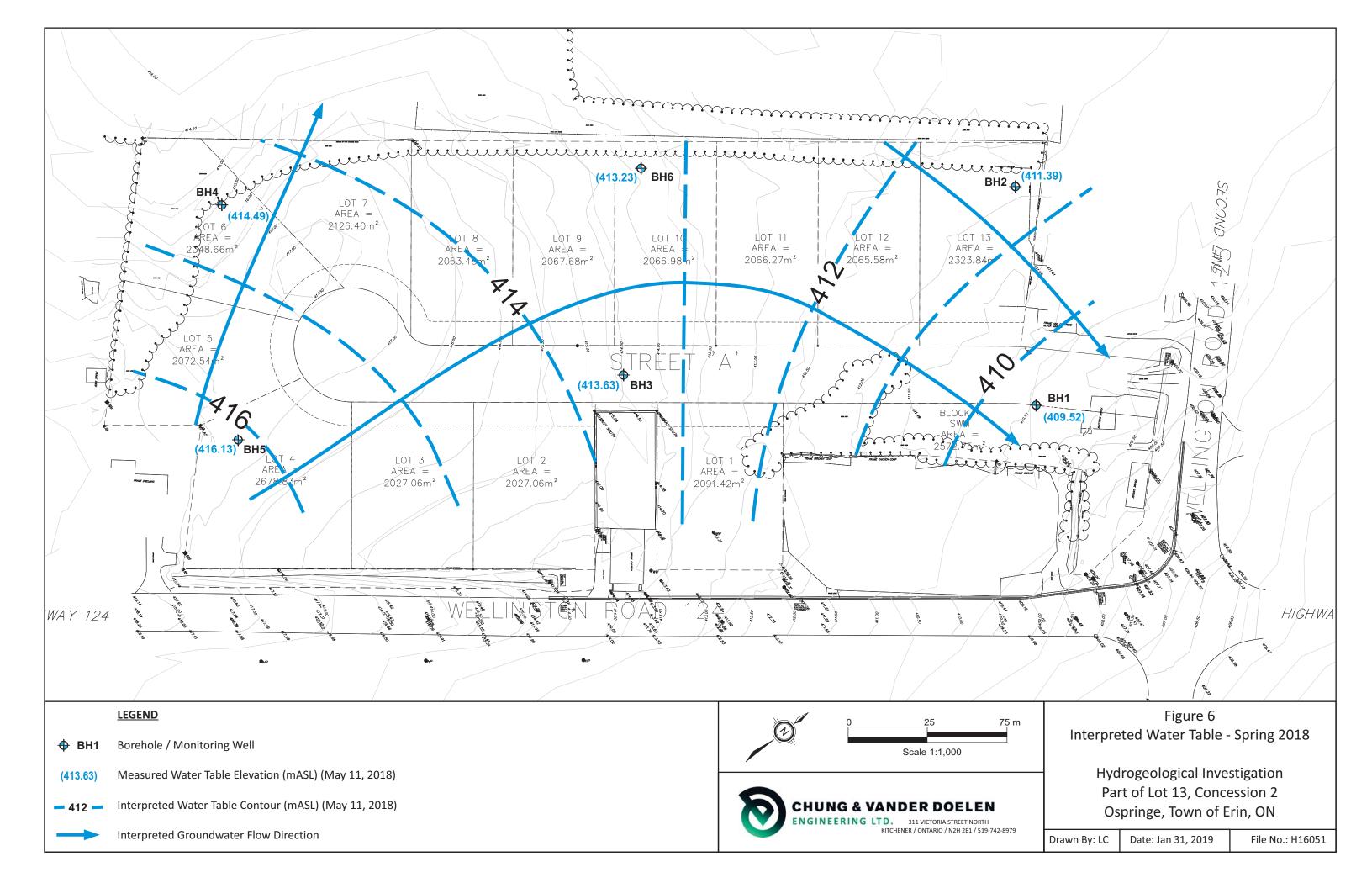


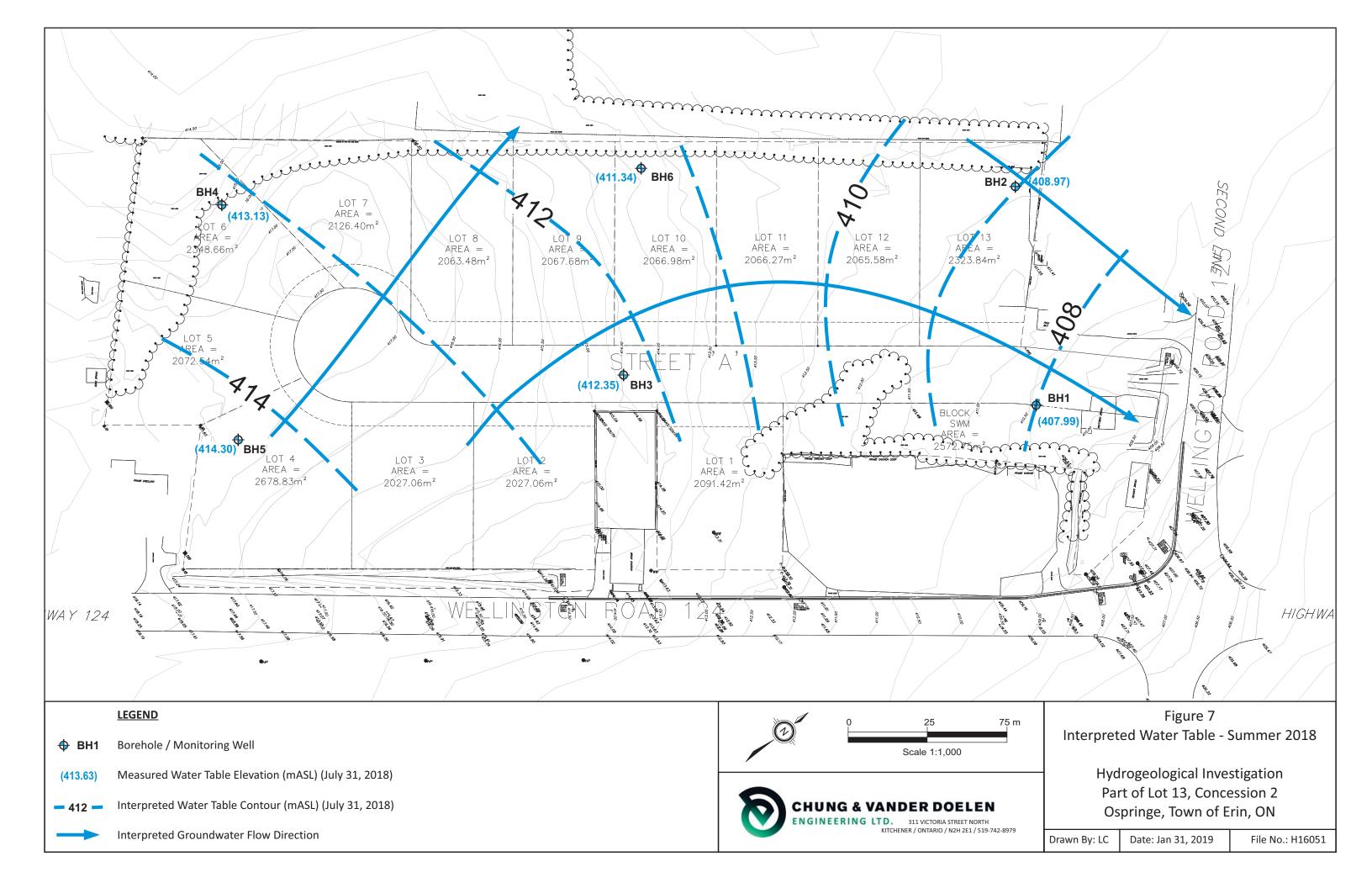


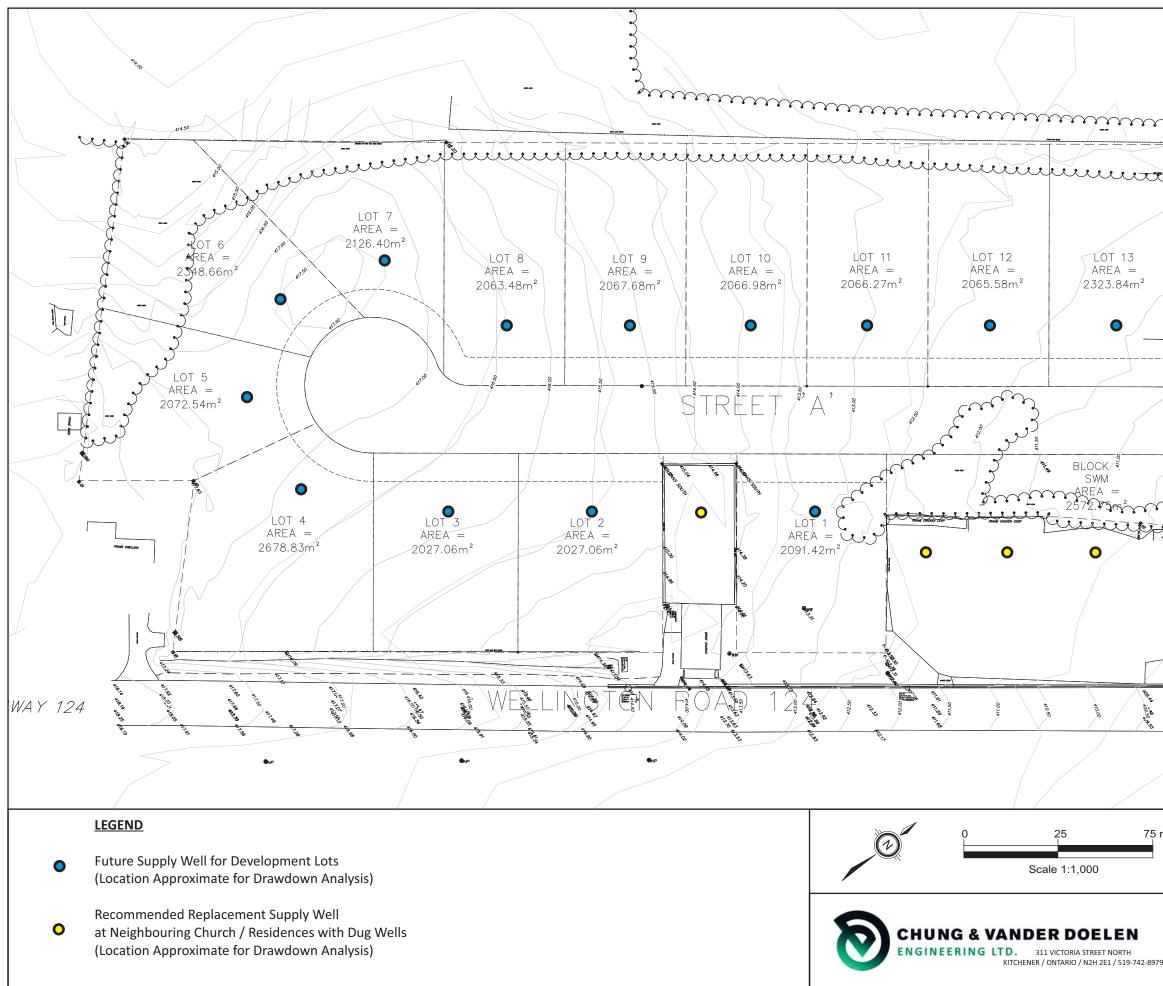












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- Contraction of the second se		
100 K00 800 800 800 800 800 800 800 800 8		HIGHWA
5 m	Figure 8 Future Supply Well Loc for Drawdown Anal	
	Hydrogeological Invest Part of Lot 13, Conces Ospringe, Town of Erig	sion 2
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Hydrogeological Investigation 13-Lot Ospringe Subdivision Part Lot 13, Conc 2, Town of Erin February 27, 2019 FILE NO.: H16051 Page B

APPENDIX B Table 1 to 4



Table 1 Summary of Monitoring Well Water Levels & Elevations

	Ground	TOP	Water Level		Water Level Elevation		Fluctuation (m)
	Elevation	Elevation	(m Below Ground)		(mA	SL)	11-May-18
Well	(m ASL)	(m ASL)	11-May-18	31-Jul-18	11-May-18	31-Jul-18	31-Jul-18
BH1	410.54	411.28	1.02	2.55	409.52	407.99	-1.53
BH2	412.09	412.84	0.70	3.12	411.39	408.97	-2.42
BH3	414.45	415.14	0.82	2.10	413.63	412.35	-1.28
BH4	415.74	416.40	1.25	2.61	414.49	413.13	-1.36
BH5	417.47	418.20	1.35	3.18	416.13	414.30	-1.83
BH6	413.88	414.56	0.64	2.54	413.23	411.34	-1.90

Notes: 1) TOP - Top of Pipe

Table 2 Well Inventory Summary

H16051 Ospringe Development

Well ID	Well Type	Total Depth (ft)	Depth to Bedrock (ft)	Static Water Level (ft)	Pumping Test Duration	Pumping Rate (gpm)	Drawdown (ft)	Specific Capacity (gpm/ft)
6700586	Drilled	270	117	87	2	20	8	2.50
6700615	Drilled	170	75	36	2	7	44	0.16
6700616	Drilled	163	98	52	11	10	8	1.25
6704166	Drilled	141	138	80	3.5	10	1	10.00
6704436	Drilled	145	116	51	2.5	10	14	0.71
6707099	Drilled	191.5	162	91	2.5	90	99	0.91
6707562	Drilled	147	142	80	12	10	3	3.33
6709054	Drilled	186	99	78	1.5	12	9	1.33
6709569	Drilled	185	95	45	1.5	10	30	0.33
6709897	Drilled	220	93	53	1.5	9	27	0.33
6713028	Drilled	143	100	65	1	12	55	0.22
6713694	Drilled	142	97	65	1	20	16	1.25
A152130	Drilled	197	87	53	1	13	17	0.76
NR4	Drilled	-	-	-	-	-	-	-
A157997	Drilled Test Well 1	142	124	83	7.6	20	3.7	5.41
A173609	Drilled Test Well 2	200	94	53	7.6	20	41.7	0.48
A173608	Drilled Test Well 3	162	106	48	5.5	20	26.9	0.74
7224420	Abandonment	-	-	-	-	-	-	-
6715717	Monitoring well	17	-	7	-	-	-	-
6706466	Dug water supply	32	-	21	-	-	-	-
6706697	Dug water supply	29	-	6	1	2	23	0.09
NR3	Dug water supply	-	-	-	-	-	-	-
NR2	Dug water supply	-	-	-	-	-	-	-
NR5	Dug water supply	-	-	-	-	-	-	-
NR6	Dug water supply	-	-	-	-	-	-	-
NR1	Dug water supply	-	-	-	-	-	-	-

Note: Test Data and Water Levels for Test Wells 1, 2 and 3 from GM BluePlan report (2016), not from well records

Table 3 Drawdown Calculations at Lot 9 Well

Transmissivity =	(m²/s)	1.0E-03	1.0E-03	1.0E-03	1.0E-03
Transmissivity –	(m²/day)	86.4	86.4	86.4	86.4
Storativity =	-	1.0E-04	1.0E-04	1.0E-04	1.0E-04
Discharge Rate =	(m³/day)	18	1.5	1.5	1.5
Instantaneous Rate =	(L/min)	12.5	1.04	1.04	1.04
Pumping Duration =	(days)	0.083	1	365	1825
		2 hour	1 day	1 year	5 year
	Distance	Drawdown	Drawdown	Drawdown	Drawdown
	to	at Peek	at Average	at Average	at Average
	Lot 9 Well	Demand Rate *	Daily Rate	Daily Rate	Daily Rate
Lot Well	(m)	(m)	(m)	(m)	(m)
1	70	0.058	0.008	0.016	0.019
2	50	0.069	0.009	0.017	0.020
3	69	0.058	0.008	0.016	0.019
4	97	0.047	0.007	0.015	0.018
5	103	0.045	0.007	0.015	0.018
6	93	0.048	0.007	0.016	0.018
7	67	0.059	0.008	0.017	0.019
8	32	0.084	0.010	0.019	0.021
9	0.1	0.275	0.026	0.034	0.037
10	32	0.084	0.010	0.019	0.021
11	64	0.061	0.009	0.017	0.019
12	96	0.047	0.007	0.016	0.018
13	128	0.038	0.007	0.015	0.017
new neighbour 1	53	0.067	0.009	0.017	0.019
new neighbour 2	99	0.046	0.007	0.015	0.018
new neighbour 3	117	0.041	0.007	0.015	0.017
new neighbour 4	137	0.036	0.006	0.015	0.017
Total Drawdown at Lot 9 f	rom All Wells	1.16	0.16	0.29	0.33

NOTES:

1) Calculated drawdown by Jacob Non-Equilibrium Equation does <u>not</u> include recharge from:

a) precipitation, b) septic system infiltration

2) Transmissivity & Storativity are based on Pumping Test Values From Tests at Wells TW1 and TW2 (Thomasfield Development)

3) For Peek Demand - assume entire daily flow of 1.5 m³ occurs in 2 hours

4) All well locations are assumed to be at the midpoint of each lot frontage (See Figure 7)

Table 4 Nitrate Loading Calculation

Basic Assumptions:

1000 L/day effluent flow per household

Post-development recharge rate to sand/silt soils with an open ditch rural cross-section roadway and SWM pond = pre-development water balance rate of 0.24 m/yr

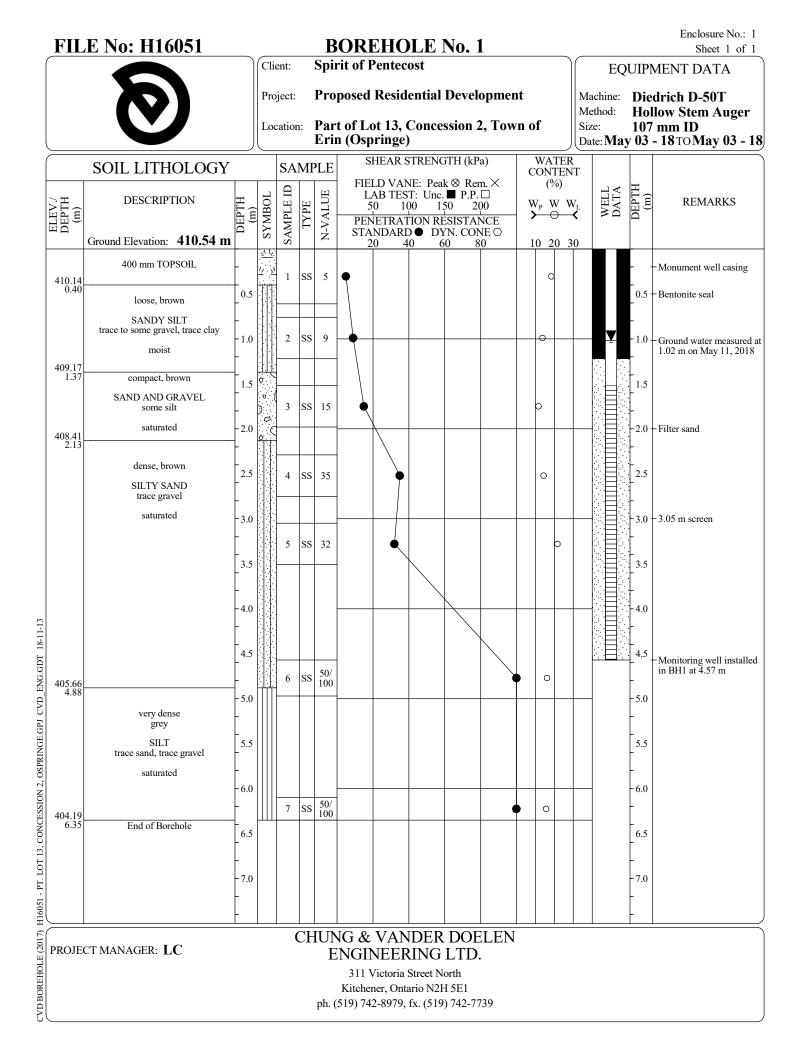
nitrate mass in effluent assumes tertiary treatment system capable of 30% nitrogen reduction (e.g., 40 g/day x 0.7) no groundwater crossflow, no enhanced recharge, no in-situ denitrification

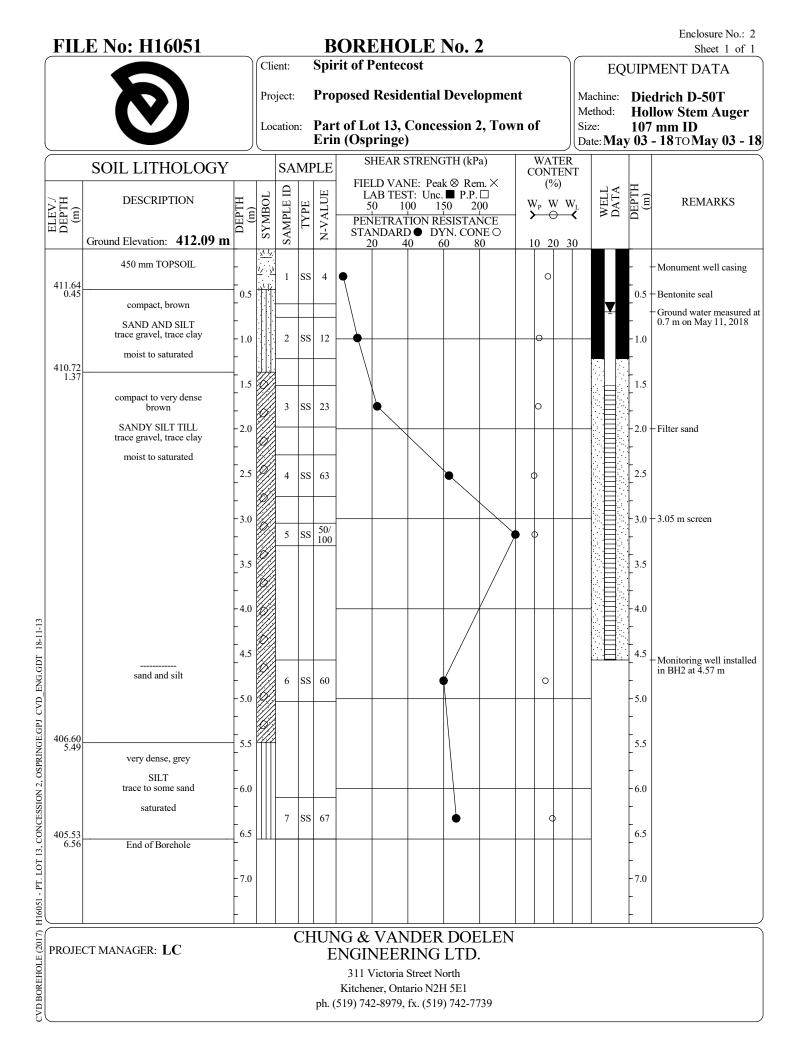
Calculation Scenario:	Tertiary 30% N Reduction
Number Houses Effluent Volume per House (L/day) Nitrate Mass in Effluent per House (g/day) Recharge Area (m ²) Recharge Rate (m/yr)	13 1,000 28 36,200 0.24
Total Mass Nitrate (g/yr)	132,860
Volume Effluent (m ³ /yr) Volume Recharge (m ³ /yr)	4,745 8,688
Total Volume Water (m³/yr)	13,433
Resultant Nitrate Loading (g/m ³ or mg/L)	9.9

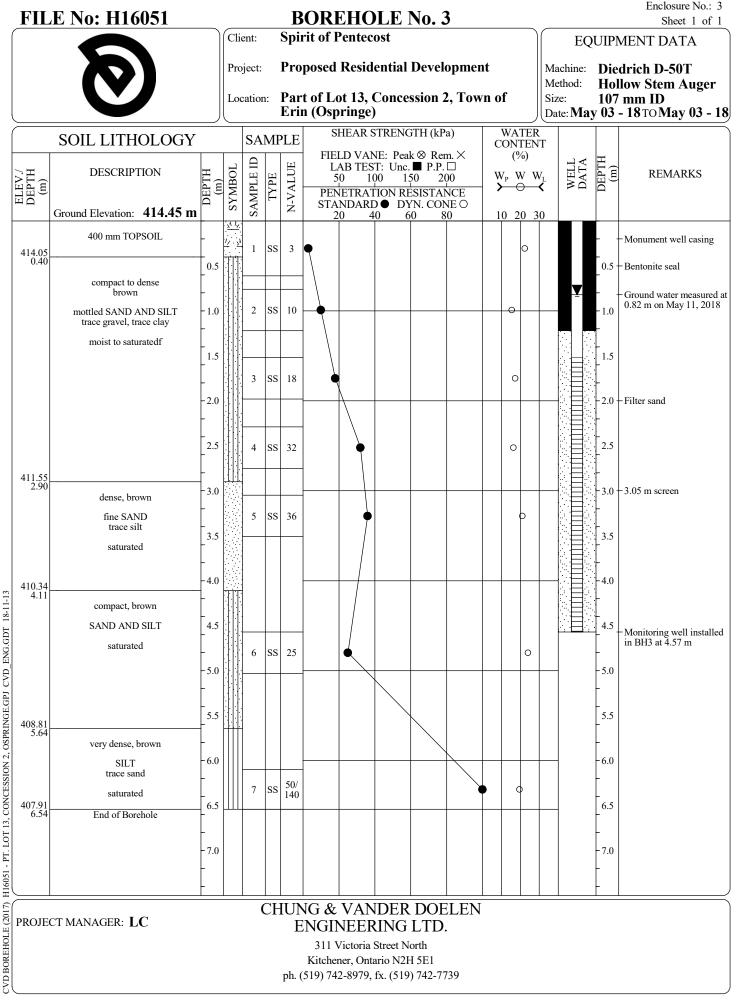
February 27, 2019 FILE NO.: H16051 Page C

APPENDIX C Borehole Logs & Grain Size Analyses

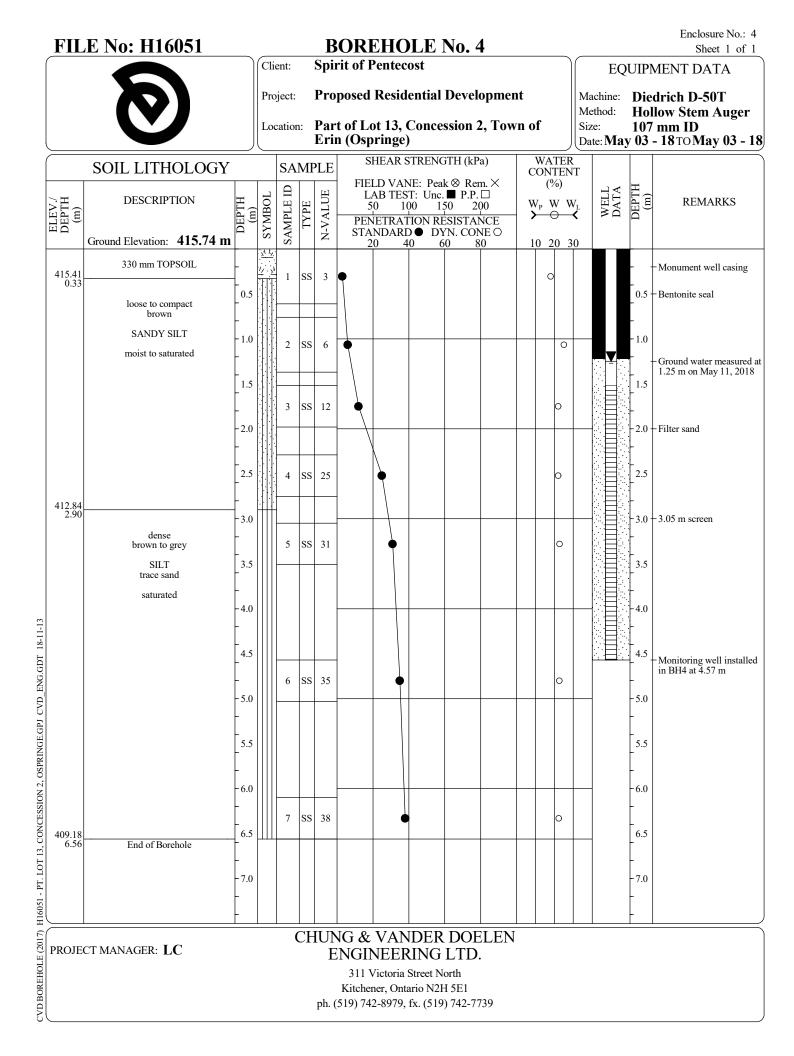


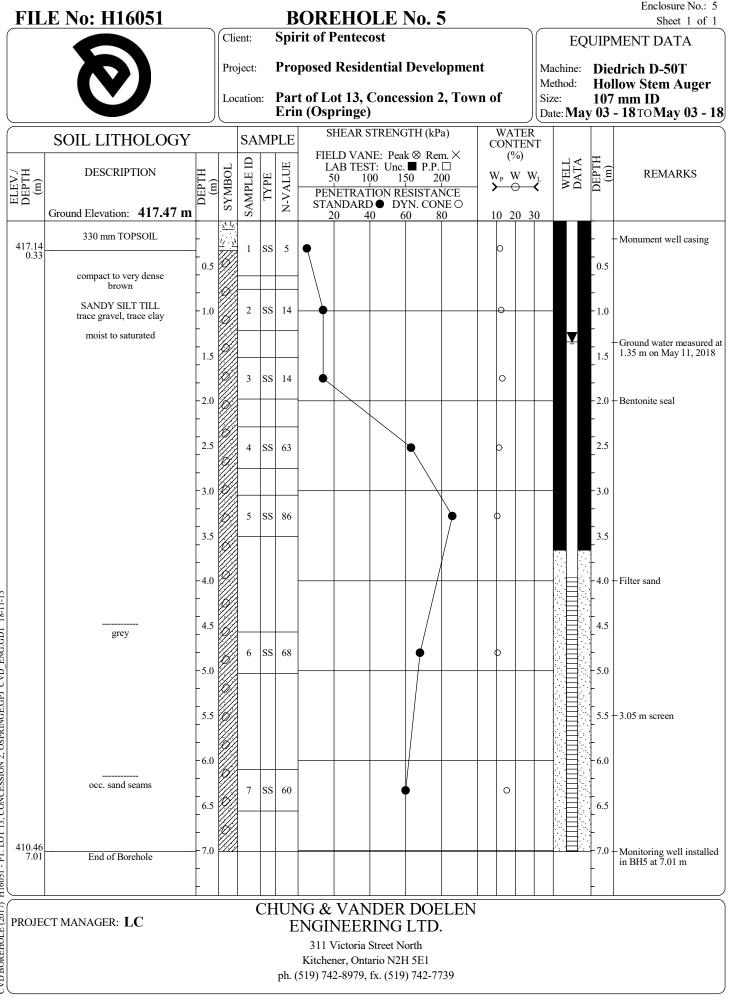




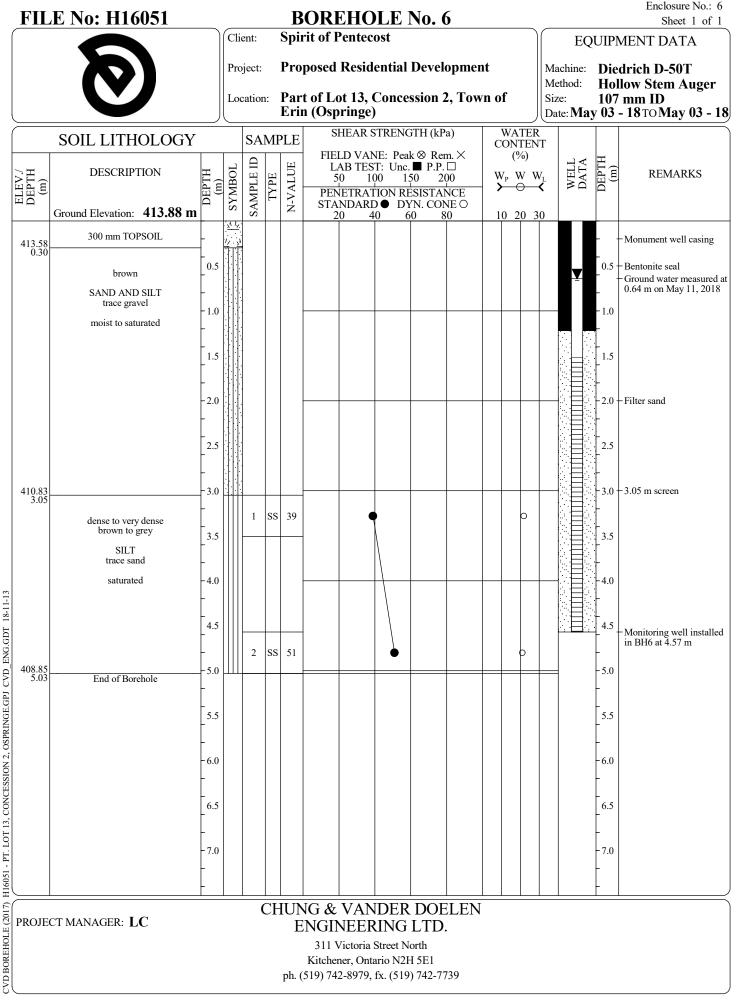


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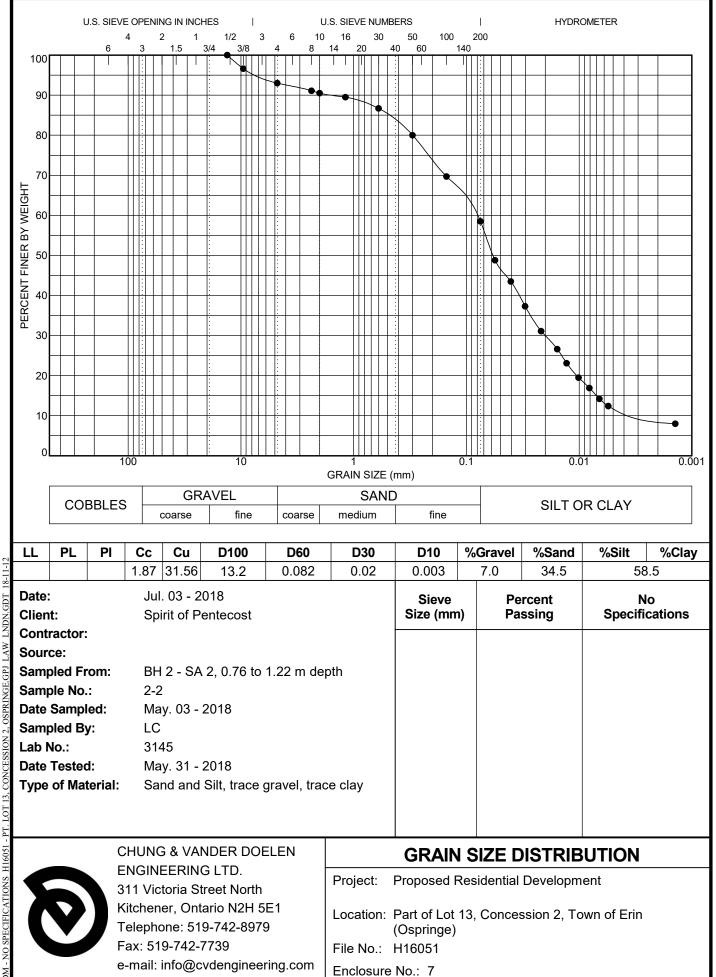


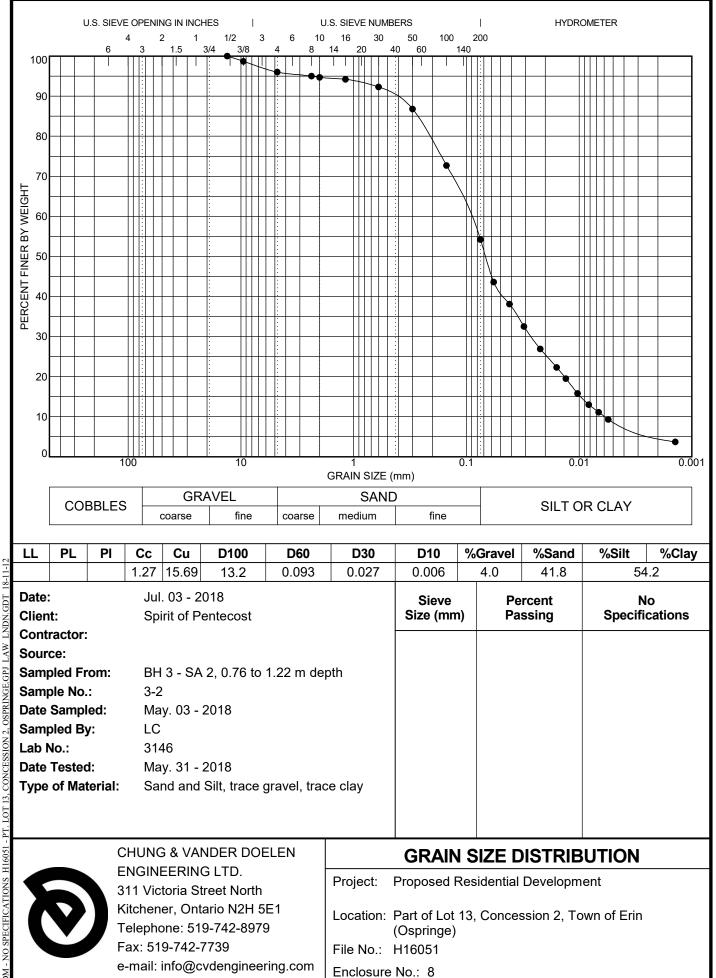


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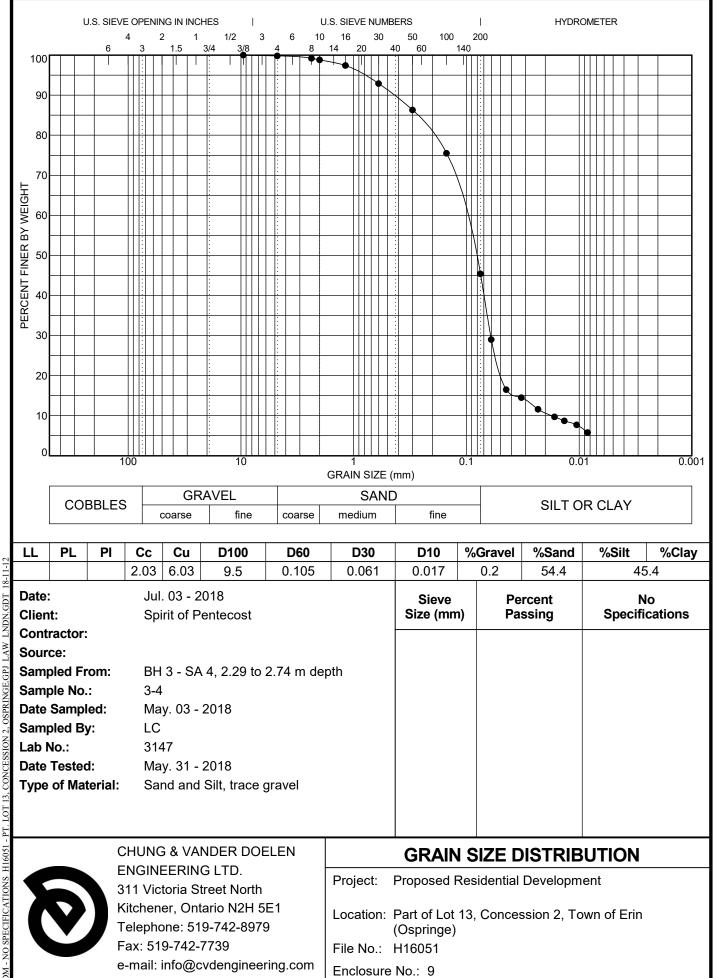


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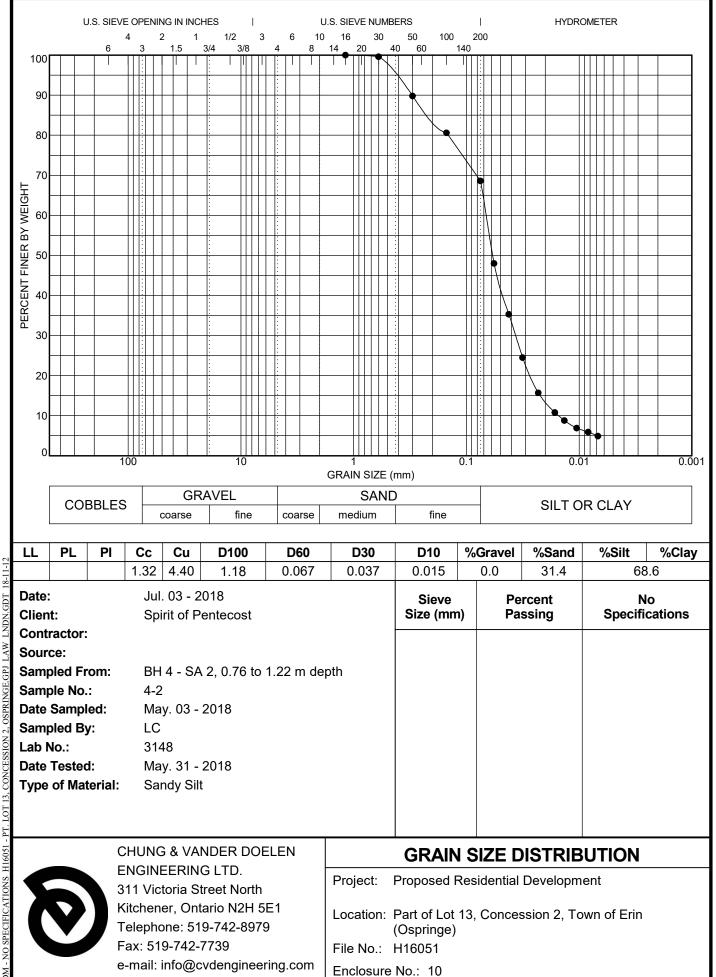




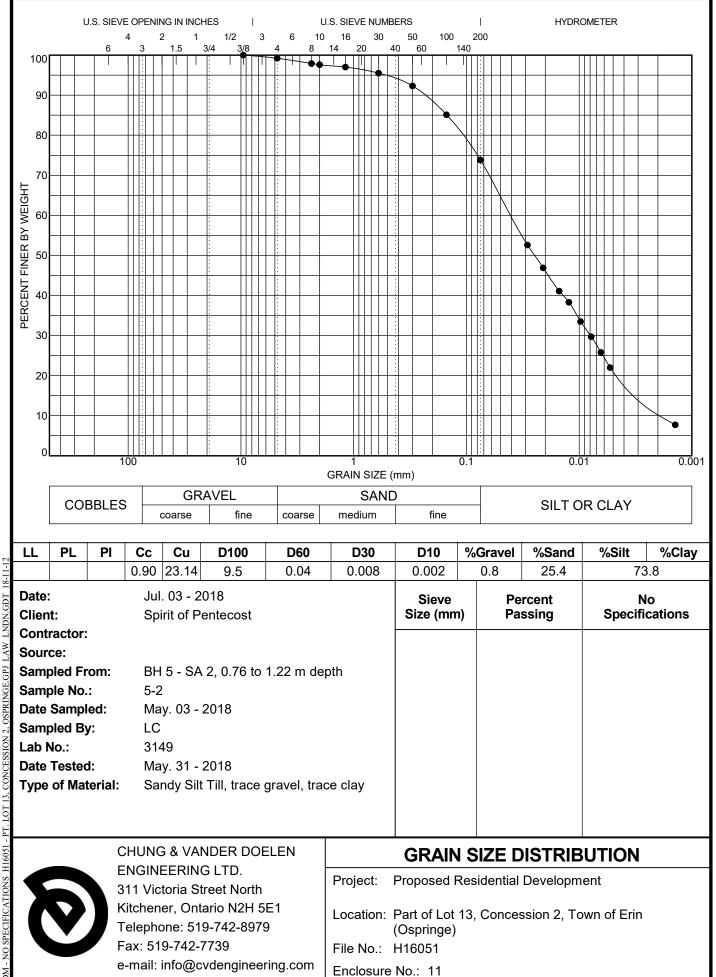
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Hydrogeological Investigation 13-Lot Ospringe Subdivision Part Lot 13, Conc 2, Town of Erin February 27, 2019 FILE NO.: H16051 Page D

APPENDIX D Water Well Records



WATER RESOURCES E 586 JUL 281964 Ontario Water Resources Commission Act Elev WFII RECORD ONTARIO WATER ATER RESOURCES COMMISS Basin LINGTON County Lot 14 Date completed 261764 Con year) Owner ERIN TSOP SCHOCK BOARD Address OS PRINCEL VT (print in block letters) **Pumping Test Casing and Screen Record** 87 f. 11 5 Static level Inside diameter of casing 120 ET 20 Test-pumping rate G.P.M. Total length of casing.... Pumping level 95 ET Type of screen Duration of test pumping 2 kis Length of screen Water clear or cloudy at end of test $C \land B \land R$ Depth to top of screen ... Recommended pumping rate 2C G.P.M. Diameter of finished hole with pump setting of 132 feet below ground surface Water Record Well Log Kind of water Depth(s) at From ft. То which water(s) (fresh, salty, sulphur) Overburden and Bedrock Record ft. found 20 0 CLAY - Rocks BROW N SAND 32 20 BEY CLAY Rocks 32 112 the second second 160 117 ROWN - LINESTONE 200 160 GREY-LIMESTONE DARK 250 200 GREY - LIMESTONE LIGHT 250 270 WHITE LIMESTONE For what purpose(s) is the water to be used? Location of Well In diagram below show distances of well from Public - Schook road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? Drilling or Boring Firm LADCO PRILLING Hillsburge - R. R.#1 Address Licence Number 801 250 Name of Driller or Borer THOMAS LANG Address H, 11sb are R. R. #1 July 15164 OSPRINGÉ (Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138 CSS.S8 OWRC COPY

1" Nº. 67 E 6 UTM Ontario Water Resources Commission Act |5|R RECORD 25 Elev. R Grun" Basin | County ingl Date completed 9 July 1966 Lot Cor prin lress..... **Pumping Test Casing and Screen Record** 36 4 Static level Inside diameter of casing \mathcal{H}G.P.M. Test-pumping rate Pumping level Length of screen NONE Duration of test pumping Clear Water clear or cloudy at end of test..... Depth to top of screen 4" G.P.M. Recommended pumping rate Diameter of finished hole with pump setting of $\beta 0$ feet below ground surface Water Record Well Log Depth(s) at Kind of water (fresh, salty, sulphur) То From which water(s) found Overburden and Bedrock Record ft. ft. 75 0 an 120 100 170 10-0 imest aun Location of Well For what purpose(s) is the water to be used? In diagram below show distances of well from Domestic road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? adco Drilling or Boring Firm q Address. Licence Number.... Name of Drille OSPRANCE Address Date. (Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138 CSS 58 OWRC COPY

			WAGET RESNO	CES 616 2
5 R The Ontario Water Reso	urces Commission	Act	TAR 1 19	
Elev. 6 R 1350 WATER WEL	L RECO	ORD	••••••••••••••••••••••••••••••••••••••	X
Basin 23 County or District Werlington		own or City	ONTARIO WATE Esqur ces commi	SSON
Con 3 part of Lot 14	Jate completed	th Fe	bruary	1967 year)
		3 Acton,(1	
Casing and Screen Record		Pumping	; Test	
Inside diameter of casing 4 inch	static iever	2 ft		
Total length of casing 115 ft	Test-pumping ra			G.P.M.
Type of screen nil	Pumping level	60 ft		
Length of screen nil	Duration of test I	oumping 11	hours	
	Water clear or cle	oudv at end of	_{test} clear	
Depth to top of billion	Recommended p	umning rate	10	G.P.M.
Diameter of finished hole 4 inch	with pump settir	and 95	feet below	
	with pump setur			Record
Well Log			Depth(s) at	Kind of water
Overburden and Bedrock Record	From ft.	To ft.	which water(s) found	(fresh, salty, sulphur)
brown clay & stones	0	28	155	fresh
brown clay & gravel	28	35		
brown bolders & clay	<u> </u>	<u>85</u> 98		
brown clay and gravel	98	140		
light brown rock	140	140		
dark brown rock		±00		
Total depth - 163 ft.				
well pit 7 ft.				
For what purpose(s) is the water to be used?		Location		
domestic			distances of wel licat ewno rth by	
Is well on upland, in valley, or on hillside?	Toau and	fot fine. The	AL	
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Sheridan Street	Hwy	24		
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Date February 25th 1967	.			
J L Graham - per MEL (Signature of Licensed Drilling or Boring Contractor)		Te ,		
Form 7 15M-60-4138		34		
OWRC COPY			U	\$5.88

ţ.

COUNTY OR DISTRICT		CES PROVIDED 11 BOX WHERE APPLICABLE 12	3	CON., BLOCK, TRACT, SURVEY,	ETC LOT 25
	INIG TORI	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		2	
		RR#1 GU	FLPH		DAY 14 MO. DEF YR.
		838260 K	ELEVATION 13174	RC. BASIN CODE	
12		G OF OVERBURDEN AND BEDRO	OCK MATERIALS	30 31	
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS		GENERAL DESCRIPTION	DEPTH - FEET FROM TO
	Chrite *	- STONES - S.	AND HI	TYERS	0 /31
	LIMESTON	15			138 14
<u>,</u>					
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32 .				54	65 75 -33 DIAMETER 34-38 LENGTH
41 WA	TER RECORD	51 CASING & OPEN HOL		Z (SLOT NO.)	INCHES
WATER FOUND AT - FEET		DIAM. MATERIAL THICKNESS INCHES F	ROM TO	MATERIAL AND TYPE	DEPTH TO TOP 41-4 OF SCREEN
P141 -	CRESH 3 SULPHUR SALTY 4 MINERAL	10-11 1 STEEL 12 2 GALVANIZED 12 05) //0	v	FEET
	I FRESH 3 SULPHUR ¹⁹ 2 SALTY 4 MINERAL	4 044 3 □ CONCRETE 4 □ OPEN HOLE	0140	DEDTU CET AT - FEET	ERIAL AND TYPE (CEMENT GROU LEAD PACKER, E
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	1 🗌 FRESH 3 🗌 SULPHUR 29 2 🗌 SALTY 4 🗌 MINERAL	24-25 1 STEEL 26	0/4_1	18-21 22-25	<u> </u>
	$1 \square FRESH \qquad 3 \square SULPHUR 2 \square SALTY \qquad 4 \square MINERAL $	2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE		26-29 30-33 80	
		11-14 DURATION OF PUMPING]	LOCATION O	FWELL
71 PUMP		GPM. 03 15-16 30 17-18 HOURS 30 MINS.	- IN DI/	GRAM BELOW SHOW DISTANCES (NE. INDICATE NORTH BY ARROW	OF WELL FROM ROAD AND
	WATER LEVEL END OF PUMPING 9-21 22-24 15 MINUTES	R LEVELS DURING 2 RECOVERY			
FOSO	EET 08/ FEET 08/	T 08/ FEET 08/ FEET 08/ FEET			
	38-41 PUNP INTAKE	SET AT WATER AT END OF TEST 42	2		
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	MOST	OTHER MATERIALS				DESCRIPTION		DEPTH	FEET
GENERAL COLOUR		01						O	18
Dr.	Clay	Stones						18	93
Gr.	Clay	Stones						93	100
Gr.		ne						100	220
Dr.	Limesto	ne	1					,00	<u>u</u>
			1						
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	······································								
31							بيا لي	1111	ļļ ļ
				<u></u>				34-38	
	TER RECORD	51 CASING & OPEN	Π	CORD			11-33 DIAMETER	INCHES	FEE
WATER FOUND AT - FEET	KIND OF WATER	DIAM MATERIAL THICK	NESS			L AND TYPE	o	EPTH TO TOP F SCREEN	41-44
1.0101	SALTY 4 IMINERALS 6 GAS	10-11 1 D'STEEL 4 2 GALVANIZED 3 CONCRETE	78 -						FEET
11000	FRESH 3 DSULPHUR 4 DMINERALS 5 SALTY 6 DGAS	5 3 CONCRETE 4 OPEN HOLE 5 PLASTIC 17-19 19	0 2) 100	DEPTH SET		ATERIAL AND T	CEME	NT GROUT
	FRESH 3 DSULPHUR 24	5 " ¹ STEEL 2 GALVANIZED 3 CONCRETE 4 BROPEN HOLE 5 PLASTIC	100	0 220	FROM (0-13	TO 14-17		LEAD PA	CKER ETC)
	FRESH 3 SULPHUR SALTY 6 GAS	24-25 26	10.	27-30	18-21	22-25			
30-33		2 GALVANIZED 3 CONCRETE 4 DOPEN HOLE 5 DPLASTIC			26-29	30-33 80			
PUMPING TEST M	$\Box SALTY & \Box GAS$	E IN-14 DURATION OF PUMPING			L	CATION O			
171	2 🗌 BAILER	grm				SHOW DISTANCES		ROM ROAD A	ND A
	PUMPING	1 2 PUMP: LEVELS DURING 2 RECOV 5 30 MINUTES 45 MINUTES 66		LOT LI		ATE NORTH BY AR	ROW		7
53	80 87	$\begin{array}{c} 30 \text{ minutes} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} 30 \\ \end{array} \\ $	80			66'	\ A		/ .
	38-41 PUNP INTAKE	E SET AT WATER AT END OF TEST	42			<u>л</u>	\rightarrow •		۸/
		FEET	CLOUDY						14
	DW DEEP SETTING	135 FEET RATE 7	GPM						
SHALL	1 K WATER SUPPLY	S ABANDONED, INSUFFICIEN	T SUPPLY	17		52			
SO-S3	I UP WATCH SUFFEL			ຸລິ	'~	ŝ			
FINAL STATUS	2 OBSERVATION WE	7 UNFINISHED		2					
FINAL	2 OBSERVATION WE 3 TEST HOLE 4 RECHARGE WELL	7 UNFINISHED 9 Dewatering		Ċ	05			1.	7 14
FINAL STATUS	2 OBSERVATION WE 3 TEST HOLE 4 RECHARGE WELL	7 UNFINISHED		ې 	CON			Lu	14
FINAL STATUS OF WELL	2 OBSERVATION WE TEST HOLE PECHARGE WELL S3-56 T CK DOMESTIC Z STOCK	7 [] UNFINISHED 9 [] DEWATERING 5 [] COMMERCIAL 6 [] MUNICIPAL		ن 	Con				
FINAL STATUS OF WELL WATER USE	2 OBSERVATION WE 3 OBSERVATION WE 4 RECHARGE WELL 53-56 I DK DOMESTIC 2 STOCK 3 IRRIGATION 4 INDUSTRIAL 0 OTHER 57 I CABLE TOOL	Y UNFINISHED 9 DEWATERING COMMERCIAL MUNICIPAL PUBLIC SUPPLY COOLING OR AIR CONDITIONIS POT USED COOLING OR AIR CONDITIONIS COOLING OR AIR CONDITIONIS		ن 	Con				T 13
FINAL STATUS OF WELL WATER USE METHOD OF	2 OBSERVATION WE 3 TEST HOLE 4 RECHARGE WELL 35-54 I DK DOMESTIC 2 STOCK 3 IRRIGATION 4 INDUSTRIAL 0 OTHER 57 1 CABLE TOOL 2 GARDY (CONVEL 3 ROTARY (REVERS	Y UNFINISHED S DEWATERING S COMMERCIAL G MUNICIPAL PUBLIC SUPPLY COOLING OR AR CONDITIONIN O NOT USED COOLING OR AR CONDITIONIN O DIAMOND SE) O JETTING		ڻ 	Con			Lo	TIZ
FINAL STATUS OF WELL WATER USE	2 OBSERVATION WE 3 TEST HOLE 4 RECHARGE WELL 35-54 I DK DOMESTIC 2 STOCK 3 IRRIGATION 4 INDUSTRIAL 0 OTHER 57 1 CABLE TOOL 2 GARDY (CONVEL 3 ROTARY (REVERS	Y UNFINISHED 9 DEWATERING 9 COMMERCIAL 4 MUNICIPAL 7 PUBLIC SUPPLY COOLING OR AIR CONDITIONIS 9 NOT USED 6 BORING NTIONAL 7 DIAMOND 5E) 6 JETTING 9 DIRVING			Con			Lo	τι <u>3</u> 5915
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCT	2 OBSERVATION WE 3 OBSERVATION WE C DESTHOLE C RECHARGE WELL 31-34 1 DK DOMESTIC 2 STOCK 3 OHER 0 OTHER 57 1 CABLE TOOL 2 DK-ROTARY (REVERS 3 OTARY (REVERS 1 OTARY (REVERS)	Y UNFINISHED 9 DEWATERING 9 COMMERCIAL 4 MUNICIPAL 7 PUBLIC SUPPLY COOLING OR AIR CONDITIONIS 9 NOT USED 6 BORING NTIONAL 7 DIAMOND 5E) 6 JETTING 9 DIRVING	THER TRACTOR'S	DATA		↓ 	DATE RECEIVED	<i>ل</i> ه 36	τ 13 5915
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCT	2 OBSERVATION WE 3 TEST HOLE 4 RECHARGE WELL 33-56 I DK DOMESTIC 2 STOCK 3 IRRIGATION 4 INDUSTRIAL 0 OTHER 57 I CABLE TOOL 2 DK-ROTARY (CONVE) 3 ROTARY (REVERS 4 ROTARY (AIR) 4 AIR PERCUSSION	Y UNFINISHED DEWATERING DEWATERING GONMERCIAL MUNICIFAL PUBLIC SUPPLY COOLING OR AIR CONDITIONIS O INOT USED O ING NTIONAL O ING DIGGING 0	THER TRACTOR'S	DATA SOURCE NO DATE OF INSP	" 2	1311 7 1155FECTOR	DATE RECEIVED	<i>ل</i> ه 36	τ 13 5915
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County or District	JE	Township	Borough/City)	(Corr block tr	act surve	ey, etc. Lo	้เรื
		Address	2#3	ALTO		_ تم		ate ompleted	æ c	
21	T L		Northing	•••••	RC Eleva		Basin Code		day m iii	onth y iv
12	10	G OF OVERBURDE	EN AND BEI		TERIALS (see instruct	ions)			
General colour	Most common material	O	ther materials			General	description		De	pth – fee To
Bacun	Chay	STONE	s. Ge	LAJEL					0	20
EREY	Curry		·						20	55
Grey	Crey	Gea	ುಕ್ಲ						55	101
LIGHT 1 EROLDA	himestant-								100	115
Beach	himester								115	143
								1	ET.	
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31 , , ,			Mer - Sc-	<u> </u>	<u>ان محت</u> ا ا را را را	2005		11		
32										
	ER RECORD 51		OPEN HOL	E RECORI		Sizes of o	pening 31-33	Diameter	34-38 Leng	th 3
Water found at – feet	Kind of water dian	n Material ies	thickness inches	From	То	Material a			Depth at top	f
	Salty 6 Gas	10-11 1 Sket 12 2 Galvanized 3 Concrete		+ >	13-16	- sc				feet
1-1-3 20	Salty & Gas	Open hole Den hole Plastic	. (88	+2	100 20-23	61	PLUGGING 8		IG RECOR	D
20-23 1	Fresh ³ C Sulphur ²⁴ 4 Minerals Salty ₆ Gas	1 ∐ Steel				Depth set at -	feet		Abandonm	
25-28 1	Fresh ³ Sulphur ²⁹	5 🛛 Plastic		100	143	From C ¹⁰⁻¹³	10	5~35,54	-	anorme, e
	Fresh ³ Sulphur ³⁴ ⁶⁰	1-25 1 ☐ Steel 26 2 ☐ Galvanized 3 ☐ Concrete			27-30	18-21	22-25			
2 []	alty 6 ☐ Gas	4 🗌 Open hole 5 🗌 Plastic				26-29	30-33 80			
71 Pumping test m	ethod ¹⁰ Pumping rate	GPM Hours	ing 17-18 Mins	1		LOC	CATION OF WI	ELL		
Chatta Jawal W	/ater level 25 Water levels during	' ⊡-₽ dmping 2	P 🗋 Recovery	1	In diagram Indicate no	below show rth by arrow.	distances of we	ell from ro	ad and lot l	ne.
19-21	22-24 15 minutes 30 minu 26-28	tes 45 minutes 32-34	60 minutes 35–37	1		-	- I	(r)		
IS I f flowing give ra	te 38-41 Pump intake set at	Water at end of te	12Cleet			مىسىن بو		1		
Recommended	GPM	feet Clear 43-45 Recommended	Cloudy 46-49	4				•		
Shallow	Deep /30	pump rate	/ 2 GPM		(010	and the second	. 1		
50-53						4.12'				
FINAL STATUS	on well 🛛 🛍 Abandoned, poor q	puality 🕫 🗌 Replac	shed cement well							
³ ☐ Test hole 4 ☐ Recharge	 7 D Abandoned (Other) well ⁸ Dewatering 									
WATER USE	⁵⁵⁻⁵⁶ ⁵ □ Commercial	9 🗋 Notus	ed	1				ne. ile		
2 🗌 Stock 3 🗍 Irrigation	⁶ Municipal ⁷ Public supply	10 🗋 Other.		-	#24	HWY	\$°			
4 🗌 Industrial	8 Cooling & air condi	uoriing			Gue	- 1-1-			FE	· ~
ו 🗌 Cable too	ONSTRUCTION 57	9 Driving	g				175	-	/	
2 ☐ Rotary (co 3 ☐ Rotary (re 4 ☐ - Potary (ai	nventional) €	10 Diggin 11 Dother	ng				منعان کــــــــــــــــــــــــــــــــــــ	ge Tend	1988	89
		Wall Oracle	ar's Linen M	Data	5	Contracctor		2 Date rece	aved	63-68
Name of Well Contra	Men Din .	Well Contract	tor's Licence No.		e	2 6	63	AUG		,99
Address RCH	56	1 4		I W	of inspection	ir	spector			
Name of Well Techn		Well Technicia	an's Licence No.		arks	I				
\sim		in 7:03								

	Environme es provided.			-	53	7136					
	with a checkmark, where	applicabl		2		130	74	6.7.0			22 23
ounty r District	LEALINGTON)	Township/Bor	rough/City/T				Con bloc	k tract surv	ey, etc. L	ot
wper's sumame	28-47 Fir	st Name	Address			, wee		- 17	Date completed		06 01
	Z	one Ea		orthing	//66/		ation RC	Basin Code		day i ii	month yea iv
12	Ш М 10										
eneral colour	Most common mater		OVERBURDEN AN Other m			ENIALO (S		description		Dep	th - feet To
Brun	CLACI								6.000	0	ß
BROWN	CLAY		Eren	ē4			×			85	97
BRGE	Limeston	テ								97	BC
Brun	LIMESTON	- تى								130	143
						10	TAL	=	146	<u>]=</u>	
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		1	<u>6 / L / "</u>	CA	5-20	K- E	Jain	ie e)MOG	=	1
$\frac{1}{2}$											
		51						opening	31-33 Diamete	ər 34-38 Len	 igth ³⁵
ter found	Kind of water	Inside diam	Material	Wall thickness	Depth - From	f oot To	1 1 101-111-			Jan 105	te
1210-1210	Fresh ³ Sulphur ¹⁴ ⁴ Minerals Salty ⁶ Gap	10-11	1 CLSteel 12 2 C Galvanized	inches		13-16	Material	and type		Depth at top	41-44
-	Fresh 3 Gas 4 Minerals	64	3 Concrete	188	12	44					feet
<u></u>	Salty 4 Minerals 6 Gas Fresh 3 Sulphur 24	17-18	5 Plastic 1 Steel 2 Galvanized		00	20-23		Annular space	G & SEALIN	G RECOR	
2 🗆	Salty 6 □ Gas	6/4	3 Concrete 4 Copen hole 5 Plastic		47	FL	Depth set	ToMa	terial and type (Cement grout, I	centonite, et
] Fresh ³ □ Sulphur ²⁹ 4 □ Minerals 6 □ Gas	24-25	1 Steel 26 2 Galvanized			27-30	010-13 18-21	22-25	ENS	EAL	•
] Fresh ³ ☐ Sulphur ³⁴ ⁶⁴ 4 ☐ Minerals] Salty ₆ ☐ Gas	D	3 Concrete 4 Open hole 5 Plastic				26-29	30-33 80		. <u>.</u>	
		D = 11-10									
Pumping test m 1 Pump 2	ethod 10 Pumping rate - Bailer 25		Hours	17-18 Mins		In diagrar	LO n below sho	CATION OF w distances		road and le	ot line.
	22-24 15 minutes 26-28	during 1 30 minutes 29-3	÷ · ·	Recovery minutes		Indicate n	orth by arro	₩.	\cap		
65	B / feet 80 feet	E1,	RIF	3/ ³⁵⁻³⁷ feet					N		
If flowing give ra	70.44		Water at end of test	42 Cloudy				and	+		
Recommended p	ump type Recommended			46-49				FUN			
Shallow	Deep pump setting a		et CA	J GPM				3	5'		
NAL STATUS	SOFWELL 54 ply 5 🗆 Abandoned		supply ⁹ 🗌 Unfinished					-1-2) م <i>الخ</i> لان		5
 ² Observation ³ Test hole 	on well ⁶ Abandoned ⁷ Abandoned	1, poor quality 1 (Other)		nt well	~		-6	Rink	Huer	1	Ē
									-11	124	
ATER USE 1 Domestic 2 Stock	55-56 5 🔲 Commercia 6 🔲 Municipal		9 🗌 Not use 10 🗌 Other								
3 🗍 Irrigation 4 🗍 Industrial	7 Public sup 8 Cooling & a	oly air conditionin					h	404			
ETHOD OF C	CONSTRUCTION 57	<u> </u>					(*	125			
1 Cable tool 2 Cable tool 3 Rotary (co	nventional) ⁶ 🗌 Boring	sion	 9 Driving 10 Digging 11 Other 				}			005	
 ³ Bolary (re- ⁴ Hotary (air 		<u></u>					Ac-	Ten		225	424
ame of Well Contra	actor	~	2) Well Contractor's	Licence No.	Data source		58 Contractor	669	59-62 Date r	N 28	20 01 °°
(Jana	N/Mere Oki	isino	- 000:	2	No Data	of inspection	2	003	JU	14 4 0	2001
ddress	/			177	1 22			1			
		ECPH	Wall Tachninian's	Licence No		arks		1			
	nician	EL	Well Technician's		SN ALLSINH	arks		1		CSS.ES	·

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	the Environment	Well Tag No. (Place Sticker Tag#: A1521	,	Regulatie	on 903 Ontario	Well R Water Res	
Address of Well Loo	cation (Street Number/Name)	Township		Lot	Conces		
340 Le We County/District/Mur	llington Rd 124	Erin		14		3	
County/District/Mur	licipality	City/Town/Village			Province Ontario	Postal	
Wellin UTM Coordinates	ne Easting Northing	Ospringe Municipal Plan and Su	blot Number		Other		46117
	17 5697974838						
Overburden and I	Bedrock Materials/Abandonment S	ealing Record (see instructions on	the back of this form)				
General Colour	Most Common Material	Other Materials	Gene	ral Descriptic	n	Depl From	h Or Ho
Brown	(las + stones					0	5.48
-	Chan & storad					5.48	
Gray	Clay & stones					11	26.3
Diown	1 in estant					26.51	51.2
pray	limestone					51.20	60.1

	Annular Space			esulte of W	ell Yield Testii		
Depth Set at (1)	Type of Sealant Used		After test of well yield, v	vater was:	Draw Dowr		covery
From To	(Material and Type)	(C)71(P)	Clear and sand fr	ее	Time Water L	evel Time \	Vater Level
0 10	Bentonite (200	-t .75	LI I LUIDAT SOACITU		(min) (m/ft)	(<i>min</i>)	(m/ft)

							If pumping discontinued, give reason:	Level	16.20		21.38
								1	17. 79	1	19.40
							Pump intake set at (m/ft)	2	18.49	2	18.77
Met	thod of Construction			Well Us	e		Pumping rate (I/min / GPM)	3	18.96	3	18.35
Cable T				Comme	rcial 🗌 Not u		Duration of pumping	4	19.30	4	18.05
Rotary	(Conventional) Use Jetting (Reverse) Driving	and the second s	omestic vestock	Municipa Test Hol			hrs +min	5	15.55	5	17.86
Boring	Digging :		igation dustrial	Cooling	& Air Conditioning		Final water level end of pumping (m/ft) 21.33	10	20.22	10	17.36
Other, s		ينيني يتبتيك والمستحد	her, <i>specil</i> y				If flowing give rate (I/min / GPM)	15	20.41	15	17.20
Inside	Construction Re Open Hole OR Material	ecord - Ca Wall		n (m/k)	Status of W			20	20.79	20	16.99
Diameter (cpt/in)	(Galvanized, Fibreglass, Concrete, Plastic, Steel)	Thickness (cn/m)	From	То	Replacement		Recommended pump depth (m/ft)	25	20,94	25	1636
15.9	stal	.48	170	27.12	Test Hole	ell.	Recommended pump rate (<i>l/min / GPM</i>)	30	21.03	30	16.76
15.6	37	10			Dewatering W		ID uj	40	21.20	40	16.70
13.6	open hole		27.12	60.04	Monitoring Hol		Well production (I/min / GPM)	50	21.30		1657
					(Construction))	Disinfected?		21.38	60	
	Construction Re	cord - Scre	en		Insufficient Su		Map of We				16.47
Outside Diameter	Material	Slot No.	Depth	(m/ft)	Abandoned, F Water Quality		Please provide a map below following i			ck.	
(cm/in)	(Plastic, Galvanized, Steel)	0.07140.	From	To	Abandoned, o specify	ther,					
					Other, specify		irell				/ /
					Other, specily		× +			/	/ /
Watar four	Water Deta nd at Depth Kind of Water:		7		le Diameter				/		
	Gas Other, spec		Contested	Depth From	To Dian	anter (M)	6			/	·
Water foun	d at Depth Kind of Water:	Fresh	Untested	0	27.12 22	8	ice and	<i>(</i>	/ /	·	
	Gas Other, spec		Untested	27.12	60.04 15.	6		- il			
	n/ft) 🔄 Gas 🗌 Other, spec						in the	124			
Business N	Well Contractor ame of Well Contractor	and Well	Techniciar								
well	Initiatives			7	Contractor's Licence	∋ No. 1					
-	dress (Street Number/Nam	ne)			icipality		Comments:				
<u>15</u> Province	Postal Code	Business	E-mail Addr	ess	angeulle						
ON	29 W3 R.	1 inte	DA L	o Miniti	iatives.co	n	Well owner's Date Package Delivered		Ministry	/ Use	Only
Bus.Telepho	ne No. (<i>inc. area code</i>) Nam 8 4 6 8 2 89 /-	ne of Well Te	echnician (La	ast Name, Fi	rst Name)		information package delivered		Audit No.Z 1		8903
	an's Licence No. Signature c	f Techniciar	and/or Cor	tragtor Date			Date Work Completed		-		
1 9 0506E (2007/1	2 7		Low	L 20	14071		- NO 2014071		Leceived JUI	_ 23	3 2014
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ell Owr	ner's Information	ast Name / 0	Organization	1		E-mail Address				Woll	Constructe
M BL	UE PLAN ENGI	NEERIN		THOMA ITED	SFIELD HOME	S				by W	ell Owner
	dress (Street Number/Nan OODLAWN RD W		00.0	UNIT 2	lunicipatity GUELPH	Province ONT	Postal Code	1 m	Telephone	No. (inc	. area code
ell Loca	total and the second	131 01	00 0	UNIL 2	. <u></u>	I UNI	IN LAIL DO				
dress of	Well Location (Street Nur	nber/Name)	14-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	T	ownship ERIN	1. 1. 1.	Lot 13		Concessio 2	n	
unty/Dist	trict/Municipality			С	ity/Town/Village	C		Provin	се	Posta	al Code
11 Canad	inates Zone , Easting	hla	rthing		lunicipal Plan and Suble	of Number		Onta	ario		
NAD	96	9682	48284	49	unicipal rian and outin			Outor			
erburde	en and Bedrock Materia		nment Sea				10			De	pth (<i>m/ft</i>)
eneral Co				Othe	er Materials	Gene	aral Description	1		From	To
ROWN			A. 15 194							0 27ft	271
RAY		STONES			TW-						124f
					1~	'				135f	
RAY	LIMESTO	NE	1			1				2002	4 1.44
						-				1717	
-					and the second	Charles and					
		Annular					Results of We	Contraction of the second		-	
Depth Se From	et at (<i>m/ft)</i> To	Type of Sea (Material an			Volume Placed (m³/ft³)	After test of well yield,		Time		el Time	Recovery Water Lev
)	129ft BENT	ONITE	SLURR	Y 90ga	1	Other, specify	od also reason:	(min) Static	(m/ft)	(min)	(m/ft)
					1000 2000	If pumping discontinu		Level	82f	Ē.	1
						TUMP NEST U	10107		1	4	
						BY ENGINT	部化	1		1	
		199				011	部化	2	(2	
Meth	nod of Construction			Well Us	e	BY ENGINI Pump intake set at (100ft Pumping rate (Vmin)	551 <u>(</u> m/ft)	2	5	2	
Cable To	ool 🗌 Diamond			Commer	rcial 🔲 Not used	BY ENGINI Pump intake set at (100 ft	EBR m/ft) / GPM)	2 3 4	5	2 3 4	
Cable To Rotary (C Rotary (F	conventional) Diamond Conventional) Jetting Reverse) Driving	Doi Live	mestic estock	Commer	rcial INot used	Pump intake set at (100 ft Pumping rate (<i>Vinin</i>) 20g pm Duration of pumping 2 hrs + 0	SBR m/ft) / GPM) min	2 3 4 5	83£1	2 3 4	B3ft
Cable To Rotary (C Rotary (F Boring Air percu	col Diamond Conventional) Jetting Reverse) Driving Digging ussion	Dol Live Inrig	mestic estock gation ustrial	Commer	cial Not used	Pump intake set at (100ft Pumping rate (<i>l/min</i> 20g pm Duration of pumping	SBR m/ft) / GPM) min	2 3 4 5 10	836	2 3 4 5 10	B3ft
Cable To Rotary (C Rotary (F Boring Air percu	col Diamond Conventional) Jetting Reverse) Driving Digging ussion pecify	Dol Live Irrig Ind Oth	mestic estock gation ustrial ner, <i>specify</i> _	Commer	rcial Not used al Dewatering e Monitoring & Air Conditioning	Pump intake set at (100 ft Pumping rate (I/min 20g pm Duration of pumping 2 hrs + 0 Final water level end	(GPM) (GPM) min of pumping (m/tt)	2 3 4 5 10 15	836	2 3 4 5 10 15	B3ft
Cable To Rotary (C Rotary (F Boring Air percu Other, sp Inside	ool Diamond Conventional) Jetting Reverse) Driving Digging ussion pecify Construction R Open Hole OR Material	Dot Live Ind Oth ecord - Cas Wall	mestic estock gation ustrial her, specify _ ing	Commer	rcial INot used	Pump intake set at (100ft Pumping rate (<i>Vinin</i>) 20gpm Duration of pumping 2 hrs + 0 Final water level end 85ft	(GPM) (GPM) (min of pumping (m/tt) (min / GPM)	2 3 4 5 10	8361	2 3 4 5 10	83ft
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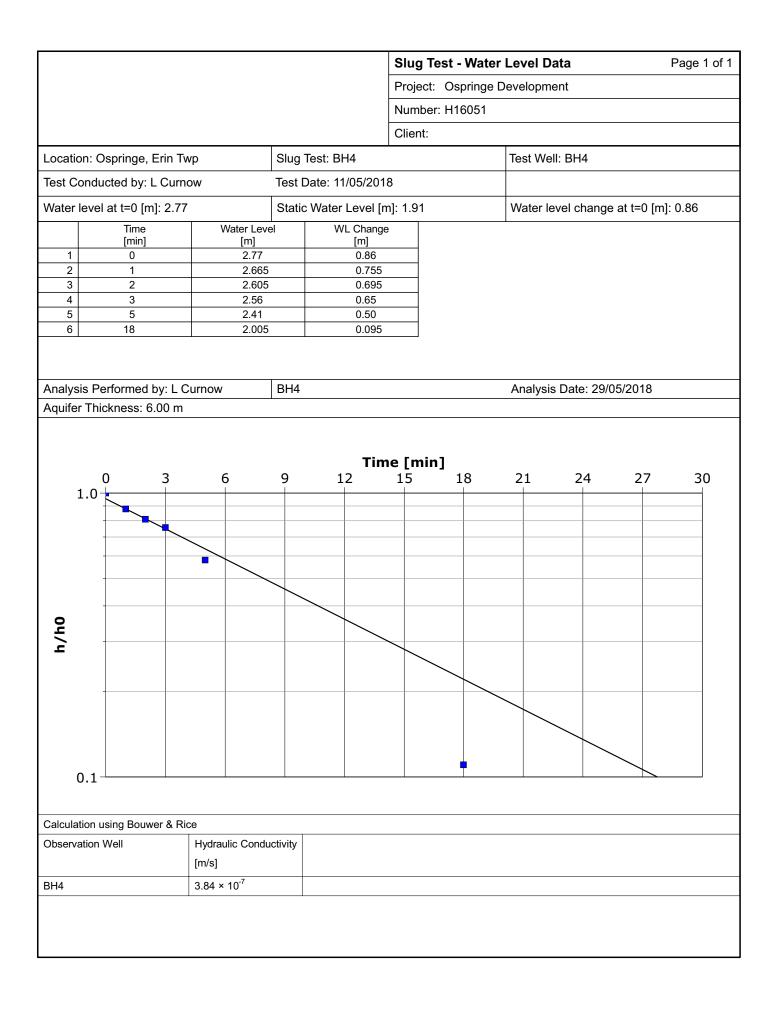
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Vell Own rst Name	ents recorded in: 🗌 M	etric 🔲 I	mperial		60{ lag#:	A173608			Page		of
	er's Information	and Marrie 10	Description			E mail Address					
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idress of V	Well Location (Street Nun	iber/Name)			ownship erin		13		2		
ounty/Distr	rict/Municipality			Ci	ty/Town/Village			Provin		Postal	Code
	nates Zone Easting		rthing		unicipal Plan and Subl	ot Number		Other		-	
NAD verburde	8 3 17 5700 n and Bedrock Materia	1.21	nment Sea	2011 - Land	d (see instructions on the	e back of this form)	a la serie de la s	192		Dee	ah. ((0))
Seneral Col				Othe	er Materials	Gen	eral Description		-	From	th (<i>m/ft</i>) To
BROWN	CLAY & S									0 36ft	36ft
BRAY	LIMESTO		ES			-				1061	t139
RAY	LIMESTON				10-	S				139ft	1621
THE A	DINEDION	D					1				
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							The second second				
		Annular	Space				Results of We				
Depth Set From	t at (<i>m/ft</i>) To	Type of Sea (Material an			Volume Placed (m³/ft³)	After test of well yield		Time	aw Down Water Lev	vel Time	ecovery Water Leve
	112ft BENT	ONITE	SLURR	¥ 75g	al	Other, specify	led, give reason:	(min) Static	(m/ft)	(min)	(m/ft)
						pumptestd		Level 1	48ft	1	-
	NO PARA		_			Pump intake set at	(m/ft)	2		2	
						100ft Pumping rate (Vmin	/ GPM)	3		3	
Metho Cable Too	od of Construction	D Put	olic	Well Use	cial 🗌 Not used	20gpm Duration of pumping		4		4	
Rotary (C	conventional) Jetting Leverse) Driving	Dor Live		Municipa	A. A CONTRACTOR AND A C	8_hrs +_0		5		5	
Boring Air percus	Digging Ssion	Irrig		Cooling &	& Air Conditioning	Final water level end 72ft	of pumping (m/ft)	10	1	10	
Other, spe			er, specify	_	Status of Well	If flowing give rate (l/min / GPM)	15	68ft	15	
Inside	Construction Re Open Hole OR Material	Wall	the second s	n (<i>m/ft</i>)	Water Supply	Recommended pur	np depth (m/ft)	20	(20	
Diameter (cm/in)	(Galvanized, Fibreglass, Concrete, Plastic, Steel)	Thickness (cm/in)	From	To	Replacement Well Test Hole	100f		25	70ft	25	
54	steel	.188	0	112f	Recharge Well Dewatering Well	1001 10010	, g p m	30 40	71ft		
6in	open hole		1121	t162f	Observation and/or Monitoring Hole	Well production (I/m	in / GPM)	50	(50	
					(Construction)	Disinfected?		60	721		
	Construction R	ecord - Scre	en		Abandoned, Insufficient Supply Abandoned, Poor		Map of W				-
Outside Diameter	Material (Plastic, Galvanized, Steel)	Slot No.	Depti	n (<i>m/ft</i>)	Water Quality	Please provide a ma	p below following	instruct	ions on the	e back.	
(cm/in)	(Flastic, Galvallized, Sleel)	-	From	То	specify	1.5.5					
				-	Other, specify						F
	Water Det	ails		H	ole Diameter						45
	d at Depth Kind of Water /ft) Gas Other, spe		Untested	Dept From	h (m/ft) Diameter To (cm/in)	I.T.L.		-			20
Vater found	d at Depth Kind of Water	r: Fresh [Untested	0	112ft 8.75	PI PI					IV Y
	(ft) Gas Other, spe d at Depth Kind of Water		Untested	112ft	162ft 6in	1 L			TH	OUSP	
	(/ft) Gas Other, spe			2		I FIE	CD				le st
	Well Contractor	r and Well	Technicia		ion I Contractor's Licence No.	Fiel XN	FIL				
	H LANG WELL		NG IN		7 154	Comments:	FUL				-
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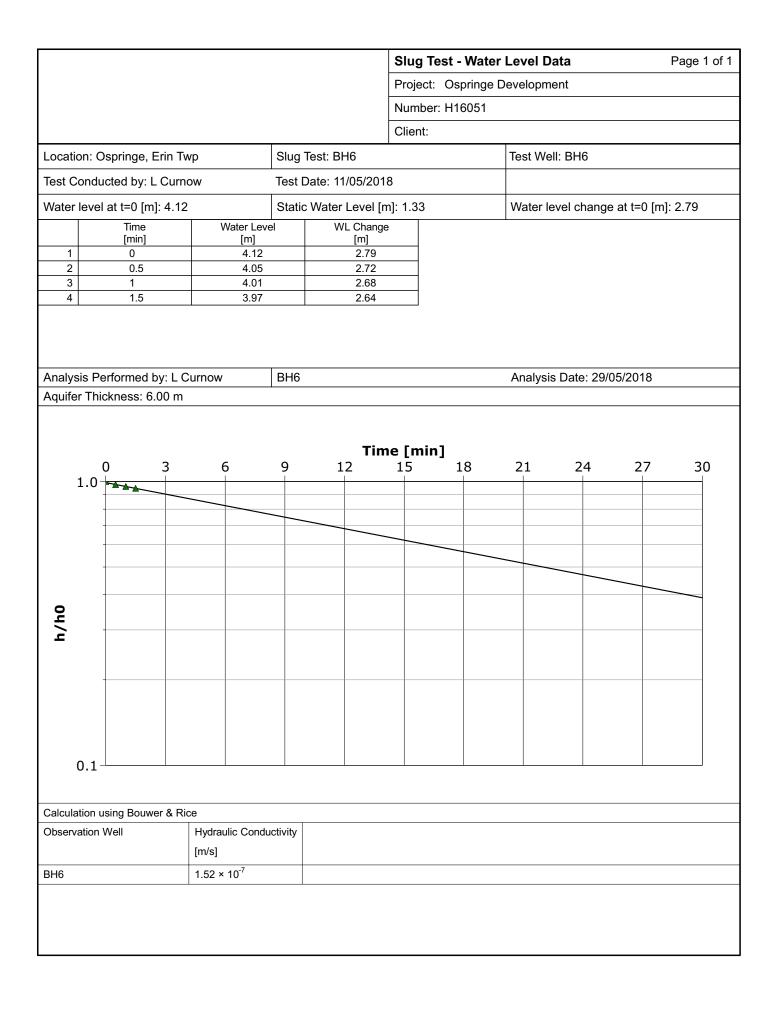
February 27, 2019 **FILE NO.:** H16051 Page E

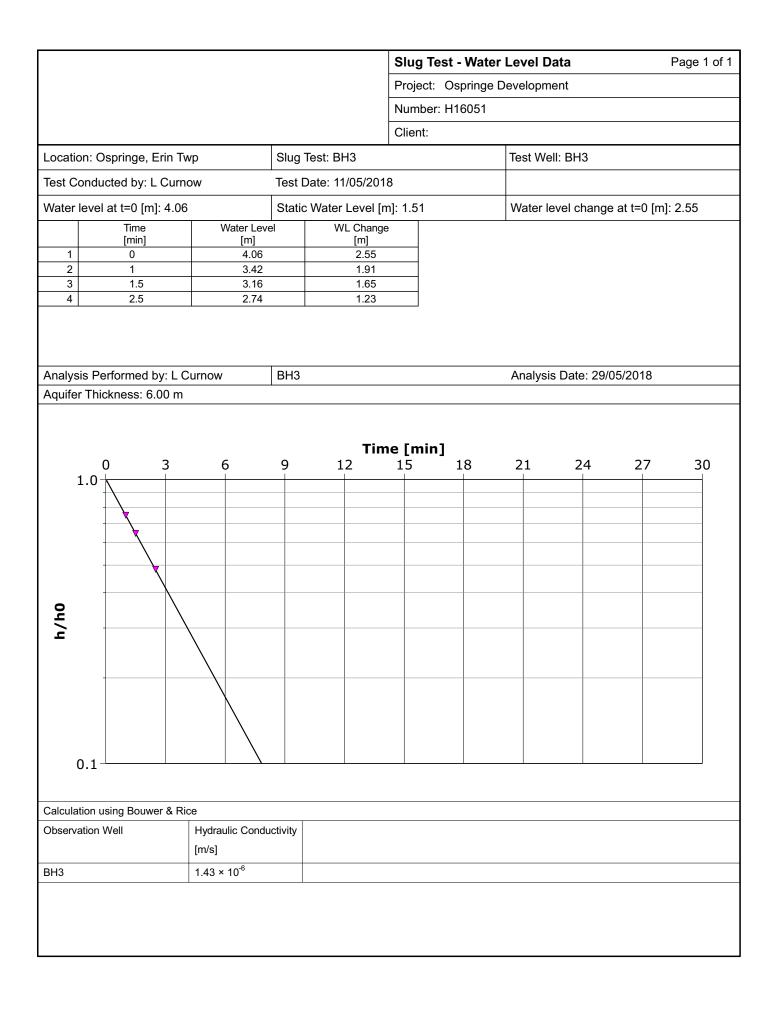
APPENDIX E Well Response Test Analyses & Thomasfield Test Well Aquifer Parameters

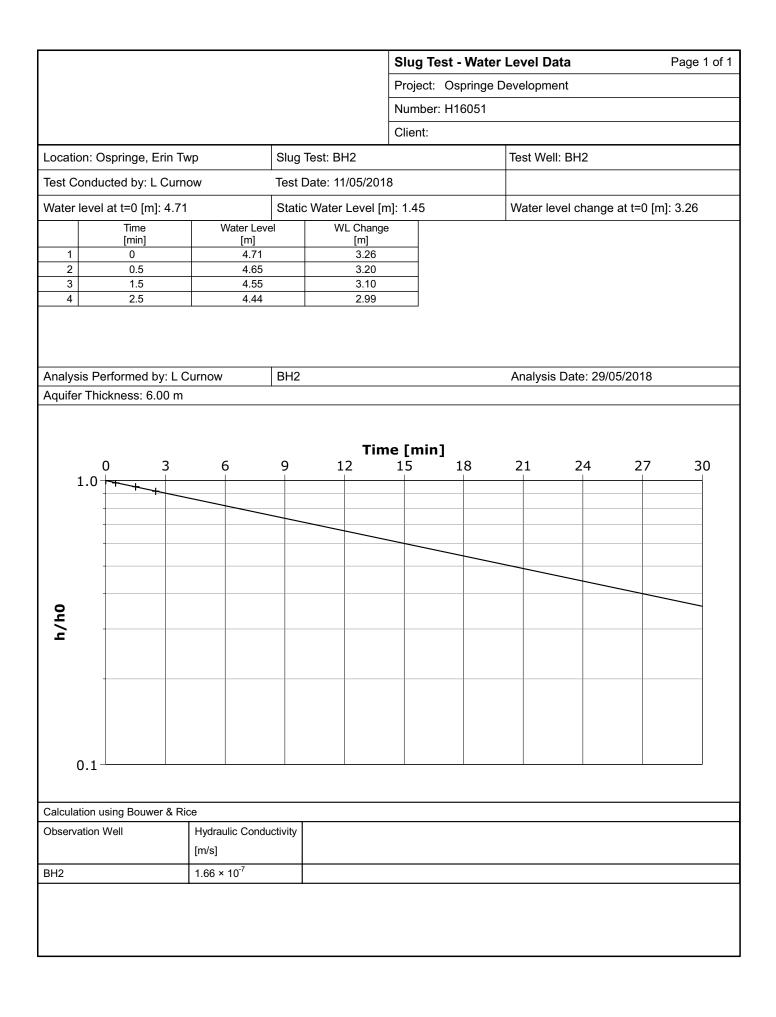




						Slug Tes	st - Wate	r Level D	ata		Page 1 of 1
						Project:	Ospringe	Developm	ent		
						Number:	H16051				
						Client:					
Location: C)springe, Eri	n Twp		Slug Te	st: BH5	•		Test We	ell: BH5		
Test Condu	icted by: L C	Curnow		Test Dat	te: 11/05/201	8					
Water level	at t=0 [m]:	6.50		Static W	/ater Level [I	n]: 2.07		Water le	evel change	at t=0 [m]:	4.43
	Time [min]		Water Leve [m]	1	WL Chang [m]	e					
1	0		6.50		4.43						
2	0.5		6.22		4.15						
3	1		5.96		3.89						
4	1.5		5.68		3.61						
Analysis Pe	erformed by	: L Curnov	V	BH5				Analysi	s Date: 29/0)5/2018	
Aquifer Thi											
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Calculation	ising Bouwer	& Rice									
Observation			aulic Condu	ctivity							
		[m/s]		Stivity							
			4.0-7								
BH5		7.11	× 10 ⁻⁷								
L											







Observation Well	Test Well	T	S	H_A
	()	(m²/s)	()	(m)
T.M/ 1	TW-2	1.19E-03	1.11E-04	13.2
Τ-ΛΛΙ	TW-3	3.08E-03	1.25E-04	13.2
C /WI	TW-1	1.10E-03	1.09E-04	13.1
7-771	TW-3	3.22E-04	4.05E-05	13.1
C /V/T	TW-1	6.13E-03	3.00E-04	18.2
C- M I	TW-2	5.13E-04	5.33E-05	18.2

 \mathbf{Q}_{t} : Discharge flow rate during pumping test

T: Transmissivity of the Bedrock Aquifer

S: Storativity of Bedrock Aquifer

H $_{\rm A}$: Available Drawdown, height of water column above the top of the aquifer.



GM BluePlan Engineering Ltd. Guelph, Owen Sound, Listowel, Kitchener, Exeter, Hamilton, GTA 650 Woodlawn Rd. W. Block C, Unit 2, Guelph, ON N1K 1B8 www.GMBluePlan.ca **Hydrogeological Investigation 13-Lot Ospringe Subdivision** Part Lot 13, Conc 2, Town of Erin February 27, 2019 **FILE NO.:** H16051 Page F

APPENDIX F ALS Laboratory Analysis Report & Thomasfield Test Well Quality





CHUNG AND VANDER DOELEN ATTN: SANDY ANDERSON 311 VICTORIA ST. N. KITCHENER ON N2H 5E1 Date Received: 12-MAY-18 Report Date: 17-MAY-18 14:20 (MT) Version: FINAL

Client Phone: 519-742-8979

Certificate of Analysis

Lab Work Order #: L2093493

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED H16051 - OSPRINGE 17-626973

Mary-Lynn Pike Client Services Supervisor

[This report shall not be reproduced except in full without the written authority of the Laboratory.]

ADDRESS: 60 Northland Road, Unit 1, Waterloo, ON N2V 2B8 Canada | Phone: +1 519 886 6910 | Fax: +1 519 886 9047 ALS CANADA LTD Part of the ALS Group An ALS Limited Company

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ALS ENVIRONMENTAL ANALYTICAL REPORT

Sample Details/Parameters	Result	Qualifier*	D.L.	Units	Extracted	Analyzed	Batch
L2093493-1 BH1 Sampled By: SA on 11-MAY-18 Matrix: WATER							
Anions and Nutrients							
Chloride (Cl)	14.0		0.50	mg/L		15-MAY-18	R4045966
Nitrate (as N)	3.01		0.020	mg/L		15-MAY-18	
L2093493-2 BH2 Sampled By: SA on 11-MAY-18 Matrix: WATER							
Anions and Nutrients							
Chloride (Cl)	19.1		0.50	mg/L		15-MAY-18	R4045966
Nitrate (as N)	4.47		0.020	mg/L		15-MAY-18	R4045966
L2093493-3 BH4 Sampled By: SA on 11-MAY-18 Matrix: WATER							
Anions and Nutrients							
Chloride (Cl)	3.82		0.50	mg/L		15-MAY-18	
Nitrate (as N)	4.86		0.020	mg/L		15-MAY-18	R4045966
L2093493-4 BH5 Sampled By: SA on 11-MAY-18 Matrix: WATER							
Anions and Nutrients							
Chloride (Cl)	3.13		0.50	mg/L		15-MAY-18	R4045966
Nitrate (as N)	0.197		0.020	mg/L		15-MAY-18	R4045966
Refer to Referenced Information for Qualifiers (if any) a							

* Refer to Referenced Information for Qualifiers (if any) and Methodology.

	Sample ID	TW-1-01	TW-2-01	TW-2-02	TW-3-01
	Sample Description	Groundwater	Groundwater	Groundwater	Groundwater
	Screened Interval (m asl)	372.4-376.4	345.0-375.2	345.0-375.2	355.5-370.8
	Sampling Date	2015-09-17	2015-09-17	2015-09-17	2015-09-17
Various Parameters in Groundwater	Criteria		Concen	Concentration	
Calculated TDS (mg/L)	500	200	190	190	200
Hardness (CaCO3) (mg/L)	80:100	<u>180</u>	<u>160</u>	170	<u>160</u>
Colour (TCU)	<u>5</u>	2	<2	<2	5
Conductivity (umho/cm)	N	360	340	340	340
(Нd) Нd	6.5:8.5	7.89	7.86	7.95	8.09
Dissolved Sulphate (SO4) (mg/L)	<u>500</u>	9.5	6.5	6.7	7.3
Turbidity (NTU)	5	<u>16</u>	1.8	0.9	<u>33</u>
Dissolved Chloride (CI) (mg/L)	250	1.8	1.9	1.6	1.2
Nitrite (N) (mg/L)	1	<0.010	<0.010	<0.010	<0.010
Nitrate (N) (mg/L)	10	<0.10	<0.10	<0.10	<0.10
Nitrate + Nitrite (N) (mg/L)	10	<0.10	<0.10	<0.10	<0.10
Dissolved Iron (Fe) (ug/L)	<u>300</u>	<100	<100	<100	<100
Dissolved Manganese (Mn) (ug/L)	<u>50</u>	<u>200</u>	18	15	<u>73</u>
Dissolved Sodium (Na) (ug/L)	20000 (<u>200000</u>)	11000	12000	13000	16000
Dissolved Organic Carbon (mg/L)	5	1.4	0.69	0.66	1
Fecal coliform (CFU/100mL)	N<	0	0	0	0
Total Coliforms (CFU/100mL)	0	NDOGT	0	NDOGN	NDOGN
Escherichia coli (CFU/100mL)	0	NDOGT	0	NDOGN	NDOGN

Notes:

Criteria are from the Ontario Drinking Water Objectives (2002). Criteria are indicated by: <u>Underlined</u> for Aesthetic Objective, **Bold** for Maximum Acceptable Concentration, *Italics* for Interim Maximum Acceptable Concentration

Concentrations are as listed for each given parameter.
 Concentrations with bold, italic, or underlined text in shaded cells exceed the corresponding criteria.

4. Screened well intervals presented are approximate.

---- represents sample parameters that were not analyzed; NV = No value specified.
 Maxxam Laboratory job number: B5I8843



GM BluePlan Engineering Ltd. Guelph, Owen Sound, Listowel, Kitchener, Exeter, Hamilton, GTA 650 Woodlawn Rd. W. Block C, Unit 2, Guelph, ON N1K 1B8 www.GMBluePlan.ca

February 27, 2019 FILE NO.: H16051 Page G

APPENDIX G Pre-Development Water Balance



Waterloo-Wellington Normals, Vegitation: Mix Cultivated and Pasture, Soil: Sand/Slit Loam Method, Thornthwaite & Mather, 1957) MegC Method, Thornthwaite & Mather, 1957 deg C Marcoldsy Marcoldsy <	s, Vegitation: Mix Cultivated and Pasture, Soil: Sand/Silt Loam % of Total JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV % of Total JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV $0:00$ <th>Precipitation: Waterloo-Wellington Normals, Vegitation Div Water Balance Method, Thornthwaite & Mather, 1957) Up Units Annual % of Total Temperature deg C - - Temperature - - - Temperature deg C - - Periodiusted PET) mm/day - - Periodiusted PET mm 917.0 - Percum. Water Loss mm 557.3 - Moisture Retention (Storage) mm - - Accum. Water Loss mm 548.0 59.8 Surplus / Deficit mm 369.0 40.2 Nobisture Retention (Storage) mm<!--</th--><th>jitation: Mix Cu Total JAN -7.3 0.00 24.3 54.3 54.3 54.3 0.0 0.0 0.0</th><th></th><th>Pasture IAR 1.5 1.00 0.0 0.0 2.7 2.7 2.7 2.7</th><th>¹, Soil: Se APR 5.8 1.25 0.90 33.6 33.6 30.2 72.6 42.4</th><th>and/Silt L MAY 12.5 4.00 1.98 38</th><th>.oam JUN 17.0 6.38 2.71 38.6</th><th>JUL 19.9</th><th>AUG</th><th>SE SE</th><th>OCT</th><th></th><th></th></th>	Precipitation: Waterloo-Wellington Normals, Vegitation Div Water Balance Method, Thornthwaite & Mather, 1957) Up Units Annual % of Total Temperature deg C - - Temperature - - - Temperature deg C - - Periodiusted PET) mm/day - - Periodiusted PET mm 917.0 - Percum. Water Loss mm 557.3 - Moisture Retention (Storage) mm - - Accum. Water Loss mm 548.0 59.8 Surplus / Deficit mm 369.0 40.2 Nobisture Retention (Storage) mm </th <th>jitation: Mix Cu Total JAN -7.3 0.00 24.3 54.3 54.3 54.3 0.0 0.0 0.0</th> <th></th> <th>Pasture IAR 1.5 1.00 0.0 0.0 2.7 2.7 2.7 2.7</th> <th>¹, Soil: Se APR 5.8 1.25 0.90 33.6 33.6 30.2 72.6 42.4</th> <th>and/Silt L MAY 12.5 4.00 1.98 38</th> <th>.oam JUN 17.0 6.38 2.71 38.6</th> <th>JUL 19.9</th> <th>AUG</th> <th>SE SE</th> <th>OCT</th> <th></th> <th></th>	jitation: Mix Cu Total JAN -7.3 0.00 24.3 54.3 54.3 54.3 0.0 0.0 0.0		Pasture IAR 1.5 1.00 0.0 0.0 2.7 2.7 2.7 2.7	¹ , Soil: Se APR 5.8 1.25 0.90 33.6 33.6 30.2 72.6 42.4	and/Silt L MAY 12.5 4.00 1.98 38	.oam JUN 17.0 6.38 2.71 38.6	JUL 19.9	AUG	SE SE	OCT		
Units Annual % of Total JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV deg C -7.3 -6.8 -1.5 5.8 12.5 17.0 19.9 18.7 14.3 8.0 2.5 - - 0.00 0.00 0.00 0.00 1.25 17.0 19.9 18.7 14.3 8.0 2.5 od PET) mm/day - - 0.00 0.00 0.00 1.25 4.00 5.3 8.10 7.37 4.91 2.04 0.35 n(r) - - 24.3 30.6 33.6 38 8.10 7.37 4.91 2.04 0.35 nm< 917.0 - 24.3 30.6 33.6 38.6 31.2 28.1 17.4 18.8 74.1 nm 917.0 - 54.3 55.6 72.7 72.6 73.3 74.4 18.8 74		ed PET) on (r) ET on (Storage)						JUN 17.0 6.38 2.71 38.6	JUL 19.9	AUG	SEP	OCT		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	ed PET) on (r) ET) ion (Storage)						17.0 6.38 2.71 38.6	19.9		i		NOV	DEC
ed PET) mm/day 0.00	ed PET) mm/day 0.00	ed PET) on (r) ET) ion (Storage)						6.38 2.71 38.6	>>>-	18.7	14.3	8.0	2.5	-4.0
ed PET) mm/day 0.00	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	ed PET)						2.71 38.6	8.10	7.37	4.91	2.04	0.35	0.00
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	on (r) estimates on (Storage)						38.6	3.19	2.99	2.27	1.25	0.38	0.00
ET) mm 557.3 0.0 $0.$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	ert)							38.9	36	31.2	28.5	24.1	22.9
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	oss ion (Storage)						104.7	124.1	107.7	70.8	35.6	9.1	0.0
mm 54.3 55.6 72.7 42.4 1.2 -25.2 -33.7 -14.4 18.8 34.8 74.0 oss mm 0.0 -25.2 -58.8 -73.2 -73.2 ion (Storage) mm 250.0 250.0 250.0 250.0 250.0 250.0 ion (Storage) mm 270.0 0.0 0.0 0.0 250.0 250.0 250.0 ion (Storage) mm 548.0 50.8 70.0 0.0 0.0 250.0 250.0 250.0 ion (Storage) mm 548.0 50.8 0.0 0.0 0.0 0.0 24.0 186.0 204.8 239.6 250.0 ion (Storage) mm 548.0 50.0 0.0 0.0 0.0 24.0 100.0 18.8 34.8 10.4 ion (Storage) mm 548.0 50.0 0.0 0.0 0.0 24.0 103.3 70.8 35.6 9.1 ion (Storage) mm 548.0 50.0 0.0 0.0 0.0 24.0 103.3 70.8 35.6 9.1	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	ion (Storage)						79.5	90.4	93.3	89.6	70.4	83.1	79.2
oss mm 25.2 -58.8 -73.2 ion (Storage) mm 250.0 250.0 250.0 250.0 250.0 250.0 250.0 106.0 186.0 204.8 239.6 250.0 250.0 106.0 186.0 204.8 239.6 250.0 250.0 106.0 186.0 204.8 239.6 250.0 250.0 106.0 186.0 204.8 239.6 250.0 250.0 104.4 106.0 108.0 10.4	oss mm 25.2 -58.8 -73.2 ion (Storage) mm 250.0 250.0 250.0 250.0 250.0 250.0 260.0 186.0 204.8 239.6 250.0 mm 548.0 50.8 0.0 0.0 0.0 0.0 24.0 -30.0 10.8 239.6 250.0 mm 548.0 59.8 0.0 0.0 0.0 0.0 24.0 -30.0 -10.0 18.8 24.8 10.4 mm 548.0 59.8 0.0 0.0 0.0 24.0 -30.0 -10.0 18.8 34.8 10.4 mm 369.0 40.2 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 74.0	ion (Storage)						-25.2	-33.7	-14.4	18.8	34.8	74.0	79.2
ion (Storage) mm 250.0 250.0 250.0 250.0 250.0 250.0 250.0 250.0 196.0 186.0 204.8 239.6 250.0 10 mm 548.0 59.8 0.0 0.0 0.0 0.0 0.0 -24.0 -30.0 -10.0 18.8 34.8 10.4 10.4 mm 548.0 59.8 0.0 0.0 0.0 30.2 75.1 103.5 120.4 103.3 70.8 35.6 9.1 10.4 10 10 10 10 10 10 10 10 10 10 10 10 10	ion (Storage) mm 250.0 250.0 250.0 250.0 250.0 250.0 250.0 226.0 196.0 186.0 204.8 239.6 250.0 10 mm 248.0 mm 248.0 0.0 0.0 0.0 0.0 0.0 -24.0 -30.0 -10.0 18.8 34.8 10.4 mm 369.0 40.2 54.3 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 34.8 74.0 74.0 mm 369.0 40.2 54.3 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 34.8 74.0 10.4 10.2 10.4 10.3 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4	ion (Storage)						-25.2	-58.8	-73.2				
mm 548.0 59.8 0.0 0.0 0.0 0.0 0.0 0.0 10.1 18.8 34.8 10.4 mm 548.0 59.8 0.0 0.0 0.0 30.2 75.1 103.5 120.4 103.3 70.8 35.6 9.1 mm 560.0 40.0 57.7 40.4 40.7 40.0 40.0 40.0 40.0	mm 548.0 59.8 0.0 0.0 0.0 0.0 -24.0 -30.0 -10.0 18.8 34.8 10.4 mm 548.0 59.8 0.0 0.0 0.0 30.2 75.1 103.5 120.4 103.3 70.8 35.6 9.1 mm 369.0 40.2 54.3 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 34.8 74.0							226.0	196.0	186.0	204.8	239.6	250.0	250.0
mm 548.0 59.8 0.0 0.0 0.0 30.2 75.1 103.5 120.4 103.3 70.8 35.6 9.1 mm 26.0 40.3 61.3 7.1 103.5 120.4 103.3 70.8 35.6 9.1	mm 548.0 59.8 0.0 0.0 0.0 30.2 75.1 103.5 120.4 103.3 70.8 35.6 9.1 mm 369.0 40.2 54.3 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 34.8 74.0							-24.0	-30.0	-10.0	18.8	34.8	10.4	0.0
	mm 369.0 40.2 54.3 55.6 72.7 42.4 1.2 -24.0 -30.0 -10.0 18.8 34.8 74.0							103.5	120.4	103.3	70.8	35.6	9.1	0.0
11111 303.U 4U.Z 34.3 33.0 12.1 42.4 1.2 -24.0 -30.0 10.0 10.0 34.0 14.0								-24.0	-30.0	-10.0	18.8	34.8	74.0	79.2
Summary of Pre-Development Water Balance			ological Technical Inf	ormation Require	ements Fo	or Land Dev	relopmetn A	\pplications	а", Аргіі 199£	5)				
Summary of Pre-Development Water Balance (by MOE Method in "Hydrogeological Technical Information Requirements For Land Developmetn Applications", April 1995)	(by MOE Method in "Hydrogeological Technical Information Requirements For Land Developmetn Applications", April 1995)	Pre-Developm	velopment Infiltrat	ion Factors			4	Annual	Annual	Annual	Annual			Annual
ation Requirements For Land Developmetn Applications", April 1995) Factors Annual Annual Annual Annual	Annual Annual				S	sum of		Evap	Surplus	Infiltration	Runoff		Area	Infiltration
ation Requirements For Land Developmetn Applications", April 1995) Factors Annual Annual Annual Annual Annual Area Surplus Infiltration Runoff Area	Annual Annual Infiltration Runoff Area	Topography Soil	Soil	Cover	ű	actors		(mm)	(mm)	(mm)	(mm)		(m ²)	(m ³ /yr)
ation Requirements For Land Developmetn Applications", April 1995) Factors Annual Annual Annual Annual Arnual Area Cover Factors (mm) (mm) (mm) (mm) (mm) (mm)	Annual Annual Infiltration Runoff Area (mm) (mm) (m ²)	Rolling 0.15 Sand/Silt 0	0.35	Cultivated 0.15		0.65		548	369	240	129		36200	8683

Cover Cultivated 0.15 & Grass Mix

Sand/Silt 0.35 Loam

Topography Rolling 0.15 to Hilly